

CITY OF TRACY

CITYWIDE STORM DRAINAGE MASTER PLAN



November 2012

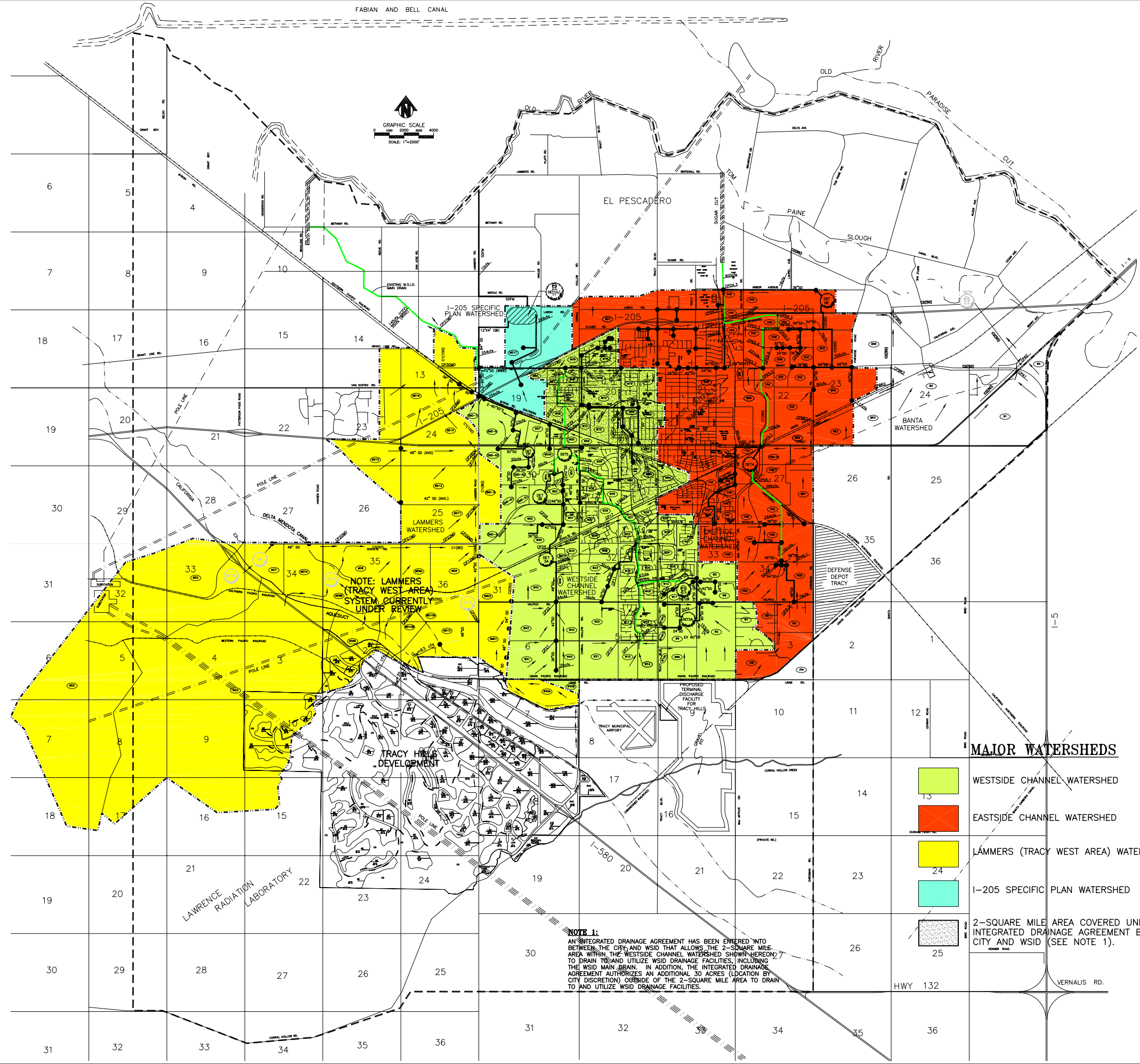
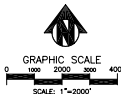
APPENDICES



APPENDIX A

EXHIBIT 3A FROM THE 1994 STORM DRAINAGE MASTER PLAN

FABIAN AND BELL CANAL



NOTE: LAMMERS (TRACY WEST AREA) SYSTEM CURRENTLY UNDER REVIEW

NOTE 1:
AN INTEGRATED DRAINAGE AGREEMENT HAS BEEN ENTERED INTO BETWEEN THE CITY AND WSID THAT ALLOWS THE 2-SQUARE MILE AREA WITHIN THE WESTSIDE CHANNEL WATERSHED SHOWN HEREON TO DRAIN TO AND UTILIZE WSID DRAINAGE FACILITIES, INCLUDING THE WSID MAIN BRAIN. IN ADDITION, THE INTEGRATED DRAINAGE AGREEMENT AUTHORIZES AN ADDITIONAL 30 ACRES (LOCATION BY CITY DISCRETION) OUTSIDE OF THE 2-SQUARE MILE AREA TO DRAIN TO AND UTILIZE WSID DRAINAGE FACILITIES.

- MAJOR WATERSHEDS**
- WESTSIDE CHANNEL WATERSHED
 - EASTSIDE CHANNEL WATERSHED
 - LAMMERS (TRACY WEST AREA) WATERSHED
 - I-205 SPECIFIC PLAN WATERSHED
 - 2-SQUARE MILE AREA COVERED UNDER INTEGRATED DRAINAGE AGREEMENT BETWEEN CITY AND WSID (SEE NOTE 1).

DETENTION BASIN DATA			
BASIN	Q IN	Q OUT	V
DET 2A	153cfs	2cfs	80AF
DET 2B	84cfs	6cfs	37AF
DET 3A	104cfs	15cfs	59AF
DET 3B	103cfs	1cfs	116AF
DET 4	344cfs	23cfs	153AF
DET 5	259cfs	136cfs	33AF
DET 6	203cfs	56cfs	109AF
DET 7	1145cfs	138cfs	279AF
DET 8	536cfs	62cfs	117AF
DET 9	231cfs	151cfs	84AF
DET 10			
DET 11	521cfs	10cfs	406AF
SW(DB3MD)	86cfs	69cfs	32AF
DET NE	471cfs	16cfs	204AF
DET CP	61cfs	9cfs	12AF

- LEGEND**
- C1 (60) DRAINAGE CHANNEL REACH AND TYPE
 - 54" SD STORM DRAIN REACH AND EQUIVALENT DIAMETER
 - 152 cfs FUTURE 100-YR DISCHARGE/ PEAK RUNOFF RATE
 - STORMWATER DETENTION FACILITY
 - PUMP STATION
 - FLOW DIRECTION
 - SPHERE OF INFLUENCE
 - MAJOR WATERSHED BOUNDARY
 - SUB-BASIN BOUNDARY
 - D40 SUB-BASIN NAME/NUMBER
 - CPA2 DRAINAGE MODEL CONCENTRATION POINT



CITY OF TRACY

SUPPLEMENT TO STORM DRAINAGE MASTER PLAN (SDMP)

EXHIBIT 3A DRAINAGE INFRASTRUCTURE PLAN, WITH WATERSHED BOUNDARIES

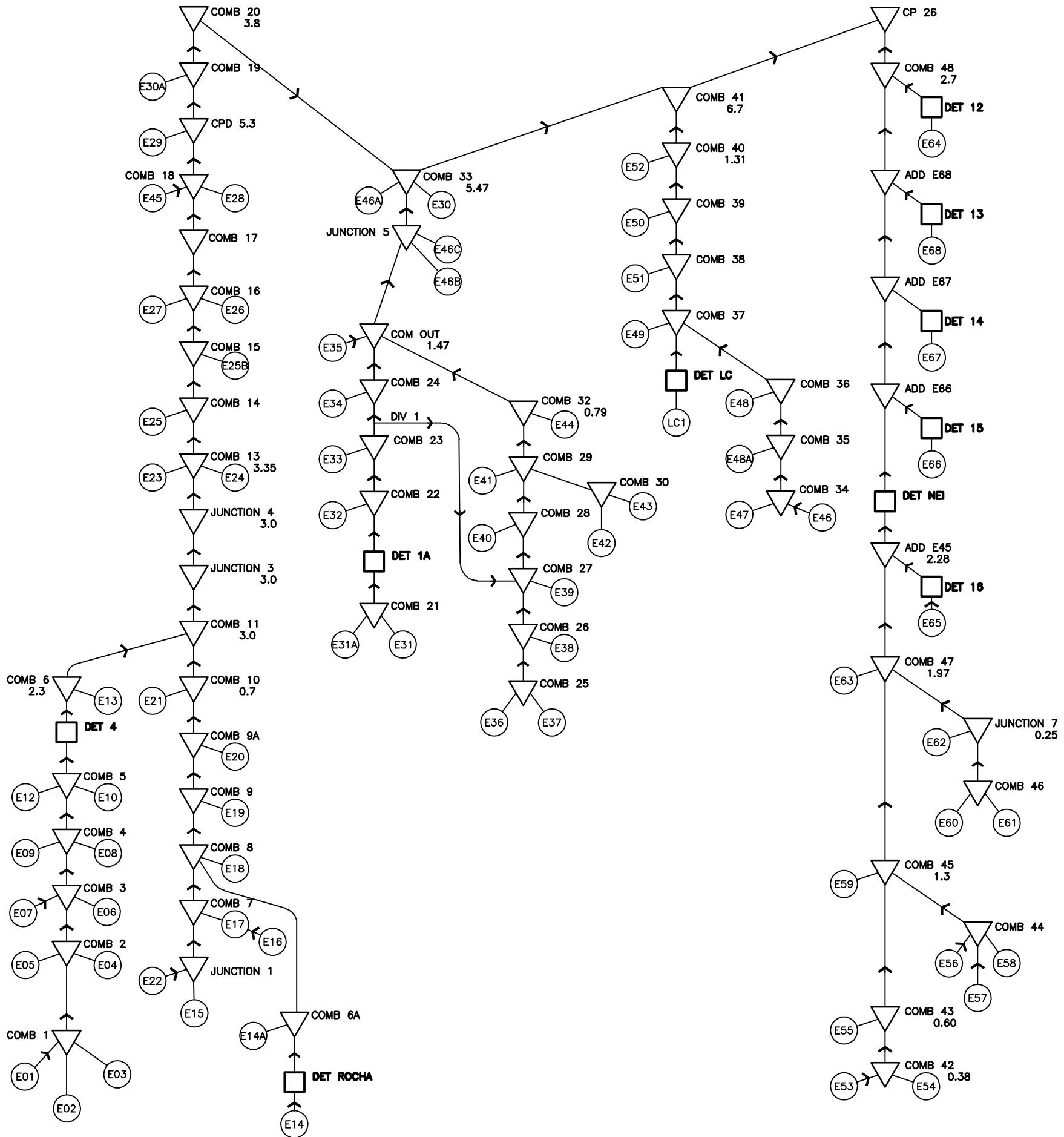
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SHEET: 1 OF 1
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


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APPENDIX B

HYDROLOGIC MODEL (HEC-HMS)



LEGEND

-  SUB-BASIN
-  ROUTING REACH
-  CONCENTRATION POINT
-  DETENTION BASIN

X.XX AREA (SQUARE MILES)

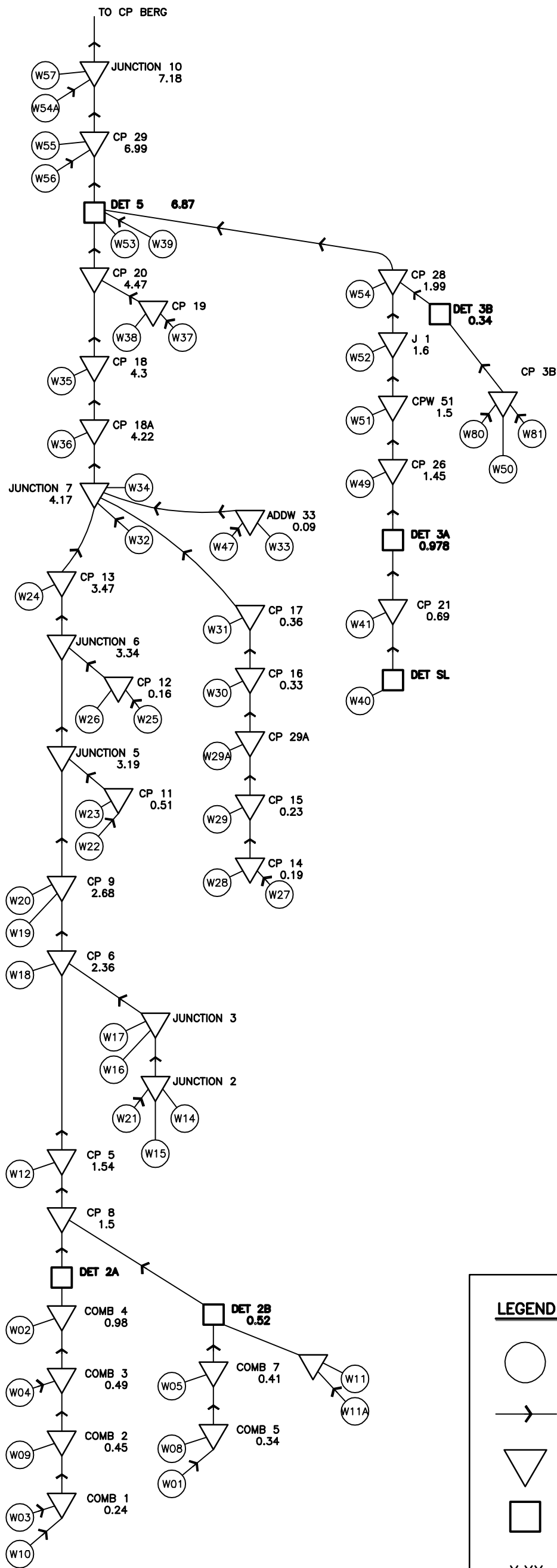
Eastside Channel Watershed



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 MASTER PLAN

APPENDIX B
 Title **HEC-HMS MODEL
 SCHEMATIC FLOW
 DIAGRAM**

MARCH 2012
 184010207
 NOT TO SCALE



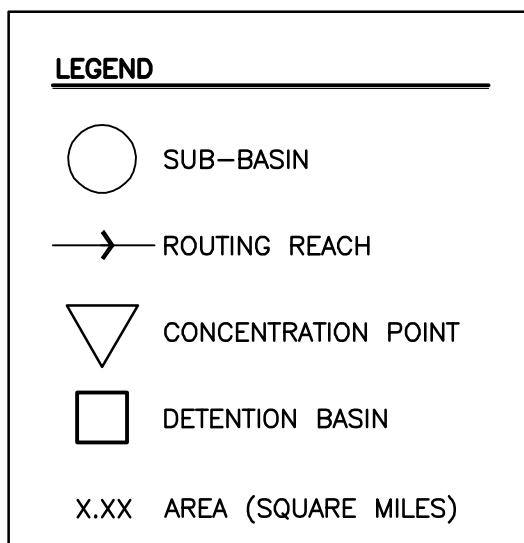
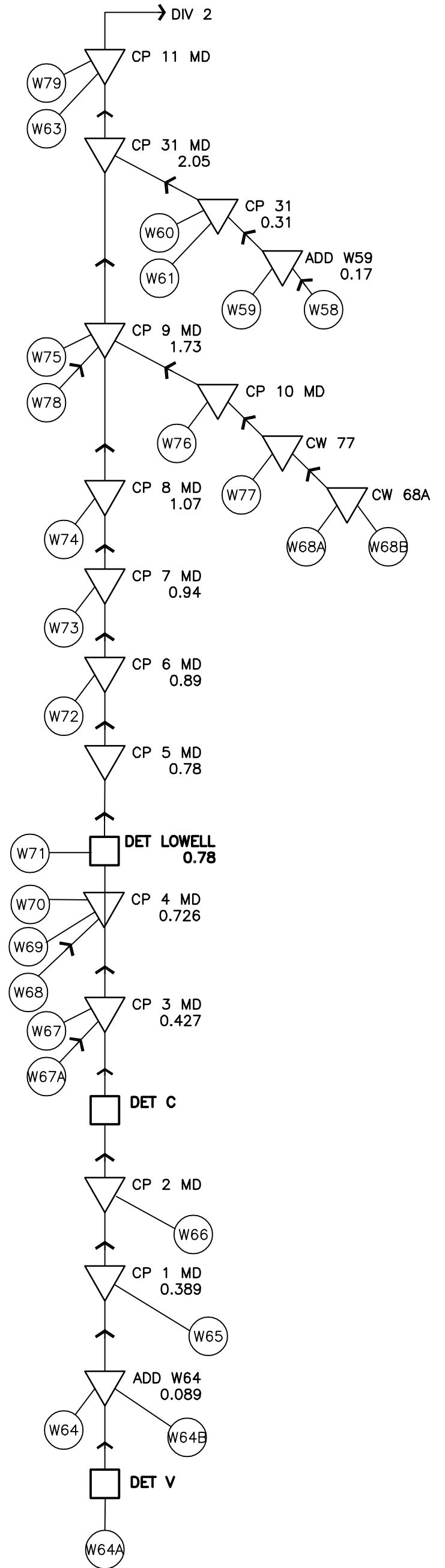
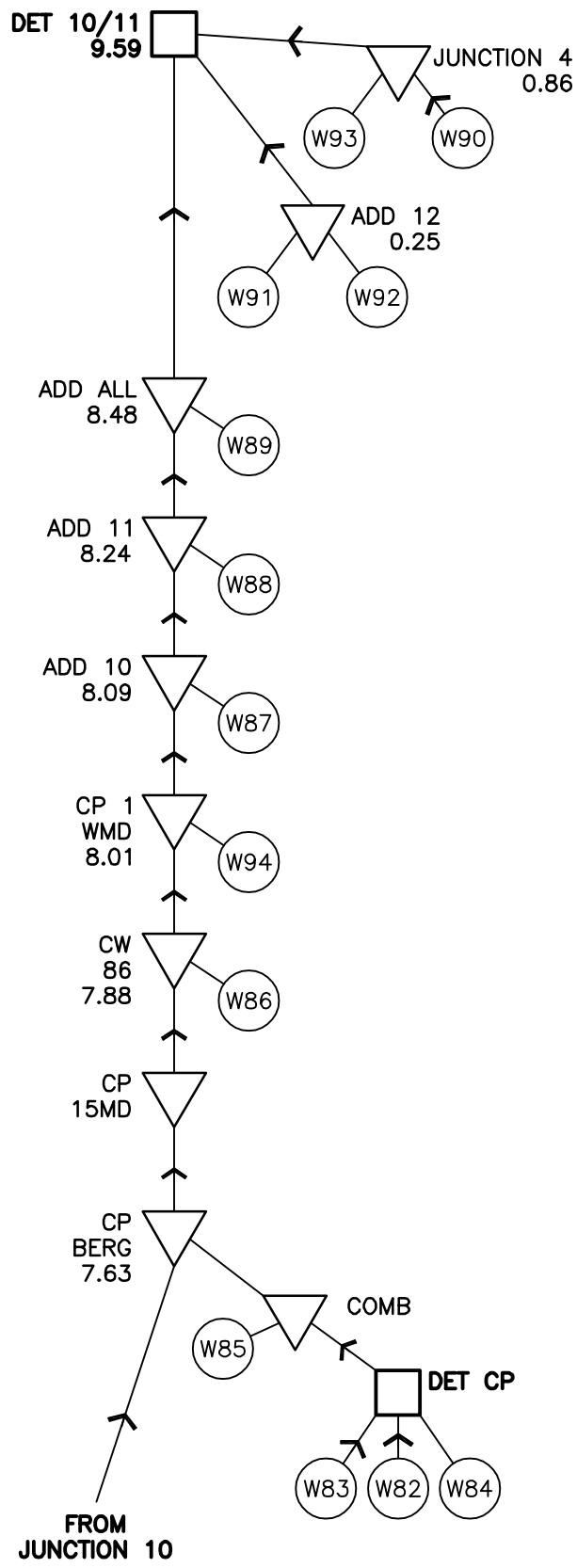
Westside Channel Watershed



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MASTER PLAN

APPENDIX B
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SCHEMATIC FLOW
DIAGRAM**

MARCH 2012
184010207
NOT TO SCALE



Westside Channel Watershed (cont'd)

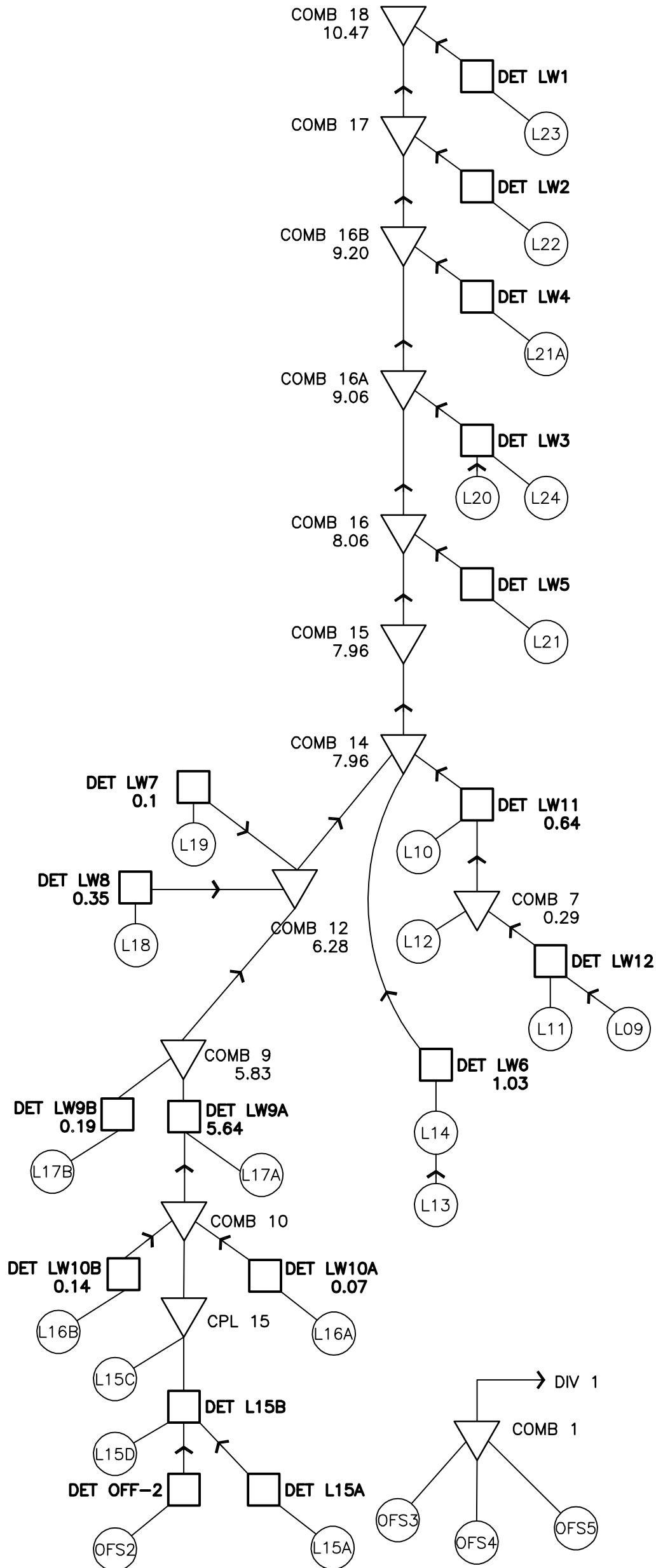


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 MASTER PLAN





APPENDIX B

Title **HEC-HMS MODEL SCHEMATIC FLOW DIAGRAM**

MARCH 2012
 184010207
 NOT TO SCALE



LEGEND

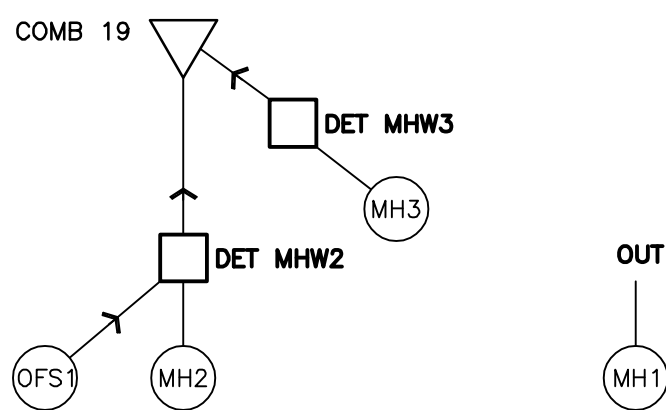
-  SUB-BASIN
-  ROUTING REACH
-  CONCENTRATION POINT
-  DETENTION BASIN
- X.XX AREA (SQUARE MILES)

Lammers Watershed

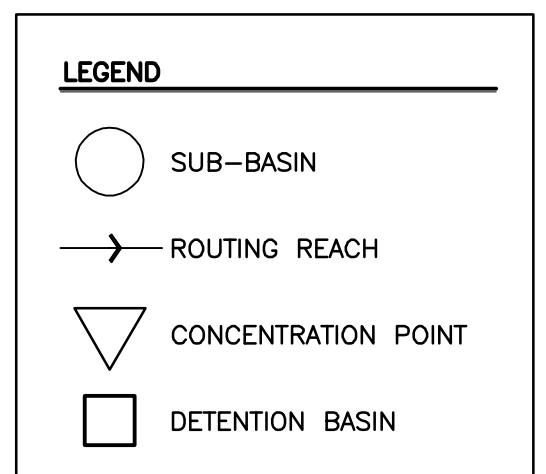


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 CITYWIDE STORM DRAINAGE
 MASTER PLAN

APPENDIX B
 Title **HEC-HMS MODEL
 SCHEMATIC FLOW
 DIAGRAM**
 MARCH 2012
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Mountain House Watershed



Client/Project
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 CITYWIDE STORM DRAINAGE
 MASTER PLAN

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 SCHEMATIC FLOW
 DIAGRAM**
 MARCH 2012
 184010207
 NOT TO SCALE

**CITY OF TRACY
STORM DRAINAGE MASTER PLAN
INPUT PARAMETERS**

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adusted CN	S value	Lagtime (hrs)	Lagtime (mins)
E01	4487.06	0.0030	0.30	74.0	3%	74.8	3.4	2.25	135.18
E02	5282.00	0.0030	0.30	74.2	25%	80.6	2.4	2.15	129.17
E03	2715.00	0.0030	0.30	74.0	25%	80.5	2.4	1.27	76.19
E04	4845.02	0.0030	0.30	74.2	25%	80.7	2.4	2.01	120.33
E05	4925.93	0.0030	0.30	74.0	26%	80.8	2.4	2.03	121.51
E06	4593.81	0.0030	0.30	75.7	13%	78.9	2.7	2.04	122.13
E07	2512.81	0.0030	0.30	74.4	17%	78.8	2.7	1.26	75.55
E08	5352.46	0.0030	0.30	78.4	13%	81.2	2.3	2.14	128.37
E09	3519.80	0.0030	0.30	77.3	13%	80.2	2.5	1.58	94.54
E10	2382.26	0.0030	0.30	78.9	13%	81.7	2.2	1.10	66.13
E12	4026.80	0.0030	0.30	80.0	21%	84.1	1.9	1.54	92.70
E13	2004.27	0.0030	0.30	80.0	13%	82.6	2.1	0.93	55.86
E14	2085.99	0.0030	0.30	74.0	22%	79.7	2.5	1.05	63.28
E14A	3225.89	0.0030	0.30	74.0	26%	80.9	2.4	1.44	86.40
E15	3193.82	0.0030	0.30	74.2	33%	82.7	2.1	1.35	80.83
E16	1401.25	0.0030	0.30	74.0	54%	88.1	1.4	0.58	34.51
E17	1844.10	0.0030	0.30	74.0	42%	84.9	1.8	0.81	48.39
E18	1323.12	0.0030	0.30	74.0	44%	85.6	1.7	0.60	36.18
E19	1754.51	0.0030	0.30	75.8	35%	84.2	1.9	0.79	47.56
E20	2956.20	0.0030	0.30	79.9	44%	88.8	1.3	1.54	92.40
E21	1365.50	0.0030	0.30	80.0	82%	96.3	0.4	1.25	75.00
E22	1202.92	0.0030	0.30	74.0	28%	81.3	2.3	0.65	38.77
E23	4117.28	0.0030	0.30	77.6	90%	97.8	0.2	0.87	51.90
E24	4882.88	0.0030	0.30	77.4	90%	97.7	0.2	0.99	59.56
E25	1511.06	0.0030	0.30	74.7	90%	97.5	0.3	0.39	23.69
E25B	1242.45	0.0030	0.30	79.6	90%	98.0	0.2	0.33	19.67
E26	1926.32	0.0030	0.30	80.0	57%	91.4	0.9	0.65	38.92
E27	3332.47	0.0030	0.30	80.0	90%	98.0	0.2	0.72	43.20
E28	3323.26	0.0030	0.30	80.0	90%	98.0	0.2	0.72	43.11
E29	1999.81	0.0030	0.30	80.0	74%	94.8	0.5	0.57	34.24
E30	2269.12	0.0030	0.30	80.0	90%	98.0	0.2	0.53	31.77
E30A	1049.76	0.0030	0.30	80.0	57%	91.4	0.9	0.40	23.95
E31	741.58	0.0030	0.30	76.2	35%	84.5	1.8	0.39	23.64

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adusted CN	S value	Lagtime (hrs)	Lagtime (mins)
E31A	2412.16	0.0030	0.30	71.7	37%	82.3	2.2	1.09	65.43
E32	1389.95	0.0030	0.30	79.7	34%	86.6	1.5	0.78	46.80
E33	4252.99	0.0030	0.30	80.0	73%	94.7	0.6	2.17	130.20
E34	5602.82	0.0030	0.30	80.0	58%	91.6	0.9	1.99	119.40
E35	1478.87	0.0030	0.30	79.3	90%	97.9	0.2	0.38	22.65
E36	1889.17	0.0030	0.30	80.0	52%	90.4	1.1	0.67	39.94
E37	1956.55	0.0030	0.30	80.0	47%	89.5	1.2	0.71	42.67
E38	2864.09	0.0030	0.30	80.0	50%	90.1	1.1	0.94	56.50
E39	1977.00	0.0030	0.30	80.0	41%	88.2	1.3	0.75	45.25
E40	3327.29	0.0030	0.30	80.0	34%	86.9	1.5	1.20	72.16
E41	2601.33	0.0030	0.30	80.0	34%	86.8	1.5	0.99	59.36
E42	4342.95	0.0030	0.30	80.0	45%	88.9	1.2	1.38	82.64
E43	4477.01	0.0030	0.30	80.0	47%	89.4	1.2	1.39	83.20
E44	2533.42	0.0030	0.30	80.0	36%	87.2	1.5	0.96	57.37
E45	1564.46	0.0030	0.30	80.0	90%	98.0	0.2	0.39	23.59
E46	1497.31	0.0030	0.30	80.0	48%	89.7	1.2	0.57	34.23
E46A	2443.88	0.0030	0.30	80.0	29%	85.7	1.7	0.98	58.82
E46B	2443.88	0.0030	0.30	80.0	25%	85.0	1.8	1.01	60.32
E46C	2296.26	0.0030	0.30	80.0	50%	90.0	1.1	0.79	47.47
E47	2000.77	0.0030	0.30	80.0	28%	85.5	1.7	0.84	50.47
E48	5281.52	0.0030	0.30	79.4	44%	88.4	1.3	1.64	98.45
E48A	3185.91	0.0030	0.30	80.0	42%	88.5	1.3	1.09	65.57
E49	1041.48	0.0030	0.30	80.0	90%	98.0	0.2	0.28	17.04
E50	3218.02	0.0030	0.30	80.0	90%	98.0	0.2	0.70	42.01
E51	1910.98	0.0030	0.30	80.0	90%	98.0	0.2	0.46	27.69
E52	2963.13	0.0030	0.30	80.0	71%	94.2	0.6	0.80	48.25
E53	4995.76	0.0030	0.30	80.0	13%	82.6	2.1	1.93	116.00
E54	4360.34	0.0030	0.30	80.0	74%	94.8	0.5	1.06	63.88
E55	3743.30	0.0030	0.30	80.0	38%	87.6	1.4	1.29	77.19
E56	4339.36	0.0030	0.30	79.7	90%	98.0	0.2	0.89	53.46
E57	1715.23	0.0030	0.30	74.1	57%	88.9	1.3	0.66	39.36
E58	2289.24	0.0030	0.30	74.5	57%	89.0	1.2	0.82	49.29
E59	5158.04	0.0030	0.30	79.0	74%	94.4	0.6	1.24	74.43
E60	3165.66	0.0030	0.30	78.2	57%	90.6	1.0	1.00	59.82
E61	1239.62	0.0030	0.30	80.0	57%	91.4	0.9	0.46	27.36
E62	2556.39	0.0030	0.30	80.0	82%	96.4	0.4	0.64	38.32
E63	3064.31	0.0030	0.30	80.0	82%	96.4	0.4	0.74	44.30

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adjusted CN	S value	Lagtime (hrs)	Lagtime (mins)
E64	1176.72	0.0030	0.30	80.0	57%	91.4	0.9	0.44	26.24
E65	5179.40	0.0030	0.30	80.0	57%	91.4	0.9	1.43	85.87
E66	2377.93	0.0030	0.30	80.0	57%	91.4	0.9	0.77	46.07
E67	1027.21	0.0030	0.30	80.0	57%	91.4	0.9	0.39	23.54
E68	1620.35	0.0030	0.30	80.0	57%	91.4	0.9	0.56	33.89
L24	9458.68	0.0030	0.30	76.9	49%	88.2	1.3	2.64	158.63
L01	3539.97	0.0030	0.30	80.0	3%	80.6	2.4	1.57	93.91
L02	2647.27	0.0030	0.30	77.3	3%	78.0	2.8	1.34	80.59
L03	2456.68	0.0030	0.30	76.6	3%	77.3	2.9	1.29	77.61
L04	6877.34	0.0030	0.30	78.6	3%	79.2	2.6	2.78	166.81
L05	3779.47	0.0030	0.30	79.7	57%	91.3	1.0	1.12	67.15
L06	3873.89	0.0030	0.30	78.3	3%	78.9	2.7	1.77	106.34
L07	3597.98	0.0030	0.30	80.0	3%	80.6	2.4	1.59	95.13
L08	4820.90	0.0030	0.30	76.6	3%	77.3	2.9	2.22	133.06
L09	3373.86	0.0030	0.30	78.7	3%	79.3	2.6	1.57	94.12
L10	3749.23	0.0030	0.30	75.1	6%	76.7	3.0	1.85	110.88
L11	1596.93	0.0030	0.30	77.5	57%	90.3	1.1	0.58	35.04
L12	3061.25	0.0030	0.30	79.6	57%	91.2	1.0	0.95	56.82
L13	1353.29	0.0030	0.30	80.0	57%	91.4	0.9	0.49	29.35
L14	5321.00	0.0030	0.30	79.1	57%	91.0	1.0	1.49	89.29
L15a	2779.00	0.0030	0.30	79.5	90%	97.9	0.2	0.62	37.48
L15b	2227.92	0.0030	0.30	80.0	90%	98.0	0.2	0.52	31.31
L15c	1300.42	0.0030	0.30	80.0	90%	98.0	0.2	0.34	20.35
L15d	1811.32	0.0030	0.30	80.0	90%	98.0	0.2	0.44	26.53
L16A	1351.00	0.0030	0.30	80.0	57%	91.4	0.9	0.49	29.31
L16B	2304.60	0.0030	0.30	78.7	57%	90.8	1.0	0.77	46.01
L17A	3722.20	0.0030	0.30	80.0	57%	91.4	0.9	1.10	65.93
L17B	3060.32	0.0030	0.30	80.0	57%	91.4	0.9	0.94	56.37
L18	3026.94	0.0030	0.30	77.7	57%	90.4	1.1	0.97	58.26
L19	2945.62	0.0030	0.30	80.0	57%	91.4	0.9	0.91	54.67
L20	2302.04	0.0030	0.30	77.6	57%	90.4	1.1	0.78	46.92
L21	2549.20	0.0030	0.30	76.0	32%	83.6	2.0	1.09	65.56
L21A	3048.00	0.0030	0.30	77.7	38%	86.2	1.6	1.15	68.98
L22	4670.31	0.0030	0.30	76.3	54%	89.0	1.2	1.46	87.35
L23	7323.40	0.0030	0.30	78.9	51%	89.7	1.1	2.02	121.45
L24	9458.68	0.0030	0.30	76.9	57%	90.1	1.1	2.45	147.00
LC1	4448.40	0.0030	0.30	80.0	57%	91.4	0.9	1.27	76.03

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adusted CN	S value	Lagtime (hrs)	Lagtime (mins)
MH1	4267.28	0.0030	2.02	61.6	56%	83.3	2.0	0.64	38.47
MH2	5480.00	0.0030	0.30	72.1	90%	97.2	0.3	1.12	67.38
MH3	3170.00	0.0030	1.26	77.3	57%	90.2	1.1	0.50	29.71
OFF1	6534.00	0.0030	4.09	65.1	3%	66.2	5.1	1.04	62.51
OFF2	24587.00	0.0030	5.78	74.0	3%	74.7	3.4	2.00	120.28
OFF3	20827.00	0.0030	6.66	74.2	3%	74.9	3.3	1.62	97.47
OFF4	3404.00	0.0030	10.58	63.0	3%	64.1	5.6	0.41	24.37
OFF5	2348.00	0.0030	2.98	80.0	3%	80.6	2.4	0.36	21.45
W01	3003.73	0.0030	0.30	74.0	25%	80.5	2.4	1.38	82.60
W02	6951.27	0.0030	0.30	74.0	26%	80.8	2.4	2.67	160.11
W03	1639.00	0.0030	0.30	68.9	57%	86.6	1.5	0.69	41.31
W04	2050.53	0.0030	0.30	63.3	57%	84.2	1.9	0.90	53.83
W05	3031.71	0.0030	0.30	74.0	25%	80.5	2.4	1.39	83.22
W08	3795.51	0.0030	0.30	74.0	35%	83.0	2.1	1.53	91.92
W09	3617.94	0.0030	0.30	75.7	74%	93.7	0.7	0.97	58.09
W10	2283.17	0.0030	0.30	78.0	35%	85.6	1.7	0.93	55.96
W11	1465.09	0.0030	0.30	74.0	53%	87.8	1.4	0.60	36.20
W11A	1094.60	0.0030	0.30	73.9	90%	97.4	0.3	0.31	18.38
W12	2015.01	0.0030	0.30	73.6	33%	82.2	2.2	0.95	56.80
W14	3036.68	0.0030	0.30	62.3	25%	71.7	3.9	1.80	107.81
W15	3923.26	0.0030	0.30	61.0	31%	73.2	3.7	2.12	127.10
W16	3325.13	0.0030	0.30	61.0	30%	72.7	3.7	1.88	112.71
W17	3921.14	0.0030	0.30	61.0	25%	70.7	4.1	2.27	136.05
W18	2723.63	0.0030	0.30	61.8	32%	74.2	3.5	1.54	92.23
W19	5327.38	0.0030	0.30	64.0	24%	72.8	3.7	2.74	164.15
W20	3608.54	0.0030	0.30	73.1	29%	80.9	2.4	1.57	94.42
W21	5096.05	0.0030	0.30	61.0	31%	72.9	3.7	2.63	157.69
W22	3647.65	0.0030	0.30	61.2	21%	69.4	4.4	2.22	132.91
W23	3726.25	0.0030	0.30	62.6	22%	70.8	4.1	2.17	130.07
W24	2831.44	0.0030	0.30	63.1	35%	75.9	3.2	1.51	90.49
W25	3068.77	0.0030	0.30	74.0	25%	80.5	2.4	1.40	84.03
W26	2626.92	0.0030	0.30	74.0	25%	80.5	2.4	1.24	74.20
W27	3666.23	0.0030	0.30	74.1	30%	81.8	2.2	1.55	92.94
W28	2406.47	0.0030	0.30	74.0	25%	80.5	2.4	1.15	69.18
W29	1617.88	0.0030	0.30	74.0	25%	80.5	2.4	0.84	50.35
W29A	1222.24	0.0030	0.30	74.9	53%	88.2	1.3	0.51	30.80
W30	2905.66	0.0030	0.30	71.4	27%	79.2	2.6	1.40	83.75

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adjusted CN	S value	Lagtime (hrs)	Lagtime (mins)
W31	1195.14	0.0030	0.30	65.0	32%	76.2	3.1	0.75	45.02
W32	1229.48	0.0030	0.30	72.5	25%	79.4	2.6	0.70	41.87
W33	1991.64	0.0030	0.30	72.6	25%	79.4	2.6	1.03	61.50
W34	2667.82	0.0030	0.30	63.8	25%	72.9	3.7	1.57	94.12
W35	2920.69	0.0030	0.30	64.0	40%	78.3	2.8	1.44	86.55
W36	1919.51	0.0030	0.30	61.3	25%	71.0	4.1	1.27	76.22
W37	1601.94	0.0030	0.30	61.0	25%	70.7	4.1	1.11	66.46
W38	3439.73	0.0030	0.30	70.2	25%	77.6	2.9	1.68	100.70
W39	4133.43	0.0030	0.30	70.2	33%	80.0	2.5	1.81	108.43
W40	3044.13	0.0030	0.30	72.6	57%	88.2	1.3	1.06	63.88
W41	3189.00	0.0030	0.30	61.0	27%	71.6	4.0	1.87	112.33
W41A	2575.00	0.0030	0.30	61.0	16%	67.2	4.9	1.78	106.56
W41B	973.50	0.0030	0.30	64.9	77%	91.8	0.9	0.37	22.20
W43	5604.71	0.0030	0.30	74.5	3%	75.3	3.3	2.65	159.15
W44	1431.75	0.0030	0.30	80.0	3%	80.6	2.4	0.76	45.52
W45	1506.20	0.0030	0.30	80.0	3%	80.6	2.4	0.79	47.40
W46	3035.28	0.0030	0.30	74.4	3%	75.1	3.3	1.63	97.91
W47	1130.16	0.0030	0.30	74.0	25%	80.5	2.4	0.63	37.82
W49	7711.03	0.0030	0.30	71.5	25%	78.5	2.7	3.12	186.93
W50	3688.16	0.0030	0.30	77.1	28%	83.5	2.0	1.47	88.25
W51	1846.87	0.0030	0.30	64.9	23%	73.1	3.7	1.16	69.69
W52	2367.85	0.0030	0.30	76.4	28%	83.0	2.0	1.05	62.91
W53	3080.60	0.0030	0.30	74.0	24%	80.1	2.5	1.42	85.27
W54	2381.72	0.0030	0.30	77.5	24%	82.9	2.1	1.06	63.47
W54A	2644.16	0.0030	0.30	74.1	25%	80.5	2.4	1.24	74.59
W55	2748.99	0.0030	0.30	74.3	25%	80.7	2.4	1.28	76.55
W56	2467.03	0.0030	0.30	75.5	90%	97.5	0.3	0.58	34.90
W57	3280.89	0.0030	0.30	76.2	30%	83.3	2.0	1.35	80.93
W58	2778.38	0.0030	0.30	75.8	32%	83.5	2.0	1.17	70.44
W59	1993.85	0.0030	0.30	74.1	25%	80.6	2.4	0.99	59.26
W60	2685.10	0.0030	0.30	78.9	42%	87.7	1.4	0.98	58.86
W61	3327.36	0.0030	0.30	79.8	36%	87.1	1.5	1.19	71.60

Subbasin	Length	Slope, ft/ft	Slope %	Weighted CN	% Imp	Adusted CN	S value	Lagtime (hrs)	Lagtime (mins)
W63	1605.59	0.0030	0.30	80.0	90%	98.0	0.2	0.40	24.09
W64	1626.53	0.0030	0.30	64.2	33%	76.2	3.1	0.96	57.62
W64A	1942.33	0.0030	0.30	80.0	57%	91.4	0.9	0.65	39.18
W64B	1237.62	0.0030	0.30	72.0	63%	89.7	1.1	0.49	29.33
W65	3301.82	0.0030	0.30	73.4	30%	81.3	2.3	1.45	86.85
W66	5312.00	0.0030	0.30	72.8	25%	79.6	2.6	2.23	133.86
W67	1077.93	0.0030	0.30	80.0	88%	97.6	0.2	0.30	17.98
W67A	1379.65	0.0030	0.30	80.0	44%	88.8	1.3	0.55	33.17
W68	3214.05	0.0030	0.30	80.0	35%	87.1	1.5	1.16	69.69
W68A	4126.66	0.0030	0.30	80.0	54%	90.8	1.0	1.22	73.47
W68B	4229.79	0.0030	0.30	80.0	37%	87.4	1.4	1.43	85.65
W69	2626.68	0.0030	0.30	80.0	31%	86.1	1.6	1.02	61.38
W70	3442.87	0.0030	0.30	80.0	41%	88.1	1.3	1.18	70.79
W71	3551.10	0.0030	0.30	80.0	32%	86.5	1.6	1.29	77.11
W72	2726.14	0.0030	0.30	79.1	24%	84.1	1.9	1.13	67.83
W73	2350.20	0.0030	0.30	74.4	25%	80.8	2.4	1.12	67.19
W74	4292.17	0.0030	0.30	79.0	54%	90.3	1.1	1.29	77.32
W75	3991.37	0.0030	0.30	79.9	29%	85.8	1.7	1.45	86.94
W76	3049.10	0.0030	0.30	79.6	26%	84.9	1.8	1.20	72.21
W77	2651.88	0.0030	0.30	80.0	56%	91.1	1.0	0.85	50.83
W78	4197.38	0.0030	0.30	78.3	40%	86.9	1.5	1.44	86.68
W79	2451.80	0.0030	0.30	80.0	74%	94.8	0.5	0.67	40.31
W80	2262.15	0.0030	0.30	80.0	23%	84.6	1.8	0.96	57.51
W81	1759.00	0.0030	0.30	76.0	19%	80.6	2.4	0.89	53.58
W82	2767.03	0.0030	0.30	76.1	23%	81.6	2.3	1.25	74.79
W83	1489.42	0.0030	0.30	80.0	37%	87.4	1.4	0.62	37.23
W84	2644.16	0.0030	0.30	77.0	22%	82.1	2.2	1.18	70.85
W85	3493.87	0.0030	0.30	80.0	24%	84.7	1.8	1.35	81.07
W86	6345.39	0.0030	0.30	80.0	35%	86.9	1.5	2.01	120.85
W87	1456.76	0.0030	0.30	80.0	57%	91.4	0.9	0.52	31.13
W88	3166.20	0.0030	0.30	80.0	56%	91.1	1.0	0.98	58.56
W89	3113.81	0.0030	0.30	80.0	57%	91.4	0.9	0.95	57.16
W90	4004.62	0.0030	0.30	80.0	52%	90.3	1.1	1.22	73.20
W91	2761.36	0.0030	0.30	80.0	57%	91.4	0.9	0.87	51.92
W92	2727.40	0.0030	0.30	79.7	6%	80.9	2.4	1.26	75.44
W93	4144.89	0.0030	0.30	79.4	76%	95.1	0.5	1.01	60.38
W94	1865.55	0.0030	0.30	80.0	90%	98.0	0.2	0.45	27.16

CITY OF TRACY
 STORM DRAINAGE MASTER PLAN
 PERCENT IMPERVIOUS CALCULATIONS

				Land Use Area (SF)												
				RVL D	RLD	RMD	RHD	C	Ofc	I	Downtwn	Village	PF	Park	OS	
				10%	25%	35%	65%	90%	90%	90%	90%	60%	10%	3%	Weighted %	
				6%	16%	22%	41%	57%	57%	57%	57%	38%	6%	2%	Impervious	
Subbasin	Developed	Area (Sq-ft)	Area (Sq-mi)													
E01	E	5568703	0.1997												5568703	3%
E02	E	8527722	0.3059		8527722											25%
E03	E	2926044	0.1050		2926044											25%
E04	E	6260749	0.2246		5972716							110544.487	177488.86			25%
E05	E	7439140	0.2668		6937317							326671.49	175151.11			26%
E06	P	10180000	0.3652	4959492	3915431.6	848299.4	326269	130507.6								13%
E07	P	3854045	0.1382	1877614	1482342.77	321157.566	123522.141	49408.8563								17%
E08	P	5971219	0.2142	2909058	2296650.077	497581.641	191377.554	76551.0218								13%
E09	P	3042726	0.1091	1482355	1170293.185	253550.338	97519.3609	39007.7443								13%
E10	P	4286755	0.1538	2088421	1648771.781	357215.31	137390.504	54956.2015								13%
E12	E	3916395	0.1405	1907989	1506323.861	326353.199	125520.461	50208.1844								21%
E13	P	2689297	0.0965	1310172	1034357.576	224099.154	86191.9825	34476.793								13%
E14	P	4283600	0.1537		3270981		1012618.63									22%
E14A	E	4754935	0.1706		4286589	390530.76	77815.24									26%
E15	E	2517808	0.0903			2167527						73339.48	276942.01			33%
E16	E	939769	0.0337			217776.3						721993				54%
E17	E	1635410	0.0587			1240226		39988.109				355195.55				42%
E18	E	546907	0.0196			452692		94214.76								44%
E19	E	1543949	0.0554			1521569							22380.44			35%
E20	P	2014940	0.0723			725630				1289309.04						44%
E21	E	621178	0.0223				119529.54					73811.72				82%
E22	E	664351	0.0238		469212	195138.525										28%
E23	E	5646478	0.2025									5646478				90%
E24	E	3698238	0.1327									3698238				90%
E25	E	1096170	0.0393									1096170				90%
E25B	E	851183	0.0305									851183				90%
E26	P	967511	0.0347									967511				57%
E27	E	3486672	0.1251									3486672				90%
E28	E	2655018	0.0952									2655018				90%
E29	P	2487456	0.0892					2487456								74%
E30	E	1602256	0.0575									1602256				90%
E30A	P	541615	0.0194					541615								57%
E31	E	2478745	0.0889			2478745										35%
E31A	E	1894429	0.0680			1714059						180369.98				37%
E32	E	922492	0.0331			860475		9991.54					52024.76			34%
E33	E	5529602	0.1983		276453.45	860098.09	236458.98	57651.42	40423		3552846	403511.31	102159.63			73%
E34	E	7445459	0.2671			393527.34		132851.82			174.07	1303014.18	32940.67			58%
E35	E	754028	0.0270					28178.32			725850					90%
E36	E	826360	0.0296			473246		177437.52				175677.15				52%
E37	E	2252791	0.0808			1742522		150205.94			360062.46					47%
E38	E	1636718	0.0587			1177734		98105.52	218558.42		142319.93					50%
E39	E	2557297	0.0917		214982.91	1305175	204994.89		20681.96		224130.47	254084.75	333246.27			41%
E40	E	4485108	0.1609		3358737.19	5538.57	578731					542101.34				34%
E41	E	1188109	0.0426		525974.53	555121	64635.13	42377.79								34%
E42	E	3746516	0.1344		790618.21	1954093	409761.14	267230.4	310541.37				14271.89			45%
E43	E	2258020	0.0810		377582.23	1000094	722603.99	157740.41								47%
E44	E	3015603	0.1082		2422095	75514.94	59267.68	458194.24			531.05					36%
E45	E	669063	0.0240								669063					90%

		Land Use Area (SF)												
		RVLD	RLD	RMD	RHD	C	Ofc	I	Downtwn	Village	PF	Park	OS	
% Impervious Existing Development		10%	25%	35%	65%	90%	90%	90%	90%	90%	60%	10%	3%	Weighted %
% Impervious Proposed Development		6%	16%	22%	41%	57%	57%	57%	57%	57%	38%	6%	2%	Impervious
Subbasin	Developed	Area (Sq-ft)	Area (Sq-mi)											
E46	E	1532182	0.0550		87836.69	737956	262534.7	74786.56					369067.17	48%
E46A	E	3632261	0.1303		3384491		127445.28	120324.55						29%
E46B	E	961029	0.0345		961029									25%
E46C	E	1361522	0.0488			987523			373999.53					50%
E47	E	1319730	0.0473		977883	341847.54								28%
E48	E	8375028	0.3004		5104165	951132.48		2298401.34				21329.1		44%
E48A	E	6644700	0.2383		1441964.34	2872813	710940.93	1038274.86				580707.4		42%
E49	E	258207	0.0093					258207						90%
E50	E	1769683	0.0635					234669.52	1535014					90%
E51	E	3575992	0.1283					167745.13	3408247					90%
E52	E	3440576	0.1234						1276951			2163625.29		71%
E53	P	7802426	0.2799	3801186	3000968.911	650176.12	250067.739	100027.095						13%
E54	E	2864729	0.1028						2864729					74%
E55	P	6083734	0.2182									6083734		38%
E56	E	7518722	0.2697						7518722					90%
E57	P	1210302	0.0434						1210302					57%
E58	P	2544193	0.0913						2544193					57%
E59	P	10610000	0.3806						10610000					74%
E60	P	2602583	0.0934						2602583					57%
E61	P	918070	0.0329						918070					57%
E62	P	3468726	0.1244						3468726					82%
E63	P	9311894	0.3340						9311894					82%
E64	P	2543066	0.0912					577463.5	1965602					57%
E65	P	8638182	0.3099						8638182					57%
E66	P	3666616	0.1315						3666616					57%
E67	P	4493578	0.1612						4493578					57%
E68	P	2761599	0.0991					235021.05	2526578					57%
L24	P	23570000	0.8455					512873.13	17651150		1864198.21		3541778.19	49%
L01	E	3572889	0.1282										3572889	3%
L02	E	5065618	0.1817										5065618	3%
L03	E	6068829	0.2177										6068829	3%
L04	E	10400000	0.3730										10400000	3%
L05	P	6501441	0.2332						6501441					57%
L06	E	9818967	0.3522										9818967	3%
L07	E	5151225	0.1848										5151225	3%
L08	E	10640000	0.3817										10640000	3%
L09	E	5451145	0.1955										5451145	3%
L10	P	9820746	0.3523						68153.55			9752592		6%
L11	P	3644642	0.1307						3644642					57%
L12	P	4498276	0.1614						4498276					57%
L13	P	1606996	0.0576						1606996					57%
L14	P	27234684	0.9769					1343487	2371051.63	23520145.8				57%
L15a	E	7247320	0.2600							7247320				90%
L15b	E	3854256	0.1383							3854256				90%
L15c	E	691362	0.0248							691362				90%
L15d	E	1774341	0.0636							1774341				90%
L16A/B	P	5703418	0.2046					281350	496539.551	4925528.62				57%
L17A/B	P	17277363	0.6197					852292	1504167.24	14920903.6				57%
L18	P	9768424	0.3504					481876	850438.973	8436108.45				57%
L19	P	2732931	0.0980					134815	237928.997	2360186.78				57%
L20	P	4053293	0.1454					199949	352879.702	3500464.5				57%

		Land Use Area (SF)													
		RVLD	RLD	RMD	RHD	C	Ofc	I	Downtwn	Village	PF	Park	OS		
% Impervious Existing Development		10%	25%	35%	65%	90%	90%	90%	90%	90%	60%	10%	3%	Weighted %	
% Impervious Proposed Development		6%	16%	22%	41%	57%	57%	57%	57%	57%	38%	6%	2%	Impervious	
Subbasin	Developed	Area (Sq-ft)	Area (Sq-mi)												
L21	P	3023662	0.1085		647015.3938	647015.394	1729631								32%
L21A	E	3748653	0.1345								3748653				38%
L22	P	8514416	0.3054				1840953	4372268	2301195						54%
L23	P	26920000	0.9656	1742400				1730686.8	1538208.8	20166304		871200	871200		51%
LC1	P	9741368	0.3494					9741368							57%
MH1	P	7489576	0.2687							7281921		207655.8			56%
MH2	E	17494960	0.6275							17494960					90%
MH3	P	10901603	0.3910					537776	949093.517	9414732.97					57%
OFF1	E	11260000	0.4039											11260000	3%
OFF2	E	125900000	4.5160											125900000	3%
OFF3	E	158700000	5.6926											158700000	3%
OFF4	E	6609771	0.2371											6609771	3%
OFF5	E	2491000	0.0894											2491000	3%
W01	E	6028660	0.2162		6028660										25%
W02	E	13870000	0.4975		12843609						626428.01	399963.13			26%
W03	P	676988	0.0243						676988						57%
W04	P	948081	0.0340					200776.7	621178	126126.23					57%
W05	E	1763905	0.0633		1763905										25%
W08	P	3590627	0.1288		1337912		1580964.1	661053.87				10697.43			35%
W09	P	5826623	0.2090							5826623					74%
W10	E	6030058	0.2163		4616394		243057.84		815337.06	1544.75		353724.97			35%
W11	P	2263692	0.0812				2263692								53%
W11A	E	890573	0.0319								890573				90%
W12	E	1008784	0.0362		239002	769781.82									33%
W14	E	3412186	0.1224		3412186										25%
W15	E	3450050	0.1238		2548440						697312.16	204298.58			31%
W16	E	3775355	0.1354		2565325	462953.9					514726.82	232348.44			30%
W17	E	3488911	0.1251		3448281							40630.14			25%
W18	E	1827129	0.0655		428330.76	1383932						14866.84			32%
W19	E	5233086	0.1877		4986388							246698.01			24%
W20	E	3602985	0.1292		2973370	284048.24	300558.78					45008.09			29%
W21	E	6922374	0.2483		5892888		283190.35	482803.07				263492.97			31%
W22	P	7017524	0.2517		3778670	2321179	917675								21%
W23	P	7171923	0.2573		7171923										22%
W24	E	3599922	0.1291		2339843	621512.61		477218.82				161347.73			35%
W25	E	2696123	0.0967		2696123										25%
W26	E	1679457	0.0602		1679457										25%
W27	E	3441060	0.1234		2260094	537231.66					408744.45	234990.31			30%
W28	E	1866401	0.0669		1866401										25%
W29	E	1195141	0.0429		1195141										25%
W29A	E	462749	0.0166			129898					332851.01				53%
W30	E	2194154	0.0787		2085879			86578.05				21696.85			27%
W31	E	953131	0.0342		281394.83	671737									32%
W32	E	2554272	0.0916		2554272										25%
W33	E	1475703	0.0529		1475703										25%
W34	E	4343411	0.1558		4343411										25%
W35	E	2206661	0.0792		1283877						922783.66				40%
W36	E	1280446	0.0459		1280446										25%
W37	E	2167875	0.0778		2147074							20801.16			25%
W38	E	2744412	0.0984		2699288							45123.34			25%
W39	E	6789283	0.2435		5099895			976874.98				712512.48			33%

		Land Use Area (SF)												
		RVLD	RLD	RMD	RHD	C	Ofc	I	Downtwn	Village	PF	Park	OS	
% Impervious Existing Development		10%	25%	35%	65%	90%	90%	90%	90%	90%	60%	10%	3%	Weighted %
% Impervious Proposed Development		6%	16%	22%	41%	57%	57%	57%	57%	57%	38%	6%	2%	Impervious
Subbasin	Developed	Area (Sq-ft)	Area (Sq-mi)											
W40	P	5227200	0.1875					5227200						57%
W41	P	13999090	0.5021		5374210	4051080	1350360	1524600				1698840		27%
W41A	P	6787057	0.2435		6787057									16%
W41B	E	1265959	0.0454		174240			1045440						77%
W43	E	29340000	1.0524										29340000	3%
W44	E	1420222	0.0509										1420222	3%
W45	E	1631138	0.0585										1631138	3%
W46	E	5053554	0.1813										5053554	3%
W47	E	1172130	0.0420		1172130									25%
W49	E	13110000	0.4703		12730006							379993.964		25%
W50	P	5502535	0.1974		3034098	2468437.39					827640			28%
W51	E	1521689	0.0546			1353628						168061.23		23%
W52	E	2792154	0.1002			2554187					237966.852			28%
W53	E	4386814	0.1574			3193299	58910.32				169125.09	965479.59		24%
W54	E	1490251	0.0535			1319559					21785.84	148906.248		24%
W54A	E	2368568	0.0850			2318169						50399.76		25%
W55	E	2321616	0.0833			2299425						22191.28		25%
W56	E	1114725	0.0400					1114725						90%
W57	E	2893656	0.1038		2357120						435748.01	100788.14		30%
W58	E	3332637	0.1195		783477	2431149						118010.7		32%
W59	E	1441511	0.0517		1302024	102743.53						36743.99		25%
W60	E	2053405	0.0737		1470430							42579.17		42%
W61	E	2038002	0.0731		1513711			288783.87	88207.53			147299.64		36%
W63	E	738501	0.0265					738501						90%
W64	E	1414716	0.0507			1327757						86958.85		33%
W64A	P	1071792	0.0384						6146.94	1065645				57%
W64B	E	445885	0.0160							48096.77		397789		63%
W65	E	4049420	0.1453		1886515.99	2037712		5853.68				2626.55	116712.71	30%
W66	E	3878067	0.1391		3786315	91751.36								25%
W67	E	386802	0.0139			15362.71		371439						88%
W67A	E	655017	0.0235			6548201		106815.25						44%
W68	E	4782067	0.1715		3083023	173037.73	52627.6	374401.72	179123.83			471583.85	448269.23	35%
W68A	E	1367453	0.0491			549321	42240.02	65849.67	330834.22			246993.17	132215.56	54%
W68B	E	2553500	0.0916		261573.44	1644376	21371.92	177367.61	93775.38			29601.87	325433.22	37%
W69	E	2469518	0.0886		2118615	79119.17	56569.86	176708.67				38505.18		31%
W70	E	1089384	0.0391		656918	163462.77	85887.91	183115.82						41%
W71	E	1570826	0.0563		1390763			180062.12						32%
W72	E	2992523	0.1073		2836836							155686.38		24%
W73	E	1365876	0.0490		1365876									25%
W74	E	3725154	0.1336		655818						3069336.02			54%
W75	E	3867365	0.1387		3329270	332131.58		205963.48						29%
W76	E	2467109	0.0885		2249315	217793.83								26%
W77	E	2911704	0.1044		24418.28	1156365	251110.49	533911.27			945898.73			56%
W78	E	5277259	0.1893		454871.44	3534659	334444.88	300793.47			390587.78	261902.39		40%
W79	P	1659227	0.0595					376671.54	1282555					74%
W80	P	2585617	0.0927		948146	1206622	430849							23%
W81	P	1329528	0.0477		604270.5674	725257.634								19%
W82	P	2559096	0.0918		1066375.391	1066375.39	426601.338							23%
W83	P	1082840	0.0388		258895.9856	258895.986	129447.993	435600						37%
W84	E	5706468	0.2047			4588968						1117500.86		22%
W85	P	2971125	0.1066		202800.01	2597645		170679.98						24%

				Land Use Area (SF)												
				RVLD	RLD	RMD	RHD	C	Ofc	I	Downtwn	Village	PF	Park	OS	
% Impervious Existing Development				10%	25%	35%	65%	90%	90%	90%	90%	90%	60%	10%	3%	Weighted %
% Impervious Proposed Development				6%	16%	22%	41%	57%	57%	57%	57%	57%	38%	6%	2%	Impervious
Subbasin	Developed	Area (Sq-ft)	Area (Sq-mi)													
W86	E	7127787	0.2557		4322.033	6625300	163068.4								335096.45	35%
W87	P	1997121	0.0716					1797409.28	199712.142							57%
W88	P	4381899	0.1572					328642.432	292272.669	3468711			292272.669			56%
W89	P	6531822	0.2343					6531822								57%
W90	E	6969194	0.2500		2024659.69	1410357	513897.03	2178168.27					428395.73	413715.87		52%
W91	P	2328343	0.0835					2328343								57%
W92	P	4616184	0.1656	4616184												6%
W93	E	17050000	0.6116	1439940.89				13246932					1514468.4		848658.57	76%
W94	E	3744732	0.1343					3744732								90%

Eastside Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E02	0.3059	15.96	13Apr2010, 12:48	11.4
E01	0.1997	3.96	13Apr2010, 13:44	3.7
RR 01	0.1997	3.95	13Apr2010, 15:00	3.7
E03	0.105	7.18	13Apr2010, 11:48	3.9
RR 02	0.105	7.18	13Apr2010, 11:56	3.9
COMB 1	0.6106	22.33	13Apr2010, 12:28	19
RR 03	0.6106	22.33	13Apr2010, 12:36	19
E05	0.2668	14.7	13Apr2010, 12:40	10.1
RR 04	0.2668	14.7	13Apr2010, 12:40	10.1
E04	0.2246	12.17	13Apr2010, 12:36	8.4
COMB 2	1.102	49.19	13Apr2010, 12:36	37.6
RR 05	1.102	49.11	13Apr2010, 12:52	37.6
E06	0.3652	14.2	13Apr2010, 12:52	10.7
E07	0.1382	7.36	13Apr2010, 11:48	4.3
RR 06	0.1382	7.36	13Apr2010, 11:52	4.3
COMB 3	1.6054	68.84	13Apr2010, 12:48	52.5
RR 07	1.6054	68.69	13Apr2010, 13:12	52.5
E08	0.2142	9.77	13Apr2010, 13:00	7.2
E09	0.1091	5.41	13Apr2010, 12:16	3.5
COMB 4	1.9287	82.93	13Apr2010, 13:08	63.1
RR 08	1.9287	82.9	13Apr2010, 13:12	63.1
E10	0.1538	10.36	13Apr2010, 11:40	5.3
E12	0.1405	10.35	13Apr2010, 12:08	5.9
COMB 5	2.223	97.11	13Apr2010, 13:00	74.3
DET 4	2.223	17.49	14Apr2010, 03:00	54.2
E13	0.0965	7.7	13Apr2010, 11:28	3.5
COMB 6	2.3195	18.34	14Apr2010, 00:48	57.7
E14A	0.1706	18.14	13Apr2010, 11:52	8.9
COMB6A	0.1706	18.14	13Apr2010, 11:52	8.9
RR09A	0.1706	18.12	13Apr2010, 12:04	8.9
E15	0.0903	7.5	13Apr2010, 11:48	4
E22	0.0238	2.48	13Apr2010, 11:00	0.9
RR09B	0.0238	2.47	13Apr2010, 11:04	0.9
Junction-1	0.1141	9.09	13Apr2010, 11:36	4.9
RR10	0.1141	9.09	13Apr2010, 11:44	4.9
E17	0.0587	7.72	13Apr2010, 11:12	3
E16	0.0337	6.52	13Apr2010, 10:52	2.1
RR11A	0.0337	6.49	13Apr2010, 11:00	2.1
Junction-2	0.0924	14.1	13Apr2010, 11:04	5.1
RR 11B	0.0924	14.08	13Apr2010, 11:08	5.1
COMB 7	0.3771	36.43	13Apr2010, 11:36	19
RR 12	0.3771	36.42	13Apr2010, 11:40	19
E14	0.1537	18.38	13Apr2010, 11:28	7.6
DET ROCHA	0.1537	1	13Apr2010, 12:16	4.7
RR 09	0.1537	1	13Apr2010, 12:52	4.6

Eastside Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E18	0.0196	3.1	13Apr2010, 10:56	1
COMB 8	0.5504	38.73	13Apr2010, 11:36	24.6
RR 13	0.5504	38.72	13Apr2010, 11:40	24.6
E19	0.0554	6.61	13Apr2010, 11:12	2.6
COMB 9	0.6058	44.3	13Apr2010, 11:32	27.2
RR13A	0.6058	44.26	13Apr2010, 11:36	27.2
E20	0.0723	8.01	13Apr2010, 12:00	4.3
COMB 9A	0.6781	51.69	13Apr2010, 11:40	31.5
RR 14	0.6781	51.68	13Apr2010, 11:40	31.5
E21	0.0223	4.36	13Apr2010, 11:36	1.9
COMB10	0.7004	56.04	13Apr2010, 11:40	33.4
RR 15	0.7004	56.01	13Apr2010, 11:44	33.4
COMB11	3.0199	66.1	13Apr2010, 11:44	91.2
RR 16	3.0199	66.02	13Apr2010, 11:52	91
Junction-3	3.0199	66.02	13Apr2010, 11:52	91
RR 17	3.0199	65.7	13Apr2010, 12:04	90.8
Junction-4	3.0199	65.7	13Apr2010, 12:04	90.8
RR 18	3.0199	65.55	13Apr2010, 12:12	90.7
E23	0.2025	51.65	13Apr2010, 11:12	18.8
E24	0.1327	31.46	13Apr2010, 11:20	12.3
COMB13	3.3551	125.02	13Apr2010, 11:40	121.8
Reach-1	3.3551	124.57	13Apr2010, 11:44	121.7
E25	0.0393	34.33	13Apr2010, 10:40	113.7
COMB14	3.3944	148.82	13Apr2010, 11:44	235.4
RR 19	3.3944	148.56	13Apr2010, 11:48	235.4
E25B	0.0305	12	13Apr2010, 10:40	2.8
COMB15	3.4249	151.41	13Apr2010, 11:48	238.2
RR 20	3.4249	151.19	13Apr2010, 11:52	238.2
E27	0.1251	35.26	13Apr2010, 11:04	11.7
E26	0.0347	7.28	13Apr2010, 11:00	2.4
COMB16	3.5847	176.56	13Apr2010, 11:40	252.2
ROCPD5	3.5847	173.78	13Apr2010, 11:44	252.1
COMB17	3.5847	173.78	13Apr2010, 11:44	252.1
RR 21	3.5847	170.02	13Apr2010, 11:48	251.9
E28	0.0952	26.86	13Apr2010, 11:04	8.9
E45	0.024	8.89	13Apr2010, 10:40	2.2
Reach-4	0.024	8.68	13Apr2010, 11:04	2.2
COMB18	3.7039	192	13Apr2010, 11:28	263
RR 22	3.7039	182.16	13Apr2010, 11:52	262.6
E29	0.0892	24.1	13Apr2010, 10:52	7.3
CPD5.3	3.7931	193.12	13Apr2010, 11:44	269.8
RR 23	3.7931	193.04	13Apr2010, 11:44	269.8
E30A	0.0194	5.06	13Apr2010, 10:44	1.3
COMB19	3.8125	194.68	13Apr2010, 11:44	271.1
RR 24	3.8125	194.62	13Apr2010, 11:44	271.1
COMB20	3.8125	194.62	13Apr2010, 11:44	271.1

Eastside Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E37	0.0808	14.18	13Apr2010, 11:04	5
E36	0.0296	5.75	13Apr2010, 11:00	1.9
COMB25	0.1104	19.89	13Apr2010, 11:04	6.9
RR 29	0.1104	19.85	13Apr2010, 11:08	6.9
E38	0.0587	9.29	13Apr2010, 11:20	3.7
COMB26	0.1691	28.78	13Apr2010, 11:08	10.6
RR 30	0.1691	28.75	13Apr2010, 11:16	10.6
E39	0.0917	14.28	13Apr2010, 11:08	5.2
COMB27	0.2608	49.77	13Apr2010, 11:16	24.2
RR 31	0.2608	49.71	13Apr2010, 11:20	24.2
E40	0.1609	17.48	13Apr2010, 11:40	8.3
COMB28	0.4217	66.01	13Apr2010, 11:24	32.6
RR 32	0.4217	65.96	13Apr2010, 11:28	32.6
E42	0.1344	16.13	13Apr2010, 11:48	8
RR 33	0.1344	16.12	13Apr2010, 11:52	8
E43	0.081	9.96	13Apr2010, 11:48	5
COMB30	0.2154	26.09	13Apr2010, 11:52	13
RR 34	0.2154	26.08	13Apr2010, 11:52	13
E41	0.0426	5.14	13Apr2010, 11:24	2.2
COMB29	0.6797	95.09	13Apr2010, 11:32	47.8
RR 35	0.6797	94.97	13Apr2010, 11:52	47.8
E44	0.1082	13.74	13Apr2010, 11:24	5.8
COMB32	0.7879	106.35	13Apr2010, 11:52	53.6
RR 36	0.7879	106.34	13Apr2010, 11:52	53.6
E33	0.1983	25.9	13Apr2010, 12:36	16
E31	0.0889	14.75	13Apr2010, 10:44	4.3
E31A	0.068	6.58	13Apr2010, 11:28	3.1
COMB21	0.1569	18.61	13Apr2010, 10:48	7.3
DET1A	0.1569	2	13Apr2010, 00:00	7.3
RR 25	0.1569	2.29	13Apr2010, 00:12	7.4
E32	0.0331	4.46	13Apr2010, 11:12	1.7
RR 26	0.0331	4.46	13Apr2010, 11:16	1.7
COMB22	0.19	6.46	13Apr2010, 11:16	9.1
Reach-2	0.19	6.45	13Apr2010, 11:24	9.2
COMB23	0.3883	30.13	13Apr2010, 12:28	25.2
DIV 1	0.3883	20.09	13Apr2010, 12:28	16.8
RR 27	0.3883	20.08	13Apr2010, 12:44	16.8
E34	0.2671	30.97	13Apr2010, 12:28	18.6
COMB24	0.6554	50.88	13Apr2010, 12:32	35.4
E35	0.027	10.15	13Apr2010, 10:40	2.5
RR 28	0.027	10.09	13Apr2010, 10:44	2.5
COMOUT	1.4703	154.56	13Apr2010, 11:56	91.5
RR 37	1.4703	153.18	13Apr2010, 12:08	91.5
E46C	0.0488	8.45	13Apr2010, 11:08	3.1
E46B	0.0345	3.49	13Apr2010, 11:28	1.6
Junction-5	1.5536	160.53	13Apr2010, 12:04	96.2

Eastside Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RR 38	1.5536	159.43	13Apr2010, 12:12	96.2
E30	0.0575	18.78	13Apr2010, 10:52	5.4
E46A	0.0473	5.23	13Apr2010, 11:24	2.3
COMB33	5.4709	358.77	13Apr2010, 12:04	374.9
RR 39	5.4709	357.87	13Apr2010, 12:04	374.9
E48A	0.2383	31.04	13Apr2010, 11:32	13.7
E46	0.055	10.87	13Apr2010, 10:56	3.4
RR 40	0.055	10.86	13Apr2010, 11:00	3.4
E47	0.0473	5.56	13Apr2010, 11:16	2.2
COMB34	0.1023	16.06	13Apr2010, 11:04	5.7
RR 41	0.1023	16	13Apr2010, 11:08	5.7
COMB35	0.3406	45.56	13Apr2010, 11:20	19.4
RR 42	0.3406	45.55	13Apr2010, 11:24	19.4
E48	0.3004	31.72	13Apr2010, 12:08	17.6
COMB36	0.641	72.76	13Apr2010, 11:40	37
RR 43	0.641	72.76	13Apr2010, 11:40	37
LC1	0.3494	51.65	13Apr2010, 11:40	24
DETL1	0.3494	1	13Apr2010, 11:52	4.7
RRL1	0.3494	1	13Apr2010, 12:16	4.7
E49	0.0093	3.84	13Apr2010, 10:36	0.9
COMB37	0.9997	74.41	13Apr2010, 11:40	42.5
RR 44	0.9997	74.37	13Apr2010, 11:44	42.5
E51	0.1283	44.55	13Apr2010, 10:44	12
COMB38	1.128	92.12	13Apr2010, 11:28	54.5
RR 45	1.128	92.05	13Apr2010, 11:28	54.5
E50	0.0635	18.16	13Apr2010, 11:00	5.9
COMB39	1.1915	106.85	13Apr2010, 11:20	60.4
RR 46	1.1915	106.82	13Apr2010, 11:24	60.4
E52	0.1234	27.33	13Apr2010, 11:08	9.8
COMB40	1.3149	133.04	13Apr2010, 11:16	70.2
COMB41	6.7858	469.69	13Apr2010, 11:56	445
RR 47	6.7858	468.1	13Apr2010, 12:00	444.9
E53	0.2799	15.09	13Apr2010, 12:44	10.1
RR 48	0.2799	15.09	13Apr2010, 12:48	10.1
E54	0.1028	20.19	13Apr2010, 11:28	8.4
COMB42	0.3827	29.59	13Apr2010, 11:44	18.5
RR 49	0.3827	29.58	13Apr2010, 11:52	18.5
E55	0.2182	24.43	13Apr2010, 11:44	11.9
COMB43	0.6009	53.91	13Apr2010, 11:48	30.4
RR 50	0.6009	53.88	13Apr2010, 11:52	30.4
Junction-6	0.6009	53.88	13Apr2010, 11:52	30.4
RR 51	0.6009	53.87	13Apr2010, 11:56	30.4
E56	0.2697	68.05	13Apr2010, 11:12	25.2
RR 52	0.2697	67.98	13Apr2010, 11:16	25.2
E58	0.0913	15.68	13Apr2010, 11:12	5.9
E57	0.0434	8.3	13Apr2010, 11:00	2.8

Eastside Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RR E57	0.0434	8.3	13Apr2010, 11:04	2.8
COMB44	0.4044	91.23	13Apr2010, 11:16	33.8
RR 53	0.4044	91.09	13Apr2010, 11:16	33.8
E59	0.3806	68.1	13Apr2010, 11:36	30.7
COMB45	1.3859	202.6	13Apr2010, 11:32	94.9
RR 54	1.3859	201.26	13Apr2010, 11:40	94.9
E63	0.334	76.2	13Apr2010, 11:08	27.2
E60	0.0934	15.25	13Apr2010, 11:24	6.3
E61	0.0329	8.11	13Apr2010, 10:48	2.3
COMB46	0.1263	20.38	13Apr2010, 11:08	8.5
RR 55	0.1263	20.36	13Apr2010, 11:12	8.5
E62	0.1244	34.54	13Apr2010, 10:56	10.9
Junction-7	0.2507	54.24	13Apr2010, 11:00	19.4
RR 56	0.2507	54.17	13Apr2010, 11:04	19.4
COMB47	1.9706	306.79	13Apr2010, 11:28	141.5
RR 57	1.9706	306.5	13Apr2010, 11:28	141.5
E65	0.3099	42.81	13Apr2010, 11:52	21.3
DET 16	0.3099	0	13Apr2010, 00:00	0
Reach-3	0.3099	0	13Apr2010, 00:00	0
ADDE45	2.2805	306.5	13Apr2010, 11:28	141.5
DET NEI	2.2805	10	13Apr2010, 00:00	53.8
RR 58	2.2805	10.81	13Apr2010, 00:12	53.9
E66	0.1315	25.37	13Apr2010, 11:08	9
DET 15	0.1315	1	13Apr2010, 11:36	4.8
RRE66	0.1315	1	13Apr2010, 12:04	4.7
ADDE66	2.412	11	13Apr2010, 12:04	58.6
RR 59	2.412	11	13Apr2010, 12:20	58.6
E67	0.1612	42.25	13Apr2010, 10:44	11.1
DET 14	0.1612	1	13Apr2010, 11:08	4.8
ADDE67	2.5732	12	13Apr2010, 12:20	63.4
RR 60	2.5732	12	13Apr2010, 12:44	63.3
E68	0.0991	22.22	13Apr2010, 10:52	6.8
DET 13	0.0991	5.49	13Apr2010, 12:44	6.8
ADDE68	2.6723	17.49	13Apr2010, 12:44	70.1
RR 61	2.6723	17.49	13Apr2010, 12:56	70.1
E64	0.0912	22.93	13Apr2010, 10:44	6.3
DET 12	0.0912	5.12	13Apr2010, 12:28	6.3
COMB48	2.7635	22.6	13Apr2010, 12:48	76.4
CP26	9.5493	490.08	13Apr2010, 12:00	521.3

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET NEI

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	306.499 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:28
Peak Outflow :	10.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	141.50370 (AC-FT)	Peak Storage :	119.91096 (AC-FT)
Total Outflow :	53.82920 (AC-FT)	Peak Elevation :	17.44 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET ROCHA

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	18.377 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:28
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 12:16
Total Inflow :	7.62974 (AC-FT)	Peak Storage :	6.27597 (AC-FT)
Total Outflow :	4.66938 (AC-FT)	Peak Elevation :	86.98 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET1A
Start of Run: 13Apr2010, 00:00 Basin Model: Basin 1
End of Run: 15Apr2010, 18:36 Meteorologic Model: 10-yr
Compute Time: 01Nov2010, 12:46:17 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	18.606 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:48
Peak Outflow :	2.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	7.33968 (AC-FT)	Peak Storage :	3.65362 (AC-FT)
Total Outflow :	7.34435 (AC-FT)	Peak Elevation :	58.14 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 4
Start of Run: 13Apr2010, 00:00 Basin Model: Basin 1
End of Run: 15Apr2010, 18:36 Meteorologic Model: 10-yr
Compute Time: 01Nov2010, 12:46:17 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	97.109 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 13:00
Peak Outflow :	17.485 (CFS)	Date/Time of Peak Outflow :	14Apr2010, 03:00
Total Inflow :	74.30691 (AC-FT)	Peak Storage :	54.62433 (AC-FT)
Total Outflow :	54.24073 (AC-FT)	Peak Elevation :	49.69 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 12

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	22.927 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:44
Peak Outflow :	5.124 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 12:28
Total Inflow :	6.27478 (AC-FT)	Peak Storage :	2.37483 (AC-FT)
Total Outflow :	6.27437 (AC-FT)	Peak Elevation :	12.13 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 13

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	22.216 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:52
Peak Outflow :	5.495 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 12:44
Total Inflow :	6.81832 (AC-FT)	Peak Storage :	2.58017 (AC-FT)
Total Outflow :	6.81784 (AC-FT)	Peak Elevation :	12.29 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 14

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	42.254 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:44
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 11:08
Total Inflow :	11.09095 (AC-FT)	Peak Storage :	9.71859 (AC-FT)
Total Outflow :	4.78562 (AC-FT)	Peak Elevation :	12.02 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 15

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	25.366 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:08
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 11:36
Total Inflow :	9.04752 (AC-FT)	Peak Storage :	7.65509 (AC-FT)
Total Outflow :	4.75247 (AC-FT)	Peak Elevation :	11.99 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DET 16
Start of Run: 13Apr2010, 00:00 Basin Model: Basin 1
End of Run: 15Apr2010, 18:36 Meteorologic Model: 10-yr
Compute Time: 01Nov2010, 12:46:17 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	42.808 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:52
Peak Outflow :	0.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	21.32186 (AC-FT)	Peak Storage :	21.32186 (AC-FT)
Total Outflow :	0.00000 (AC-FT)	Peak Elevation :	17.10 (FT)

Project: tracy-eastside
Simulation Run: 10-yr Reservoir: DETLC1

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 12:46:17	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	51.646 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:40
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 11:52
Total Inflow :	24.03956 (AC-FT)	Peak Storage :	22.51578 (AC-FT)
Total Outflow :	4.74364 (AC-FT)	Peak Elevation :	5.52 (FT)

Eastside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E02	0.3059	30.57	13Apr2010, 12:48	20.5
E01	0.1997	11.53	13Apr2010, 13:20	8.7
RR 01	0.1997	11.49	13Apr2010, 14:16	8.7
E03	0.105	14	13Apr2010, 11:48	7
RR 02	0.105	13.99	13Apr2010, 11:56	7
COMB 1	0.6106	47.35	13Apr2010, 13:08	36.2
RR 03	0.6106	47.34	13Apr2010, 13:16	36.2
E05	0.2668	27.92	13Apr2010, 12:40	18.1
RR 04	0.2668	27.92	13Apr2010, 12:40	18.1
E04	0.2246	23.37	13Apr2010, 12:40	15.1
COMB 2	1.102	97.2	13Apr2010, 12:56	69.4
RR 05	1.102	97.07	13Apr2010, 13:08	69.4
E06	0.3652	31.88	13Apr2010, 12:48	21.1
E07	0.1382	16	13Apr2010, 11:48	8.2
RR 06	0.1382	15.99	13Apr2010, 11:52	8.2
COMB 3	1.6054	139.35	13Apr2010, 12:56	98.6
RR 07	1.6054	139.13	13Apr2010, 13:16	98.6
E08	0.2142	21.09	13Apr2010, 12:52	13.8
E09	0.1091	12.06	13Apr2010, 12:12	6.7
COMB 4	1.9287	169.38	13Apr2010, 13:08	119.1
RR 08	1.9287	169.34	13Apr2010, 13:08	119.1
E10	0.1538	22.73	13Apr2010, 11:36	10.1
E12	0.1405	20.32	13Apr2010, 12:04	10.6
COMB 5	2.223	197.55	13Apr2010, 12:56	139.8
DET 4	2.223	24.98	13Apr2010, 17:28	9
E13	0.0965	16.6	13Apr2010, 11:24	6.6
COMB 6	2.3195	28.55	13Apr2010, 17:28	15.6
E14A	0.1706	30.13	13Apr2010, 11:52	14.6
COMB6A	0.1706	30.13	13Apr2010, 11:52	14.6
RR09A	0.1706	30.11	13Apr2010, 12:00	14.6
E15	0.0903	13.54	13Apr2010, 11:52	6.8
E22	0.0238	4.76	13Apr2010, 11:00	1.7
RR09B	0.0238	4.75	13Apr2010, 11:04	1.7
Junction-1	0.1141	16.53	13Apr2010, 11:40	8.5
RR10	0.1141	16.53	13Apr2010, 11:44	8.5
E17	0.0587	13.26	13Apr2010, 11:12	5
E16	0.0337	10.6	13Apr2010, 10:56	3.3
RR11A	0.0337	10.56	13Apr2010, 11:00	3.3
Junction-2	0.0924	23.53	13Apr2010, 11:04	8.3
RR 11B	0.0924	23.51	13Apr2010, 11:08	8.3
COMB 7	0.3771	62.24	13Apr2010, 11:36	31.4
RR 12	0.3771	62.22	13Apr2010, 11:40	31.4
E14	0.1537	30.97	13Apr2010, 11:28	12.7
DET ROCHA	0.1537	1	13Apr2010, 11:20	4.8
RR 09	0.1537	1	13Apr2010, 11:52	4.7

Eastside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E18	0.0196	5.27	13Apr2010, 10:56	1.7
COMB 8	0.5504	66.23	13Apr2010, 11:36	37.8
RR 13	0.5504	66.19	13Apr2010, 11:40	37.8
E19	0.0554	11.97	13Apr2010, 11:12	4.5
COMB 9	0.6058	76.37	13Apr2010, 11:32	42.2
RR13A	0.6058	76.35	13Apr2010, 11:32	42.2
E20	0.0723	13.6	13Apr2010, 12:00	6.9
COMB 9A	0.6781	88.83	13Apr2010, 11:36	49.2
RR 14	0.6781	88.79	13Apr2010, 11:40	49.2
E21	0.0223	6.56	13Apr2010, 11:36	2.9
COMB10	0.7004	95.35	13Apr2010, 11:40	52.1
RR 15	0.7004	95.33	13Apr2010, 11:44	52.1
COMB11	3.0199	115.51	13Apr2010, 11:40	67.7
RR 16	3.0199	115.34	13Apr2010, 11:48	67.6
Junction-3	3.0199	115.34	13Apr2010, 11:48	67.6
RR 17	3.0199	114.79	13Apr2010, 12:00	67.6
Junction-4	3.0199	114.79	13Apr2010, 12:00	67.6
RR 18	3.0199	114.46	13Apr2010, 12:04	67.6
E23	0.2025	76.48	13Apr2010, 11:12	27.7
E24	0.1327	46.57	13Apr2010, 11:20	18.2
COMB13	3.3551	206.66	13Apr2010, 11:40	113.5
Reach-1	3.3551	205.81	13Apr2010, 11:44	113.4
E25	0.0393	41.2	13Apr2010, 10:40	115.4
COMB14	3.3944	232.22	13Apr2010, 11:40	228.9
RR 19	3.3944	231.91	13Apr2010, 11:44	228.9
E25B	0.0305	17.78	13Apr2010, 10:40	4.2
COMB15	3.4249	236.39	13Apr2010, 11:44	233.1
RR 20	3.4249	236.06	13Apr2010, 11:48	233.1
E27	0.1251	52.25	13Apr2010, 11:04	17.2
E26	0.0347	11.78	13Apr2010, 11:00	3.7
COMB16	3.5847	274.5	13Apr2010, 11:40	254
ROCPD5	3.5847	269.3	13Apr2010, 11:44	253.9
COMB17	3.5847	269.3	13Apr2010, 11:44	253.9
RR 21	3.5847	262.36	13Apr2010, 11:48	253.8
E28	0.0952	39.8	13Apr2010, 11:04	13.1
E45	0.024	13.17	13Apr2010, 10:40	3.3
Reach-4	0.024	12.84	13Apr2010, 11:00	3.3
COMB18	3.7039	293.29	13Apr2010, 11:36	270.2
RR 22	3.7039	278.11	13Apr2010, 11:48	269.8
E29	0.0892	37.03	13Apr2010, 10:52	11
CPD5.3	3.7931	294.89	13Apr2010, 11:44	280.8
RR 23	3.7931	294.63	13Apr2010, 11:44	280.8
E30A	0.0194	8.19	13Apr2010, 10:44	2.1
COMB19	3.8125	297.27	13Apr2010, 11:44	282.9
RR 24	3.8125	297.04	13Apr2010, 11:44	282.9
COMB20	3.8125	297.04	13Apr2010, 11:44	282.9

Eastside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
E37	0.0808	23.87	13Apr2010, 11:04	8
E36	0.0296	9.48	13Apr2010, 11:00	3.1
COMB25	0.1104	33.29	13Apr2010, 11:04	11.1
RR 29	0.1104	33.2	13Apr2010, 11:04	11.1
E38	0.0587	15.42	13Apr2010, 11:20	6
COMB26	0.1691	48.03	13Apr2010, 11:08	17
RR 30	0.1691	47.9	13Apr2010, 11:16	17
E39	0.0917	24.74	13Apr2010, 11:08	8.6
COMB27	0.2608	82.82	13Apr2010, 11:16	38.2
RR 31	0.2608	82.78	13Apr2010, 11:16	38.2
E40	0.1609	31.36	13Apr2010, 11:40	14
COMB28	0.4217	111.87	13Apr2010, 11:20	52.3
RR 32	0.4217	111.83	13Apr2010, 11:24	52.3
E42	0.1344	27.28	13Apr2010, 11:48	13.1
RR 33	0.1344	27.26	13Apr2010, 11:52	13.1
E43	0.081	16.7	13Apr2010, 11:48	8
COMB30	0.2154	43.95	13Apr2010, 11:52	21.1
RR 34	0.2154	43.95	13Apr2010, 11:52	21.1
E41	0.0426	9.24	13Apr2010, 11:24	3.7
COMB29	0.6797	161.04	13Apr2010, 11:32	77.1
RR 35	0.6797	160.88	13Apr2010, 11:48	77.1
E44	0.1082	24.41	13Apr2010, 11:20	9.6
COMB32	0.7879	181.76	13Apr2010, 11:48	86.7
RR 36	0.7879	181.74	13Apr2010, 11:48	86.7
E33	0.1983	39.73	13Apr2010, 12:36	24.3
E31	0.0889	26.95	13Apr2010, 10:44	7.2
E31A	0.068	11.41	13Apr2010, 11:32	5.2
COMB21	0.1569	33.03	13Apr2010, 10:48	12.4
DET1A	0.1569	2	13Apr2010, 00:00	10.7
RR 25	0.1569	2.49	13Apr2010, 01:12	10.8
E32	0.0331	8.05	13Apr2010, 11:08	2.9
RR 26	0.0331	8.04	13Apr2010, 11:16	2.9
COMB22	0.19	10.04	13Apr2010, 11:16	13.6
Reach-2	0.19	10.03	13Apr2010, 11:24	13.6
COMB23	0.3883	45.6	13Apr2010, 12:28	37.9
DIV 1	0.3883	30.4	13Apr2010, 12:28	25.3
RR 27	0.3883	30.38	13Apr2010, 12:40	25.3
E34	0.2671	49.61	13Apr2010, 12:28	29.1
COMB24	0.6554	79.8	13Apr2010, 12:32	54.4
E35	0.027	15.05	13Apr2010, 10:40	3.7
RR 28	0.027	14.95	13Apr2010, 10:44	3.7
COMOUT	1.4703	255.88	13Apr2010, 11:52	144.8
RR 37	1.4703	253.54	13Apr2010, 12:04	144.7
E46C	0.0488	14.05	13Apr2010, 11:08	5
E46B	0.0345	6.67	13Apr2010, 11:28	2.7
Junction-5	1.5536	267.09	13Apr2010, 12:00	152.4

Eastside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RR 38	1.5536	265.13	13Apr2010, 12:08	152.4
E30	0.0575	27.83	13Apr2010, 10:52	7.9
E46A	0.0473	9.74	13Apr2010, 11:24	3.9
COMB33	5.4709	573.92	13Apr2010, 11:56	447.1
RR 39	5.4709	572.36	13Apr2010, 12:00	447.1
E48A	0.2383	53.34	13Apr2010, 11:32	22.5
E46	0.055	18.24	13Apr2010, 10:56	5.5
RR 40	0.055	18.23	13Apr2010, 11:00	5.5
E47	0.0473	10.43	13Apr2010, 11:16	3.9
COMB34	0.1023	28	13Apr2010, 11:04	9.4
RR 41	0.1023	27.95	13Apr2010, 11:08	9.4
COMB35	0.3406	78.57	13Apr2010, 11:20	31.9
RR 42	0.3406	78.53	13Apr2010, 11:24	31.9
E48	0.3004	53.82	13Apr2010, 12:08	28.6
COMB36	0.641	124.24	13Apr2010, 11:36	60.5
RR 43	0.641	124.21	13Apr2010, 11:40	60.5
LC1	0.3494	83.3	13Apr2010, 11:40	37.7
DETL1	0.3494	1	13Apr2010, 11:08	4.8
RRL1	0.3494	1	13Apr2010, 11:36	4.8
E49	0.0093	5.69	13Apr2010, 10:36	1.3
COMB37	0.9997	126.57	13Apr2010, 11:40	66.6
RR 44	0.9997	126.55	13Apr2010, 11:44	66.6
E51	0.1283	66.02	13Apr2010, 10:44	17.7
COMB38	1.128	153.06	13Apr2010, 11:28	84.2
RR 45	1.128	152.96	13Apr2010, 11:32	84.2
E50	0.0635	26.92	13Apr2010, 11:00	8.7
COMB39	1.1915	174.44	13Apr2010, 11:20	93
RR 46	1.1915	174.42	13Apr2010, 11:24	92.9
E52	0.1234	42.3	13Apr2010, 11:08	14.9
COMB40	1.3149	214.41	13Apr2010, 11:20	107.8
COMB41	6.7858	759.24	13Apr2010, 11:52	554.9
RR 47	6.7858	756.35	13Apr2010, 11:56	554.8
E53	0.2799	31.87	13Apr2010, 12:36	19.2
RR 48	0.2799	31.87	13Apr2010, 12:40	19.2
E54	0.1028	30.99	13Apr2010, 11:28	12.7
COMB42	0.3827	53.13	13Apr2010, 11:52	31.8
RR 49	0.3827	53.1	13Apr2010, 11:56	31.8
E55	0.2182	42.81	13Apr2010, 11:44	19.8
COMB43	0.6009	95.35	13Apr2010, 11:52	51.7
RR 50	0.6009	95.33	13Apr2010, 11:56	51.7
Junction-6	0.6009	95.33	13Apr2010, 11:56	51.7
RR 51	0.6009	95.27	13Apr2010, 11:56	51.7
E56	0.2697	100.81	13Apr2010, 11:12	37.1
RR 52	0.2697	100.73	13Apr2010, 11:16	37.1
E58	0.0913	25.17	13Apr2010, 11:12	9.3
E57	0.0434	13.33	13Apr2010, 11:00	4.4

Eastside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RR E57	0.0434	13.31	13Apr2010, 11:04	4.4
COMB44	0.4044	138.09	13Apr2010, 11:16	50.7
RR 53	0.4044	137.98	13Apr2010, 11:16	50.7
E59	0.3806	104.54	13Apr2010, 11:36	46.4
COMB45	1.3859	319.47	13Apr2010, 11:32	148.8
RR 54	1.3859	317.43	13Apr2010, 11:40	148.8
E63	0.334	117.02	13Apr2010, 11:08	41.2
E60	0.0934	24.64	13Apr2010, 11:24	9.9
E61	0.0329	13.13	13Apr2010, 10:48	3.6
COMB46	0.1263	32.87	13Apr2010, 11:08	13.4
RR 55	0.1263	32.85	13Apr2010, 11:12	13.4
E62	0.1244	52.05	13Apr2010, 10:56	16.2
Junction-7	0.2507	83.86	13Apr2010, 11:00	29.6
RR 56	0.2507	83.73	13Apr2010, 11:04	29.6
COMB47	1.9706	480.5	13Apr2010, 11:28	219.6
RR 57	1.9706	480.19	13Apr2010, 11:28	219.6
E65	0.3099	68.99	13Apr2010, 11:52	33.5
DET 16	0.3099	0	13Apr2010, 00:00	0
Reach-3	0.3099	0	13Apr2010, 00:00	0
ADDE45	2.2805	480.19	13Apr2010, 11:28	219.6
DET NEI	2.2805	10	13Apr2010, 00:00	53.9
RR 58	2.2805	10.81	13Apr2010, 00:12	54
E66	0.1315	41.02	13Apr2010, 11:08	14.2
DET 15	0.1315	1	13Apr2010, 10:56	4.8
RRE66	0.1315	1	13Apr2010, 11:24	4.8
ADDE66	2.412	11	13Apr2010, 11:24	58.8
RR 59	2.412	11	13Apr2010, 11:40	58.8
E67	0.1612	68.41	13Apr2010, 10:44	17.4
DET 14	0.1612	1	13Apr2010, 10:36	4.9
ADDE67	2.5732	12	13Apr2010, 11:40	63.7
RR 60	2.5732	12	13Apr2010, 12:04	63.7
E68	0.0991	35.97	13Apr2010, 10:52	10.7
DET 13	0.0991	8.38	13Apr2010, 12:52	10.7
ADDE68	2.6723	20.38	13Apr2010, 12:52	74.4
RR 61	2.6723	20.38	13Apr2010, 13:04	74.3
E64	0.0912	37.14	13Apr2010, 10:44	9.8
DET 12	0.0912	7.81	13Apr2010, 12:36	9.8
COMB48	2.7635	28.17	13Apr2010, 12:52	84.2
CP26	9.5493	783.88	13Apr2010, 11:56	638.9

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET NEI

Start of Run: 13Apr2010, 00:00 Basin Model: Basin 1
End of Run: 15Apr2010, 18:36 Meteorologic Model: 100yr
Compute Time: 16Sep2010, 09:45:19 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	480.189 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:28
Peak Outflow :	10.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	219.59894 (AC-FT)	Peak Storage :	197.53056 (AC-FT)
Total Outflow :	53.93939 (AC-FT)	Peak Elevation :	20.32 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET ROCHA

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	30.968 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:28
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 11:20
Total Inflow :	12.67027 (AC-FT)	Peak Storage :	11.20819 (AC-FT)
Total Outflow :	4.75383 (AC-FT)	Peak Elevation :	88.39 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET1A

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	33.032 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:48
Peak Outflow :	2.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	12.38342 (AC-FT)	Peak Storage :	8.40270 (AC-FT)
Total Outflow :	10.73278 (AC-FT)	Peak Elevation :	59.63 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 4

Start of Run: 13Apr2010, 00:00 Basin Model: Basin 1
End of Run: 15Apr2010, 18:36 Meteorologic Model: 100yr
Compute Time: 16Sep2010, 09:45:19 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	197.551 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 12:56
Peak Outflow :	24.978 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 17:28
Total Inflow :	139.82955 (AC-FT)	Peak Storage :	130.83903 (AC-FT)
Total Outflow :	8.99052 (AC-FT)	Peak Elevation :	52.99 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 12

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	37.136 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:44
Peak Outflow :	7.813 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 12:36
Total Inflow :	9.84716 (AC-FT)	Peak Storage :	3.81884 (AC-FT)
Total Outflow :	9.84647 (AC-FT)	Peak Elevation :	13.26 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 13

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	35.969 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:52
Peak Outflow :	8.383 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 12:52
Total Inflow :	10.70014 (AC-FT)	Peak Storage :	4.15383 (AC-FT)
Total Outflow :	10.69933 (AC-FT)	Peak Elevation :	13.49 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 14

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	68.409 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 10:44
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 10:36
Total Inflow :	17.40528 (AC-FT)	Peak Storage :	15.94219 (AC-FT)
Total Outflow :	4.86699 (AC-FT)	Peak Elevation :	13.22 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 15

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	41.017 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:08
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 10:56
Total Inflow :	14.19848 (AC-FT)	Peak Storage :	12.70565 (AC-FT)
Total Outflow :	4.83525 (AC-FT)	Peak Elevation :	13.21 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DET 16

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	68.987 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:52
Peak Outflow :	0.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 00:00
Total Inflow :	33.46089 (AC-FT)	Peak Storage :	33.46089 (AC-FT)
Total Outflow :	0.00000 (AC-FT)	Peak Elevation :	18.25 (FT)

Project: tracy-eastside
Simulation Run: 100-yr Reservoir: DETLC1

Start of Run:	13Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	15Apr2010, 18:36	Meteorologic Model:	100yr
Compute Time:	16Sep2010, 09:45:19	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	83.302 (CFS)	Date/Time of Peak Inflow :	13Apr2010, 11:40
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 11:08
Total Inflow :	37.72584 (AC-FT)	Peak Storage :	36.08584 (AC-FT)
Total Outflow :	4.83178 (AC-FT)	Peak Elevation :	6.95 (FT)

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
W02	0.4975	22.82	12Apr2010, 13:24	18.3
RCP4	0.4975	22.82	12Apr2010, 13:28	18.3
W10	0.2163	24.66	12Apr2010, 11:20	10.5
RCP2	0.2163	24.64	12Apr2010, 11:28	10.5
W03	0.0243	4.28	12Apr2010, 11:00	1.5
RCP1	0.0243	4.26	12Apr2010, 11:08	1.5
COMB 1	0.2406	28.33	12Apr2010, 11:24	11.9
W09	0.209	40.87	12Apr2010, 11:20	16.2
COMB2	0.4496	69.11	12Apr2010, 11:20	28.1
W04	0.034	5.2	12Apr2010, 11:12	2
RCP3	0.034	5.19	12Apr2010, 11:24	2
COMB 3	0.4836	74.28	12Apr2010, 11:20	30.1
COMB 4	0.9811	84.39	12Apr2010, 11:24	48.4
DET2A	0.9811	2.85	13Apr2010, 04:00	10.4
CP1	0.9811	2.85	13Apr2010, 04:20	10.3
W01	0.2162	13.68	12Apr2010, 11:52	7.8
RCP5	0.2162	13.68	12Apr2010, 12:04	7.8
W08	0.1288	10.1	12Apr2010, 12:00	5.7
COMB 5	0.345	23.76	12Apr2010, 12:04	13.5
W05	0.0633	3.99	12Apr2010, 11:52	2.3
CP7	0.4083	27.72	12Apr2010, 12:00	15.7
W11	0.0812	14.74	12Apr2010, 10:56	4.8
W11A	0.0319	12.41	12Apr2010, 10:36	2.9
RR03	0.0319	12.34	12Apr2010, 10:40	2.9
Junction-1	0.1131	25.25	12Apr2010, 10:44	7.7
DET2B	0.5214	5	12Apr2010, 09:52	22.8
CP8	1.5025	7.85	13Apr2010, 04:20	33.2
RC1 1C	1.5025	7.85	13Apr2010, 04:52	32.9
W12	0.0362	3.44	12Apr2010, 11:20	1.5
CP5	1.5387	8.44	12Apr2010, 11:20	34.4
RCP6	1.5387	8.37	12Apr2010, 11:40	34.3
W21	0.2483	11.27	12Apr2010, 13:04	8
RPCP7	0.2483	11.27	12Apr2010, 13:12	8
W15	0.1238	6.38	12Apr2010, 12:32	4
W14	0.1224	5.59	12Apr2010, 12:12	3.3
Junction-2	0.4945	22.31	12Apr2010, 12:44	15.3
RCP8	0.4945	22.29	12Apr2010, 13:00	15.3
W16	0.1354	7.24	12Apr2010, 12:16	4.2
W17	0.1251	4.99	12Apr2010, 12:40	3.3
Junction-3	0.755	33.78	12Apr2010, 12:48	22.9
Reach-1	0.755	33.77	12Apr2010, 12:56	22.9
W18	0.0655	4.19	12Apr2010, 11:56	2.2
CP6	2.3592	43.93	12Apr2010, 12:44	59.4
RCP9	2.3592	43.9	12Apr2010, 13:00	59.2
W19	0.1877	6.47	12Apr2010, 13:12	5.1

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
W20	0.1292	8.35	12Apr2010, 12:04	5
CP9	2.6761	57.09	12Apr2010, 12:52	69.3
W23	0.2573	9.28	12Apr2010, 12:36	6.3
W22	0.2517	8.56	12Apr2010, 12:36	5.7
RPCP11	0.2517	8.55	12Apr2010, 12:44	5.7
CP11	0.509	17.81	12Apr2010, 12:40	12
RPCP9	0.509	17.8	12Apr2010, 12:48	12
Junction-5	3.1851	74.88	12Apr2010, 12:52	81.3
W25	0.0967	6.06	12Apr2010, 11:56	3.5
RCP12	0.0967	6.06	12Apr2010, 12:00	3.5
W26	0.0602	4.03	12Apr2010, 11:44	2.2
CP12	0.1569	10.02	12Apr2010, 11:52	5.6
Reach-2	0.1569	10.02	12Apr2010, 11:56	5.6
Junction-6	3.342	82.73	12Apr2010, 12:44	86.9
RCP13	3.342	82.68	12Apr2010, 12:52	86.8
W24	0.1291	9.13	12Apr2010, 11:52	4.8
CP13	3.4711	89.61	12Apr2010, 12:44	91.6
RCP17	3.4711	89.57	12Apr2010, 12:48	91.6
W27	0.1234	8.5	12Apr2010, 12:04	5
RCP14	0.1234	8.5	12Apr2010, 12:12	5
W28	0.0669	4.63	12Apr2010, 11:36	2.4
CP14	0.1903	12.66	12Apr2010, 12:00	7.4
RPCP15	0.1903	12.66	12Apr2010, 12:00	7.4
W29	0.0429	3.48	12Apr2010, 11:16	1.5
CP15	0.2332	15.16	12Apr2010, 11:52	8.9
RPCP16	0.2332	15.15	12Apr2010, 11:56	8.9
W29A	0.0166	3.27	12Apr2010, 10:48	1
C W29A	0.2498	16.42	12Apr2010, 11:48	9.9
RRW29A	0.2498	16.41	12Apr2010, 11:56	9.9
W30	0.0787	4.87	12Apr2010, 11:52	2.8
CP16	0.3285	21.27	12Apr2010, 11:56	12.7
RR W30	0.3285	21.26	12Apr2010, 11:56	12.7
W31	0.0342	3.22	12Apr2010, 11:04	1.2
CP17	0.3627	23.04	12Apr2010, 11:52	13.9
W34	0.1558	7.7	12Apr2010, 11:56	4.3
W33	0.0529	3.71	12Apr2010, 11:28	1.8
W47	0.042	3.89	12Apr2010, 11:00	1.5
RR W47	0.042	3.88	12Apr2010, 11:12	1.5
ADDW33	0.0949	7.42	12Apr2010, 11:16	3.3
RR W33	0.0949	7.4	12Apr2010, 11:44	3.3
W32	0.0916	7.69	12Apr2010, 11:04	3.2
RP17A	0.0916	7.67	12Apr2010, 11:12	3.2
Junction-7	4.1761	124.35	12Apr2010, 12:28	116.2
RCP18A	4.1761	124.24	12Apr2010, 12:36	116.1
W36	0.0459	2.55	12Apr2010, 11:36	1.2
CP18A	4.222	125.93	12Apr2010, 12:36	117.4

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RCP18	4.222	125.86	12Apr2010, 12:40	117.3
W35	0.0792	6.57	12Apr2010, 11:48	3.3
CP18	4.3012	130.95	12Apr2010, 12:36	120.7
RCP19	4.3012	130.76	12Apr2010, 12:48	120.5
W38	0.0984	5.01	12Apr2010, 12:12	3.2
W37	0.0778	4.66	12Apr2010, 11:28	2.1
RPCP19	0.0778	4.66	12Apr2010, 11:32	2.1
CP19	0.1762	9.25	12Apr2010, 11:48	5.2
RCP20	0.1762	9.18	12Apr2010, 11:56	5.2
CP20	4.4774	138.21	12Apr2010, 12:44	125.8
Reach-6	4.4774	138.1	12Apr2010, 12:52	125.7
W41	0.502	24.2	12Apr2010, 12:16	14.3
W40	0.1875	26.93	12Apr2010, 11:24	11.6
DET SL	0.1875	1	12Apr2010, 12:48	4.5
RPCP21	0.1875	1	12Apr2010, 13:28	4.4
CP21	0.6895	24.92	12Apr2010, 12:16	18.7
RCP22	0.6895	24.9	12Apr2010, 12:32	18.7
W41A	0.243	7.16	12Apr2010, 12:08	4.3
W41B	0.045	14.03	12Apr2010, 10:40	3.5
CP22	0.9775	34.36	12Apr2010, 12:24	26.5
RRCP3A	0.9775	34.36	12Apr2010, 12:24	26.5
DET 3A	0.9775	2.05	13Apr2010, 04:04	8.1
RCP26	0.9775	2.05	13Apr2010, 04:24	8
W49	0.4703	17.84	12Apr2010, 13:52	15.7
CP26	1.4478	18.95	12Apr2010, 13:56	23.7
RCP27	1.4478	18.95	12Apr2010, 14:00	23.7
W51	0.0546	2.94	12Apr2010, 11:32	1.5
CPW51	1.5024	20	12Apr2010, 13:56	25.2
RR W51	1.5024	19.99	12Apr2010, 13:56	25.2
W52	0.1002	8.68	12Apr2010, 11:32	4.1
J1	1.6026	23.4	12Apr2010, 13:40	29.3
RRW52	1.6026	23.39	12Apr2010, 13:44	29.3
W50	0.1974	15.07	12Apr2010, 12:00	10.2
W80	0.0927	9.21	12Apr2010, 11:24	5.8
RR 06	0.0927	9.2	12Apr2010, 11:32	5.8
W81	0.0477	3.38	12Apr2010, 11:24	1.6
RR 08	0.0477	3.38	12Apr2010, 11:24	1.6
CP 3B	0.3378	26.65	12Apr2010, 11:44	17.6
DET 3B	0.3378	0.46	14Apr2010, 15:56	1.8
RCP28	0.3378	0.46	14Apr2010, 15:56	1.8
W54	0.0535	4.37	12Apr2010, 11:32	2.1
CP28	1.9939	25.5	12Apr2010, 13:28	33.2
RR W54	1.9939	25.5	12Apr2010, 13:32	33.2
W39	0.2435	15.38	12Apr2010, 12:16	9.6
RPCP20	0.2435	15.38	12Apr2010, 12:20	9.6
W53	0.1574	9.5	12Apr2010, 11:56	5.5

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
DET5	6.8722	125.56	12Apr2010, 15:08	173.6
RWCR11	6.8722	125.54	12Apr2010, 15:16	173.4
W55	0.0833	5.54	12Apr2010, 11:48	3
W56	0.04	12.11	12Apr2010, 10:52	3.6
RPCP29	0.04	12.09	12Apr2010, 10:56	3.6
CP29	6.9955	129.17	12Apr2010, 15:08	180.1
RCP29	6.9955	129.16	12Apr2010, 15:12	180
W57	0.1038	8.22	12Apr2010, 11:52	4.4
W54A	0.085	5.69	12Apr2010, 11:44	3.1
CPBYRN	0.085	5.69	12Apr2010, 11:52	3.1
Junction-10	7.1843	134.34	12Apr2010, 14:56	187.5
RR 01	7.1843	134.34	12Apr2010, 15:04	187.4
W84	0.2047	14.61	12Apr2010, 11:40	7.6
W82	0.0918	6.27	12Apr2010, 11:48	3.4
RR 02	0.0918	6.27	12Apr2010, 11:56	3.4
W83	0.0388	6.03	12Apr2010, 11:00	2
Reach-3	0.0388	6.02	12Apr2010, 11:04	2
COBFD	0.3353	24.52	12Apr2010, 11:36	13
DET CP	0.3353	0	12Apr2010, 00:00	0
RR 04	0.3353	0	12Apr2010, 00:00	0
W85	0.1066	8.66	12Apr2010, 11:52	4.6
COMB	0.4419	8.66	12Apr2010, 11:52	4.6
CPBERG	7.6262	137.74	12Apr2010, 14:52	191.9
CP15MD	7.6262	137.74	12Apr2010, 15:00	191.8
W86	0.2557	21.11	12Apr2010, 12:36	16.3
CW86	7.8819	151.01	12Apr2010, 14:04	208.1
RP16MD	7.8819	151.01	12Apr2010, 14:08	208
W94	0.1343	46.06	12Apr2010, 10:44	12.3
CP1WMD	8.0162	158.88	12Apr2010, 12:20	220.3
W87	0.0716	16.22	12Apr2010, 10:52	4.8
ADD10	8.0878	163.44	12Apr2010, 12:16	225.1
RRW87	8.0878	163.4	12Apr2010, 12:20	225
W88	0.1572	25.69	12Apr2010, 11:20	10.4
ADD11	8.245	179.7	12Apr2010, 12:12	235.5
RRW88	8.245	179.66	12Apr2010, 12:16	235.4
W89	0.2343	39.3	12Apr2010, 11:20	15.7
ADDALL	8.4793	204.18	12Apr2010, 12:08	251.1
W93	0.6116	123.18	12Apr2010, 11:20	49.4
W90	0.25	34.36	12Apr2010, 11:40	15.8
RRW90	0.25	34.36	12Apr2010, 11:44	15.8
Junction-8	0.25	34.36	12Apr2010, 11:44	15.8
Reach-7	0.25	34.33	12Apr2010, 11:52	15.8
Junction-4	0.8616	152.73	12Apr2010, 11:28	65.2
W92	0.1656	8.53	12Apr2010, 11:56	4.8
W91	0.0835	14.72	12Apr2010, 11:12	5.6
ADD 12	0.2491	21.34	12Apr2010, 11:24	10.4

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
DET 11	9.59	10	12Apr2010, 04:28	50.3
W65	0.1453	10.17	12Apr2010, 11:56	5.7
W64	0.0507	4.33	12Apr2010, 11:16	1.8
W64A	0.0384	7.82	12Apr2010, 11:00	2.6
DET V	0.0384	2	12Apr2010, 00:00	2.6
W64B	0.016	3.7	12Apr2010, 10:48	1.1
ADDW64	0.1051	9.21	12Apr2010, 11:00	5.5
RCP1MD	0.1051	9.2	12Apr2010, 11:04	5.5
CP1MD	0.2504	17.58	12Apr2010, 11:36	11.3
RCP2MD	0.2504	17.58	12Apr2010, 11:36	11.3
W66	0.1391	6.55	12Apr2010, 12:52	4.8
CP2MD	0.3895	22.33	12Apr2010, 11:48	16.1
DET C	0.3895	11.91	12Apr2010, 15:00	11.4
RCP3MD	0.3895	11.91	12Apr2010, 15:04	11.4
W67A	0.0235	4.32	12Apr2010, 10:52	1.4
RW67A	0.0235	4.31	12Apr2010, 10:56	1.4
W67	0.0139	5.43	12Apr2010, 10:36	1.2
CP3MD	0.4269	13.19	12Apr2010, 14:40	14
Reach-4	0.4269	13.19	12Apr2010, 14:48	14
W68	0.1715	18.67	12Apr2010, 11:36	8.7
RCP4MD	0.1715	18.66	12Apr2010, 11:40	8.7
W69	0.0886	9.61	12Apr2010, 11:28	4.3
W70	0.0391	4.66	12Apr2010, 11:36	2.2
CP4MD	0.7261	37.32	12Apr2010, 11:28	29.1
RCP5MD	0.7261	37.31	12Apr2010, 11:32	29.1
W71	0.0563	5.5	12Apr2010, 11:48	2.7
DET LOWELL	0.7824	22.74	12Apr2010, 16:08	31.8
CP5MD	0.7824	22.74	12Apr2010, 16:08	31.8
RCP6MD	0.7824	22.74	12Apr2010, 16:12	31.8
W72	0.1073	9.16	12Apr2010, 11:36	4.5
CP6MD	0.8897	25.46	12Apr2010, 15:20	36.3
RCP7D	0.8897	25.46	12Apr2010, 15:20	36.3
W73	0.049	3.5	12Apr2010, 11:36	1.8
CP7MD	0.9387	26.61	12Apr2010, 15:00	38.1
RCP8MD	0.9387	26.61	12Apr2010, 15:00	38.1
W74	0.1336	17.98	12Apr2010, 11:44	8.5
CP8MD	1.0723	40.91	12Apr2010, 11:56	46.6
RCP9MD	1.0723	40.89	12Apr2010, 12:00	46.6
W68B	0.0916	9.22	12Apr2010, 11:56	4.8
W68A	0.0491	6.92	12Apr2010, 11:40	3.2
CW68A	0.1407	15.98	12Apr2010, 11:48	8
RRW68A	0.1407	15.97	12Apr2010, 11:48	8
W77	0.1044	18.39	12Apr2010, 11:12	6.9
CW77	0.2451	32	12Apr2010, 11:24	14.9
RRW77	0.2451	31.97	12Apr2010, 11:28	14.9
W76	0.0885	7.83	12Apr2010, 11:44	3.9

Westside Watershed 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
CP10MD	0.3336	39.52	12Apr2010, 11:28	18.8
Reach-5	0.3336	39.49	12Apr2010, 11:36	18.8
W78	0.1893	18.93	12Apr2010, 11:56	10
RR W78	0.1893	18.93	12Apr2010, 12:04	10
W75	0.1387	11.94	12Apr2010, 12:00	6.4
CP9MD	1.7339	108.88	12Apr2010, 11:52	81.8
RCP31	1.7339	108.86	12Apr2010, 11:52	81.8
W58	0.1195	10.55	12Apr2010, 11:36	5.2
RWCR13	0.1195	10.43	12Apr2010, 12:08	5.2
W59	0.0517	3.88	12Apr2010, 11:28	1.9
ADDW59	0.1712	13.5	12Apr2010, 12:00	7.1
RR W59	0.1712	13.4	12Apr2010, 12:16	7.1
W60	0.0737	9.55	12Apr2010, 11:24	4
W61	0.0731	7.93	12Apr2010, 11:40	3.8
CP31	0.318	27.4	12Apr2010, 11:56	14.9
CP31MD	2.0519	136.21	12Apr2010, 11:52	96.7
RP11MD	2.0519	136.13	12Apr2010, 11:56	96.7
W63	0.0265	9.51	12Apr2010, 10:44	2.4
W79	0.0595	14.51	12Apr2010, 11:00	4.7
CP11MD	2.1379	146.03	12Apr2010, 11:52	103.8
RP1WMD	2.1379	146.02	12Apr2010, 11:56	103.8
DIV2	2.1379	4.6	12Apr2010, 11:56	0.5
W43	0.761	14.05	12Apr2010, 14:24	13.9
OUT2	0.761	14.05	12Apr2010, 14:24	13.9
W46	0.1813	3.97	12Apr2010, 12:44	3.3
OUT4	0.1813	3.97	12Apr2010, 12:44	3.3
W45	0.0585	3.54	12Apr2010, 11:20	1.6
OUT1	0.0585	3.54	12Apr2010, 11:20	1.6
W44	0.0509	3.14	12Apr2010, 11:16	1.4
OUT3	0.0509	3.14	12Apr2010, 11:16	1.4

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET V

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 11:58:31	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	7.815 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:00
Peak Outflow :	2.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 00:00
Total Inflow :	2.57484 (AC-FT)	Peak Storage :	0.57479 (AC-FT)
Total Outflow :	2.62810 (AC-FT)	Peak Elevation :	53.17 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET LOWELL

Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: 10-yr
Compute Time: 08Aug2011, 14:57:38 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	42.6 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:32
Peak Outflow :	22.7 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 16:08
Total Inflow :	31.9 (AC-FT)	Peak Storage :	8.6 (AC-FT)
Total Outflow :	31.8 (AC-FT)	Peak Elevation :	28.9 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET2A

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 11:58:31	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	84.386 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:24
Peak Outflow :	2.855 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 04:00
Total Inflow :	48.40536 (AC-FT)	Peak Storage :	45.21435 (AC-FT)
Total Outflow :	10.39237 (AC-FT)	Peak Elevation :	97.64 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET2B
Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: 10-yr
Compute Time: 01Nov2010, 11:58:31 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	37.552 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:20
Peak Outflow :	5.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 09:52
Total Inflow :	23.44309 (AC-FT)	Peak Storage :	14.98318 (AC-FT)
Total Outflow :	22.84554 (AC-FT)	Peak Elevation :	85.07 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET SL

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 11:58:31	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	26.931 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:24
Peak Outflow :	1.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 12:48
Total Inflow :	11.59687 (AC-FT)	Peak Storage :	10.20744 (AC-FT)
Total Outflow :	4.47065 (AC-FT)	Peak Elevation :	111.64 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET 3A

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	02Dec2010, 17:05:14	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	34.357 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 12:24
Peak Outflow :	2.049 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 04:04
Total Inflow :	26.48 (AC-FT)	Peak Storage :	21.04 (AC-FT)
Total Outflow :	8.07 (AC-FT)	Peak Elevation :	99.05 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET 3B

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	01Nov2010, 11:58:31	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	26.651 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:44
Peak Outflow :	0.465 (CFS)	Date/Time of Peak Outflow :	14Apr2010, 15:56
Total Inflow :	17.57457 (AC-FT)	Peak Storage :	15.73698 (AC-FT)
Total Outflow :	1.83759 (AC-FT)	Peak Elevation :	38.46 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET5

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	10-yr
Compute Time:	02Dec2010, 17:05:14	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	185.214 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 12:44
Peak Outflow :	125.562 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 15:08
Total Inflow :	173.96 (AC-FT)	Peak Storage :	19.07 (AC-FT)
Total Outflow :	173.55 (AC-FT)	Peak Elevation :	38.15 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET 11

Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: 10-yr
Compute Time: 13Dec2011, 17:00:16 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	372.60 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:28
Peak Outflow :	10.00 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 04:28
Total Inflow :	326.70 (AC-FT)	Peak Storage :	276.55 (AC-FT)
Total Outflow :	50.29 (AC-FT)	Peak Elevation :	7.12 (FT)

Project: tracy_westside
Simulation Run: 10-yr 24-hr Reservoir: DET CP
Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: 10-yr
Compute Time: 01Nov2010, 11:58:31 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	24.520 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:36
Peak Outflow :	0.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 00:00
Total Inflow :	12.99011 (AC-FT)	Peak Storage :	12.99007 (AC-FT)
Total Outflow :	0.00000 (AC-FT)	Peak Elevation :	44.89 (FT)

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
W02	0.4975	42.87	12Apr2010, 13:28	32.6
RCP4	0.4975	42.87	12Apr2010, 13:32	32.6
W10	0.2163	44.45	12Apr2010, 11:20	17.7
RCP2	0.2163	44.37	12Apr2010, 11:28	17.7
W03	0.0243	6.6	12Apr2010, 11:00	2.3
RCP1	0.0243	6.59	12Apr2010, 11:08	2.3
COMB 1	0.2406	50.22	12Apr2010, 11:24	20
W09	0.209	62.41	12Apr2010, 11:20	24.5
COMB2	0.4496	112.5	12Apr2010, 11:20	44.5
W04	0.034	7.7	12Apr2010, 11:16	3
RCP3	0.034	7.7	12Apr2010, 11:24	3
COMB 3	0.4836	120.16	12Apr2010, 11:20	47.5
COMB 4	0.9811	137.05	12Apr2010, 11:24	80.1
DET2A	0.9811	5.29	13Apr2010, 03:52	19.4
CP1	0.9811	5.29	13Apr2010, 04:12	19.3
W01	0.2162	26.51	12Apr2010, 11:56	14
RCP5	0.2162	26.5	12Apr2010, 12:04	14
W08	0.1288	17.85	12Apr2010, 12:04	9.7
COMB 5	0.345	44.35	12Apr2010, 12:04	23.6
W05	0.0633	7.73	12Apr2010, 11:56	4.1
CP7	0.4083	52	12Apr2010, 12:04	27.7
W11	0.0812	23.98	12Apr2010, 10:56	7.6
W11A	0.0319	18.33	12Apr2010, 10:36	4.2
RR03	0.0319	18.23	12Apr2010, 10:40	4.2
Junction-1	0.1131	39.12	12Apr2010, 10:48	11.9
DET2B	0.5214	5	12Apr2010, 07:56	24.5
CP8	1.5025	10.29	13Apr2010, 04:12	43.8
RC1 1C	1.5025	10.29	13Apr2010, 04:40	43.3
W12	0.0362	6.22	12Apr2010, 11:24	2.6
CP5	1.5387	11.22	12Apr2010, 11:24	45.9
RCP6	1.5387	11.09	12Apr2010, 11:40	45.6
W21	0.2483	17.32	12Apr2010, 13:12	13.4
RPCP7	0.2483	17.32	12Apr2010, 13:20	13.4
W15	0.1238	9.72	12Apr2010, 12:36	6.7
W14	0.1224	8.84	12Apr2010, 12:20	5.8
Junction-2	0.4945	34.61	12Apr2010, 12:48	25.8
RCP8	0.4945	34.58	12Apr2010, 13:08	25.8
W16	0.1354	11.01	12Apr2010, 12:20	7.1
W17	0.1251	7.77	12Apr2010, 12:48	5.8
Junction-3	0.755	52.39	12Apr2010, 12:56	38.7
Reach-1	0.755	52.38	12Apr2010, 13:00	38.7
W18	0.0655	6.38	12Apr2010, 12:00	3.7
CP6	2.3592	66.48	12Apr2010, 12:52	88
RCP9	2.3592	66.43	12Apr2010, 13:04	87.7
W19	0.1877	10.93	12Apr2010, 13:28	9

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
W20	0.1292	15.39	12Apr2010, 12:08	8.7
CP9	2.6761	89.39	12Apr2010, 12:56	105.4
W23	0.2573	15.17	12Apr2010, 12:48	11.3
W22	0.2517	13.6	12Apr2010, 12:48	10.3
RPCP11	0.2517	13.6	12Apr2010, 12:56	10.3
CP11	0.509	28.74	12Apr2010, 12:52	21.7
RPCP9	0.509	28.73	12Apr2010, 13:00	21.7
Junction-5	3.1851	118.12	12Apr2010, 12:56	127.1
W25	0.0967	11.75	12Apr2010, 11:56	6.2
RCP12	0.0967	11.74	12Apr2010, 12:00	6.2
W26	0.0602	7.83	12Apr2010, 11:44	3.9
CP12	0.1569	19.43	12Apr2010, 11:52	10.1
Reach-2	0.1569	19.42	12Apr2010, 11:56	10.1
Junction-6	3.342	132.81	12Apr2010, 12:44	137.2
RCP13	3.342	132.76	12Apr2010, 12:52	137.1
W24	0.1291	14.06	12Apr2010, 11:56	7.9
CP13	3.4711	143.96	12Apr2010, 12:44	145
RCP17	3.4711	143.88	12Apr2010, 12:48	144.9
W27	0.1234	15.63	12Apr2010, 12:04	8.6
RCP14	0.1234	15.63	12Apr2010, 12:12	8.6
W28	0.0669	9.06	12Apr2010, 11:40	4.3
CP14	0.1903	23.74	12Apr2010, 11:56	13
RPCP15	0.1903	23.74	12Apr2010, 12:00	13
W29	0.0429	6.84	12Apr2010, 11:16	2.8
CP15	0.2332	28.64	12Apr2010, 11:52	15.7
RPCP16	0.2332	28.62	12Apr2010, 11:56	15.7
W29A	0.0166	5.36	12Apr2010, 10:52	1.6
C W29A	0.2498	30.7	12Apr2010, 11:48	17.3
RRW29A	0.2498	30.7	12Apr2010, 11:56	17.3
W30	0.0787	9.04	12Apr2010, 11:56	4.9
CP16	0.3285	39.74	12Apr2010, 11:56	22.2
RR W30	0.3285	39.71	12Apr2010, 12:00	22.2
W31	0.0342	5.08	12Apr2010, 11:08	2
CP17	0.3627	42.82	12Apr2010, 11:52	24.2
W34	0.1558	12.52	12Apr2010, 12:04	7.7
W33	0.0529	7.18	12Apr2010, 11:28	3.3
W47	0.042	7.73	12Apr2010, 11:00	2.7
RR W47	0.042	7.7	12Apr2010, 11:08	2.7
ADDW33	0.0949	14.41	12Apr2010, 11:16	6
RR W33	0.0949	14.39	12Apr2010, 11:40	6
W32	0.0916	15.02	12Apr2010, 11:04	5.7
RP17A	0.0916	15.01	12Apr2010, 11:12	5.7
Junction-7	4.1761	208.42	12Apr2010, 12:24	188.4
RCP18A	4.1761	208.24	12Apr2010, 12:32	188.2
W36	0.0459	3.89	12Apr2010, 11:40	2.1
CP18A	4.222	211.2	12Apr2010, 12:32	190.4

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
RCP18	4.222	211.07	12Apr2010, 12:36	190.3
W35	0.0792	10.1	12Apr2010, 11:52	5.4
CP18	4.3012	219.45	12Apr2010, 12:32	195.7
RCP19	4.3012	219.17	12Apr2010, 12:44	195.4
W38	0.0984	9.35	12Apr2010, 12:16	5.7
W37	0.0778	7.04	12Apr2010, 11:32	3.6
RPCP19	0.0778	7.04	12Apr2010, 11:36	3.6
CP19	0.1762	15.57	12Apr2010, 11:56	9.3
RCP20	0.1762	15.48	12Apr2010, 12:04	9.3
CP20	4.4774	232.89	12Apr2010, 12:44	204.7
Reach-6	4.4774	232.69	12Apr2010, 12:48	204.6
W41	0.502	37.09	12Apr2010, 12:20	24.4
W40	0.1875	42.67	12Apr2010, 11:28	18.1
DET SL	0.1875	1	12Apr2010, 11:32	4.6
RPCP21	0.1875	1	14Apr2010, 15:56	4.5
CP21	0.6895	38.09	12Apr2010, 12:20	28.9
RCP22	0.6895	38.06	12Apr2010, 12:36	28.9
W41A	0.243	11.51	12Apr2010, 12:20	8.3
W41B	0.045	20.57	12Apr2010, 10:40	5.2
CP22	0.9775	53.2	12Apr2010, 12:28	42.4
RRCP3A	0.9775	53.2	12Apr2010, 12:28	42.4
DET 3A	0.9775	3	12Apr2010, 21:40	12.5
RCP26	0.9775	3	12Apr2010, 22:04	12.4
W49	0.4703	33.31	12Apr2010, 14:00	28.3
CP26	1.4478	35.03	12Apr2010, 14:04	40.7
RCP27	1.4478	35.02	12Apr2010, 14:08	40.7
W51	0.0546	4.92	12Apr2010, 11:40	2.6
CPW51	1.5024	37.14	12Apr2010, 14:00	43.3
RR W51	1.5024	37.14	12Apr2010, 14:04	43.3
W52	0.1002	16.53	12Apr2010, 11:32	7.2
J1	1.6026	43.14	12Apr2010, 13:48	50.5
RRW52	1.6026	43.14	12Apr2010, 13:52	50.5
W50	0.1974	28.09	12Apr2010, 12:00	16.4
W80	0.0927	17.66	12Apr2010, 11:24	8.8
RR 06	0.0927	17.61	12Apr2010, 11:32	8.8
W81	0.0477	7.19	12Apr2010, 11:20	3
RR 08	0.0477	7.18	12Apr2010, 11:24	3
CP 3B	0.3378	50.62	12Apr2010, 11:40	28.2
DET 3B	0.3378	0.75	13Apr2010, 06:12	3
RCP28	0.3378	0.75	13Apr2010, 06:24	3
W54	0.0535	8.62	12Apr2010, 11:32	3.8
CP28	1.9939	46.88	12Apr2010, 13:40	57.3
RR W54	1.9939	46.87	12Apr2010, 13:44	57.3
W39	0.2435	26.65	12Apr2010, 12:20	16.4
RPCP20	0.2435	26.64	12Apr2010, 12:24	16.4
W53	0.1574	18.59	12Apr2010, 12:00	10

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
DET5	6.8722	196.49	12Apr2010, 15:44	287.5
RWCR11	6.8722	196.47	12Apr2010, 15:48	287.4
W55	0.0833	10.78	12Apr2010, 11:48	5.4
W56	0.04	17.9	12Apr2010, 10:52	5.3
RPCP29	0.04	17.88	12Apr2010, 10:56	5.3
CP29	6.9955	202.1	12Apr2010, 15:44	298.1
RCP29	6.9955	202.1	12Apr2010, 15:44	298
W57	0.1038	15.29	12Apr2010, 11:52	7.6
W54A	0.085	11.07	12Apr2010, 11:44	5.5
CPBYRN	0.085	11.06	12Apr2010, 11:52	5.5
Junction-10	7.1843	210.59	12Apr2010, 15:28	311.2
RR 01	7.1843	210.59	12Apr2010, 15:32	311
W84	0.2047	29.43	12Apr2010, 11:40	13.8
W82	0.0918	12.52	12Apr2010, 11:44	6.1
RR 02	0.0918	12.51	12Apr2010, 11:56	6.1
W83	0.0388	10.69	12Apr2010, 11:00	3.4
Reach-3	0.0388	10.68	12Apr2010, 11:04	3.4
COBFD	0.3353	48.25	12Apr2010, 11:40	23.3
DET CP	0.3353	8.54	12Apr2010, 14:44	8.4
RR 04	0.3353	8.54	12Apr2010, 14:56	8.4
W85	0.1066	16.67	12Apr2010, 11:52	8.1
COMB	0.4419	16.67	12Apr2010, 11:52	16.5
CPBERG	7.6262	224.63	12Apr2010, 15:20	327.5
CP15MD	7.6262	224.62	12Apr2010, 15:28	327.2
W86	0.2557	36.89	12Apr2010, 12:36	25.1
CW86	7.8819	244.54	12Apr2010, 14:56	352.4
RP16MD	7.8819	244.53	12Apr2010, 15:00	352.3
W94	0.1343	68.23	12Apr2010, 10:44	18.1
CP1WMD	8.0162	252.52	12Apr2010, 15:00	370.4
W87	0.0716	26.25	12Apr2010, 10:52	7.5
ADD10	8.0878	256.27	12Apr2010, 15:00	377.9
RRW87	8.0878	256.19	12Apr2010, 15:00	377.8
W88	0.1572	41.63	12Apr2010, 11:20	16.4
ADD11	8.245	265.44	12Apr2010, 15:00	394.2
RRW88	8.245	265.41	12Apr2010, 15:04	394.1
W89	0.2343	63.45	12Apr2010, 11:20	24.6
ADDALL	8.4793	321.16	12Apr2010, 11:32	418.7
W93	0.6116	187.93	12Apr2010, 11:20	74.4
W90	0.25	56.45	12Apr2010, 11:36	25.2
RRW90	0.25	56.42	12Apr2010, 11:44	25.2
Junction-8	0.25	56.42	12Apr2010, 11:44	25.2
Reach-7	0.25	56.39	12Apr2010, 11:52	25.2
Junction-4	0.8616	237.36	12Apr2010, 11:28	99.6
W92	0.1656	20.39	12Apr2010, 11:48	9.8
W91	0.0835	23.78	12Apr2010, 11:12	8.8
ADD 12	0.2491	41.16	12Apr2010, 11:28	18.5

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
DET 11	9.59	10	12Apr2010, 03:44	50.6
W65	0.1453	18.66	12Apr2010, 11:56	10
W64	0.0507	6.74	12Apr2010, 11:20	3
W64A	0.0384	12.64	12Apr2010, 11:00	4
DET V	0.0384	2	12Apr2010, 00:00	4
W64B	0.016	5.76	12Apr2010, 10:48	1.6
ADDW64	0.1051	13.07	12Apr2010, 11:00	8.7
RCP1MD	0.1051	13.06	12Apr2010, 11:04	8.8
CP1MD	0.2504	28.9	12Apr2010, 11:40	18.7
RCP2MD	0.2504	28.88	12Apr2010, 11:40	18.8
W66	0.1391	12.43	12Apr2010, 12:56	8.7
CP2MD	0.3895	38.13	12Apr2010, 12:00	27.4
DET C	0.3895	12.47	12Apr2010, 12:00	17.3
RCP3MD	0.3895	12.47	12Apr2010, 12:04	17.3
W67A	0.0235	7.39	12Apr2010, 10:52	2.2
RW67A	0.0235	7.36	12Apr2010, 10:56	2.2
W67	0.0139	8.08	12Apr2010, 10:36	1.8
CP3MD	0.4269	16.93	12Apr2010, 11:56	21.4
Reach-4	0.4269	16.92	12Apr2010, 12:04	21.4
W68	0.1715	33.35	12Apr2010, 11:36	14.7
RCP4MD	0.1715	33.34	12Apr2010, 11:40	14.7
W69	0.0886	17.63	12Apr2010, 11:28	7.3
W70	0.0391	8.05	12Apr2010, 11:36	3.6
CP4MD	0.7261	74.47	12Apr2010, 11:36	46.9
RCP5MD	0.7261	74.41	12Apr2010, 11:40	46.9
W71	0.0563	9.99	12Apr2010, 11:44	4.7
DET LOWELL	0.7824	33.4	12Apr2010, 15:16	51.5
CP5MD	0.7824	33.4	12Apr2010, 15:16	51.5
RCP6MD	0.7824	33.4	12Apr2010, 15:20	51.5
W72	0.1073	17.86	12Apr2010, 11:36	7.9
CP6MD	0.8897	41.41	12Apr2010, 12:00	59.4
RCP7D	0.8897	41.37	12Apr2010, 12:04	59.4
W73	0.049	6.84	12Apr2010, 11:36	3.2
CP7MD	0.9387	47.54	12Apr2010, 12:00	62.6
RCP8MD	0.9387	47.46	12Apr2010, 12:04	62.6
W74	0.1336	29.29	12Apr2010, 11:44	13.5
CP8MD	1.0723	75.62	12Apr2010, 11:56	76.1
RCP9MD	1.0723	75.55	12Apr2010, 11:56	76.1
W68B	0.0916	16.22	12Apr2010, 11:52	8
W68A	0.0491	11.28	12Apr2010, 11:36	5
CW68A	0.1407	27.26	12Apr2010, 11:48	13
RRW68A	0.1407	27.24	12Apr2010, 11:48	13
W77	0.1044	29.81	12Apr2010, 11:12	10.9
CW77	0.2451	53.1	12Apr2010, 11:24	23.9
RRW77	0.2451	53.05	12Apr2010, 11:28	23.9
W76	0.0885	14.91	12Apr2010, 11:40	6.8

Westside Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
CP10MD	0.3336	67.53	12Apr2010, 11:28	30.7
Reach-5	0.3336	67.5	12Apr2010, 11:32	30.7
W78	0.1893	32.85	12Apr2010, 11:56	16.5
RR W78	0.1893	32.83	12Apr2010, 12:04	16.5
W75	0.1387	22.11	12Apr2010, 11:56	11.1
CP9MD	1.7339	194.03	12Apr2010, 11:52	134.4
RCP31	1.7339	194.03	12Apr2010, 11:52	134.4
W58	0.1195	19.34	12Apr2010, 11:40	9
RWCR13	0.1195	19.1	12Apr2010, 12:04	9
W59	0.0517	7.61	12Apr2010, 11:28	3.3
ADDW59	0.1712	25.39	12Apr2010, 11:56	12.3
RR W59	0.1712	25.17	12Apr2010, 12:12	12.3
W60	0.0737	16.47	12Apr2010, 11:24	6.6
W61	0.0731	14.07	12Apr2010, 11:40	6.3
CP31	0.318	50.85	12Apr2010, 11:52	25.2
CP31MD	2.0519	244.88	12Apr2010, 11:52	159.7
RP11MD	2.0519	244.73	12Apr2010, 11:52	159.7
W63	0.0265	14.09	12Apr2010, 10:44	3.6
W79	0.0595	22.27	12Apr2010, 11:00	7.2
CP11MD	2.1379	259.99	12Apr2010, 11:48	170.4
RP1WMD	2.1379	259.88	12Apr2010, 11:56	170.4
DIV2	2.1379	114.88	12Apr2010, 11:56	15.8
W43	0.761	39.73	12Apr2010, 13:56	32.4
OUT2	0.761	39.73	12Apr2010, 13:56	32.4
W46	0.1813	12.12	12Apr2010, 12:28	7.7
OUT4	0.1813	12.12	12Apr2010, 12:28	7.7
W45	0.0585	8.97	12Apr2010, 11:16	3.3
OUT1	0.0585	8.97	12Apr2010, 11:16	3.3
W44	0.0509	7.97	12Apr2010, 11:12	2.9
OUT3	0.0509	7.97	12Apr2010, 11:12	2.9

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET V

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	01Nov2010, 10:02:40	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	12.640 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:00
Peak Outflow :	2.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 00:00
Total Inflow :	4.03818 (AC-FT)	Peak Storage :	1.33163 (AC-FT)
Total Outflow :	4.04959 (AC-FT)	Peak Elevation :	53.96 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET LOWELL

Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: Met 1
Compute Time: 08Aug2011, 15:37:59 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	84.4 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:40
Peak Outflow :	33.4 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 15:16
Total Inflow :	51.5 (AC-FT)	Peak Storage :	14.2 (AC-FT)
Total Outflow :	51.5 (AC-FT)	Peak Elevation :	31.0 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET2A

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	01Nov2010, 10:02:40	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	137.046 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:24
Peak Outflow :	5.285 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 03:52
Total Inflow :	80.10784 (AC-FT)	Peak Storage :	74.02771 (AC-FT)
Total Outflow :	19.44946 (AC-FT)	Peak Elevation :	103.71 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET2B

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	01Nov2010, 10:02:40	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	66.900 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:44
Peak Outflow :	5.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 07:56
Total Inflow :	39.56902 (AC-FT)	Peak Storage :	30.33367 (AC-FT)
Total Outflow :	24.45329 (AC-FT)	Peak Elevation :	93.45 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET SL

Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: Met 1
Compute Time: 06Oct2010, 15:43:42 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	42.67 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:28
Peak Outflow :	1.00 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 11:32
Total Inflow :	18.14 (AC-FT)	Peak Storage :	16.64 (AC-FT)
Total Outflow :	4.56 (AC-FT)	Peak Elevation :	112.63 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET 3A

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	02Dec2010, 13:57:58	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	53.197 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 12:28
Peak Outflow :	3.000 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 21:40
Total Inflow :	42.41 (AC-FT)	Peak Storage :	35.58 (AC-FT)
Total Outflow :	12.53 (AC-FT)	Peak Elevation :	100.40 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET 3B

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	01Nov2010, 10:02:40	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	50.615 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:40
Peak Outflow :	0.746 (CFS)	Date/Time of Peak Outflow :	13Apr2010, 06:12
Total Inflow :	28.21 (AC-FT)	Peak Storage :	25.24 (AC-FT)
Total Outflow :	3.04 (AC-FT)	Peak Elevation :	38.75 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET5

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	02Dec2010, 13:57:58	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	318.964 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 12:44
Peak Outflow :	196.490 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 15:44
Total Inflow :	288.24 (AC-FT)	Peak Storage :	44.68 (AC-FT)
Total Outflow :	287.53 (AC-FT)	Peak Elevation :	39.76 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET 11

Start of Run:	12Apr2010, 00:00	Basin Model:	Basin 1
End of Run:	14Apr2010, 15:56	Meteorologic Model:	Met 1
Compute Time:	13Dec2011, 16:48:42	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	598.98 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:28
Peak Outflow :	10.00 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 03:44
Total Inflow :	536.82 (AC-FT)	Peak Storage :	486.18 (AC-FT)
Total Outflow :	50.65 (AC-FT)	Peak Elevation :	10.47 (FT)

Project: tracy_westside
Simulation Run: 100-yr 24-hr Reservoir: DET CP
Start of Run: 12Apr2010, 00:00 Basin Model: Basin 1
End of Run: 14Apr2010, 15:56 Meteorologic Model: Met 1
Compute Time: 01Nov2010, 10:02:40 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	48.252 (CFS)	Date/Time of Peak Inflow :	12Apr2010, 11:40
Peak Outflow :	8.538 (CFS)	Date/Time of Peak Outflow :	12Apr2010, 14:44
Total Inflow :	23.28460 (AC-FT)	Peak Storage :	13.46454 (AC-FT)
Total Outflow :	8.37882 (AC-FT)	Peak Elevation :	45.00 (FT)

Lammers Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
OFS2	4.516	115.59	01Jan2008, 11:44	79.24
DET-OFF2	4.516	2.34	02Jan2008, 02:56	9.4
L15a	0.26	76.93	01Jan2008, 10:56	23.71
DET L15a	0.26	8	01Jan2008, 08:36	23.71
L15b	0.138	44.42	01Jan2008, 10:48	12.6
L15d	0.064	22.2	01Jan2008, 10:44	5.84
DET L15b	4.978	62.15	01Jan2008, 11:04	51.54
L15c	0.025	9.55	01Jan2008, 10:40	2.28
CPL15	5.003	67.67	01Jan2008, 11:00	53.82
RR14	5.003	67.63	01Jan2008, 11:20	53.63
L16B	0.139	25.96	01Jan2008, 11:08	9.29
DET LW10B	0.139	0.67	01Jan2008, 11:44	3.18
L16A	0.066	15.41	01Jan2008, 10:48	4.43
DET LW10A	0.066	0.33	01Jan2008, 11:12	1.58
COMB10	5.208	68.53	01Jan2008, 11:20	58.39
RR 15	5.208	68.46	01Jan2008, 11:44	58.09
L17A	0.431	66.97	01Jan2008, 11:28	28.9
DET LW9A	5.639	9.48	01Jan2008, 12:40	44.48
L17B	0.189	31.93	01Jan2008, 11:20	12.67
DET LW9B	0.189	1.52	01Jan2008, 12:04	7.18
RR09	0.189	1.52	01Jan2008, 12:12	7.17
COMB9	5.828	11	01Jan2008, 12:40	51.64
RR 16	5.828	11	01Jan2008, 12:52	51.43
L18	0.351	56.15	01Jan2008, 11:20	22.89
DET LW8	0.351	1.5	01Jan2008, 11:52	7.11
RR 17	0.351	1.5	01Jan2008, 12:12	7.07
L19	0.098	16.83	01Jan2008, 11:16	6.57
DET LW7	0.098	0.5	01Jan2008, 11:52	2.37
COMB12	6.277	13	01Jan2008, 12:52	60.87
RR 18	6.277	13	01Jan2008, 13:08	60.61
L14	0.976	126.5	01Jan2008, 11:56	64.71
L13	0.0576	13.44	01Jan2008, 10:48	3.86
RR 13	0.0576	13.32	01Jan2008, 11:24	3.85
DET LW6	1.0336	4	01Jan2008, 13:04	18.64
L10	0.3523	9.47	01Jan2008, 12:52	7.6
L12	0.1614	26.97	01Jan2008, 11:20	10.77
L11	0.1307	26.9	01Jan2008, 10:56	8.5
L09	0.00002	0	01Jan2008, 12:28	0
RR 9	0.00002	0	01Jan2008, 16:04	0
DET LW12	0.13072	0.5	01Jan2008, 11:28	2.38
RR 10	0.13072	0.5	01Jan2008, 12:24	2.34
COMB7	0.29212	27.23	01Jan2008, 11:20	13.11
RR 11	0.29212	27.16	01Jan2008, 11:32	13.07
DET LW11	0.64442	2.5	01Jan2008, 14:00	11.52
RR 12	0.64442	2.5	01Jan2008, 14:20	11.44

Lammers Watershed - 10-yr Peak Flows

COMB14	7.95502	19.5	01Jan2008, 14:20	90.7
RR 19	7.95502	19.5	01Jan2008, 14:40	90.26
COMB15	7.95502	19.5	01Jan2008, 14:40	90.26
RR 20	7.95502	19.5	01Jan2008, 14:44	90.21
L21	0.1085	10.01	01Jan2008, 11:32	4.76
DET LW5	0.1085	0.5	01Jan2008, 13:12	2.33
COMB16	8.06352	20	01Jan2008, 14:44	92.54
RR 21	8.06352	20	01Jan2008, 14:52	92.41
L24	0.851	71.4	01Jan2008, 13:00	49.6
L20	0.146	26.1	01Jan2008, 11:08	9.51
Reach-1	0.146	26.1	01Jan2008, 11:12	9.51
DET LW3	0.997	4	01Jan2008, 13:52	18.47
Reach-2	0.997	4	01Jan2008, 13:56	18.44
COMB16A	9.06052	24	01Jan2008, 14:52	110.86
RR21A	9.06052	24	01Jan2008, 15:00	110.63
L21A	0.1345	14.44	01Jan2008, 11:36	6.78
DET LW4	0.1345	0.5	01Jan2008, 14:16	2.31
Reach-4	0.1345	0.5	01Jan2008, 14:20	2.31
COMB16B	9.19502	24.5	01Jan2008, 15:00	112.94
RR21B	9.19502	24.5	01Jan2008, 15:12	112.59
L22	0.3054	36.82	01Jan2008, 11:52	18.87
DET LW2	0.3054	2	01Jan2008, 12:40	9.38
COMB17	9.50042	26.5	01Jan2008, 15:12	121.97
RR 22	9.50042	26.5	01Jan2008, 15:32	121.23
L23	0.9656	96.48	01Jan2008, 12:32	59.49
DET LW1	0.9656	3	01Jan2008, 13:40	13.86
COMB18	10.46602	29.5	01Jan2008, 15:32	135.08
OFS3	5.6926	121.79	01Jan2008, 12:44	101.47
OFS4	0.2371	2.77	01Jan2008, 10:44	1.55
OFS5	0.0894	7.7	01Jan2008, 10:44	2.45
COMB1	6.0191	124.99	01Jan2008, 12:44	105.48
DIV1	6.0191	0	01Jan2008, 00:00	0
L05	0.2332	35.7	01Jan2008, 11:32	15.58
RR 5	0.2332	35.68	01Jan2008, 11:52	15.56
OUT1	0.2332	0	01Jan2008, 00:00	0
L04	0.00002	0	01Jan2008, 14:16	0
L03	0.00002	0	01Jan2008, 12:08	0
RR 3	0.00002	0	01Jan2008, 18:48	0
COMB3	0.00004	0	01Jan2008, 18:44	0
RR 4	0.00004	0	01Jan2008, 22:48	0
L06	0.00002	0	01Jan2008, 12:48	0
COMB4	0.00006	0	01Jan2008, 22:48	0
OUT2	0.00006	0	01Jan2008, 00:00	0
L02	0.00002	0	01Jan2008, 12:12	0
L01	0.00002	0	01Jan2008, 12:24	0
RR 1	0.00002	0	01Jan2008, 16:16	0
COMB2	0.00004	0	01Jan2008, 16:12	0

Lammers Watershed - 10-yr Peak Flows

RR 2	0.00004	0	01Jan2008, 20:00	0
OUT5	0.00004	0	01Jan2008, 00:00	0
L07	0.00002	0	01Jan2008, 12:28	0
OUT3	0.00002	0	01Jan2008, 00:00	0
L08	0.00002	0	01Jan2008, 13:32	0
OUT4	0.00002	0	01Jan2008, 00:00	0

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW1
Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 26Apr2010, 15:38:18 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	96.48 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 12:32
Peak Outflow :	3.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 13:40
Total Inflow :	59.5 (AC-FT)	Peak Storage :	54.9 (AC-FT)
Total Outflow :	13.9 (AC-FT)	Peak Elevation :	27.0 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW2
Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 26Apr2010, 15:38:18 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	36.82 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:52
Peak Outflow :	2.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 12:40
Total Inflow :	18.9 (AC-FT)	Peak Storage :	16.0 (AC-FT)
Total Outflow :	9.4 (AC-FT)	Peak Elevation :	31.9 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW3

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 09Nov2010, 14:04:18 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	75.889 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 13:04
Peak Outflow :	4.000 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 14:08
Total Inflow :	58.77 (AC-FT)	Peak Storage :	52.43 (AC-FT)
Total Outflow :	18.41 (AC-FT)	Peak Elevation :	76.96 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW4

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 09Nov2010, 14:04:18 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	14.437 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:36
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 14:16
Total Inflow :	6.78 (AC-FT)	Peak Storage :	6.11 (AC-FT)
Total Outflow :	2.31 (AC-FT)	Peak Elevation :	51.53 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW5

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 09Nov2010, 14:04:18 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	10.008 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:32
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 13:12
Total Inflow :	4.76 (AC-FT)	Peak Storage :	4.08 (AC-FT)
Total Outflow :	2.33 (AC-FT)	Peak Elevation :	66.71 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW6

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	135.98 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:48
Peak Outflow :	4.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:32
Total Inflow :	68.56 (AC-FT)	Peak Storage :	62.20 (AC-FT)
Total Outflow :	19.16 (AC-FT)	Peak Elevation :	103.33 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW7

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	16.83 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:16
Peak Outflow :	0.50 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:52
Total Inflow :	6.57 (AC-FT)	Peak Storage :	5.86 (AC-FT)
Total Outflow :	2.37 (AC-FT)	Peak Elevation :	107.02 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW8

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	56.15 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:20
Peak Outflow :	1.50 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:52
Total Inflow :	22.89 (AC-FT)	Peak Storage :	20.73 (AC-FT)
Total Outflow :	7.11 (AC-FT)	Peak Elevation :	107.10 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW9A

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	133.86 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:40
Peak Outflow :	10.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 10:56
Total Inflow :	86.99 (AC-FT)	Peak Storage :	52.24 (AC-FT)
Total Outflow :	48.72 (AC-FT)	Peak Elevation :	129.04 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW9B

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	31.93 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:20
Peak Outflow :	1.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 12:00
Total Inflow :	12.67 (AC-FT)	Peak Storage :	11.26 (AC-FT)
Total Outflow :	4.73 (AC-FT)	Peak Elevation :	126.97 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW10A

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	10-yr
Compute Time:	14Mar2012, 15:31:23	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	15.41 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:48
Peak Outflow :	0.33 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:12
Total Inflow :	4.43 (AC-FT)	Peak Storage :	3.97 (AC-FT)
Total Outflow :	1.58 (AC-FT)	Peak Elevation :	187.05 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW10B

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 14Mar2012, 15:31:23 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	25.96 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:08
Peak Outflow :	0.67 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:44
Total Inflow :	9.29 (AC-FT)	Peak Storage :	8.35 (AC-FT)
Total Outflow :	3.18 (AC-FT)	Peak Elevation :	186.99 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW11

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 09Nov2010, 14:04:18 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	32.970 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:40
Peak Outflow :	2.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 14:00
Total Inflow :	20.67 (AC-FT)	Peak Storage :	15.56 (AC-FT)
Total Outflow :	11.52 (AC-FT)	Peak Elevation :	126.81 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET LW12

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 10-yr
Compute Time: 09Nov2010, 14:04:18 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	26.902 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:56
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:28
Total Inflow :	8.50 (AC-FT)	Peak Storage :	7.81 (AC-FT)
Total Outflow :	2.38 (AC-FT)	Peak Elevation :	186.99 (FT)

Project: lammers_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET-OFF2

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	10-yr
Compute Time:	09Nov2010, 14:04:18	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	115.589 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:44
Peak Outflow :	2.337 (CFS)	Date/Time of Peak Outflow :	02Jan2008, 02:56
Total Inflow :	79.24 (AC-FT)	Peak Storage :	77.07 (AC-FT)
Total Outflow :	9.40 (AC-FT)	Peak Elevation :	229.20 (FT)

Lammers Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
OFS2	4.516	386.76	01Jan2008, 11:32	186.96
DET-OFF2	4.516	4.94	02Jan2008, 02:52	20.35
L15a	0.26	113.93	01Jan2008, 10:56	34.93
DET L15a	0.26	8	01Jan2008, 05:52	34.93
L15b	0.138	65.79	01Jan2008, 10:48	18.56
L15d	0.064	32.88	01Jan2008, 10:44	8.61
DET L15b	4.978	86.58	01Jan2008, 11:08	82.44
L15c	0.025	14.14	01Jan2008, 10:40	3.36
CPL15	5.003	94.57	01Jan2008, 11:04	85.8
RR14	5.003	94.41	01Jan2008, 11:20	85.47
L16B	0.139	41.98	01Jan2008, 11:08	14.56
DET LW10B	0.139	0.67	01Jan2008, 11:00	3.23
L16A	0.066	24.95	01Jan2008, 10:48	6.94
DET LW10A	0.066	0.33	01Jan2008, 10:40	1.6
COMB10	5.208	95.41	01Jan2008, 11:20	90.31
RR 15	5.208	95.14	01Jan2008, 11:40	89.84
L17A	0.431	108.06	01Jan2008, 11:28	45.32
DET LW9A	5.639	10	01Jan2008, 09:40	49.76
L17B	0.189	51.56	01Jan2008, 11:20	19.88
DET LW9B	0.189	1	01Jan2008, 11:08	4.81
RR09	0.189	1	01Jan2008, 11:16	4.8
COMB9	5.828	11	01Jan2008, 11:16	54.56
RR 16	5.828	11	01Jan2008, 11:32	54.35
L18	0.351	90.57	01Jan2008, 11:20	35.91
DET LW8	0.351	1.5	01Jan2008, 11:08	7.24
RR 17	0.351	1.5	01Jan2008, 11:28	7.2
L19	0.098	27.18	01Jan2008, 11:16	10.31
DET LW7	0.098	0.5	01Jan2008, 11:04	2.41
COMB12	6.277	13	01Jan2008, 11:32	63.96
RR 18	6.277	13	01Jan2008, 11:48	63.7
L14	0.976	203.6	01Jan2008, 11:56	101.52
L13	0.0576	21.76	01Jan2008, 10:48	6.06
RR 13	0.0576	21.65	01Jan2008, 11:20	6.04
DET LW6	1.0336	4	01Jan2008, 10:48	19.55
L10	0.3523	25.3	01Jan2008, 12:40	16.6
L12	0.1614	43.56	01Jan2008, 11:20	16.89
L11	0.1307	43.54	01Jan2008, 10:56	13.34
L09	0.00002	0	01Jan2008, 12:16	0
RR 9	0.00002	0	01Jan2008, 15:08	0
DET LW12	0.13072	0.5	01Jan2008, 10:48	2.42
RR 10	0.13072	0.5	01Jan2008, 11:40	2.38
COMB7	0.29212	43.96	01Jan2008, 11:20	19.27
RR 11	0.29212	43.89	01Jan2008, 11:32	19.23
DET LW11	0.64442	2.5	01Jan2008, 12:00	11.75
RR 12	0.64442	2.5	01Jan2008, 12:20	11.67

Lammers Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
COMB14	7.95502	19.5	01Jan2008, 12:20	94.92
RR 19	7.95502	19.5	01Jan2008, 12:36	94.48
COMB15	7.95502	19.5	01Jan2008, 12:36	94.48
RR 20	7.95502	19.5	01Jan2008, 12:40	94.43
L21	0.1085	18.39	01Jan2008, 11:32	8.17
DET LW5	0.1085	0.5	01Jan2008, 11:36	2.37
COMB16	8.06352	20	01Jan2008, 12:40	96.81
RR 21	8.06352	20	01Jan2008, 12:48	96.68
L24	0.851	117.38	01Jan2008, 13:00	79.61
L20	0.146	42.17	01Jan2008, 11:08	14.92
Reach-1	0.146	42.12	01Jan2008, 11:12	14.92
DET LW3	0.997	4	01Jan2008, 12:20	18.84
Reach-2	0.997	4	01Jan2008, 12:28	18.82
COMB16A	9.06052	24	01Jan2008, 12:48	115.5
RR21A	9.06052	24	01Jan2008, 12:56	115.28
L21A	0.1345	25.44	01Jan2008, 11:36	11.32
DET LW4	0.1345	0.5	01Jan2008, 11:56	2.36
Reach-4	0.1345	0.5	01Jan2008, 12:04	2.35
COMB16B	9.19502	24.5	01Jan2008, 12:56	117.63
RR21B	9.19502	24.5	01Jan2008, 13:08	117.28
L22	0.3054	59.64	01Jan2008, 11:52	29.84
DET LW2	0.3054	2	01Jan2008, 11:40	9.55
COMB17	9.50042	26.5	01Jan2008, 13:08	126.83
RR 22	9.50042	26.5	01Jan2008, 13:28	126.09
L23	0.9656	158.26	01Jan2008, 12:32	94.86
DET LW1	0.9656	3	01Jan2008, 12:20	14.12
COMB18	10.46602	29.5	01Jan2008, 13:28	140.21
OFS3	5.6926	375.08	01Jan2008, 12:28	238.36
OFS4	0.2371	7.05	01Jan2008, 11:04	4.83
OFS5	0.0894	19.79	01Jan2008, 10:44	5.11
COMB1	6.0191	385.06	01Jan2008, 12:28	248.3
DIV1	6.0191	0	01Jan2008, 00:00	0
L05	0.2332	57.6	01Jan2008, 11:32	24.43
RR 5	0.2332	57.5	01Jan2008, 11:48	24.41
OUT1	0.2332	0	01Jan2008, 00:00	0
L04	0.00002	0	01Jan2008, 13:56	0
L03	0.00002	0	01Jan2008, 11:56	0
RR 3	0.00002	0	01Jan2008, 17:00	0
COMB3	0.00004	0	01Jan2008, 17:00	0
RR 4	0.00004	0	01Jan2008, 20:08	0
L06	0.00002	0	01Jan2008, 12:32	0
COMB4	0.00006	0	01Jan2008, 20:08	0
OUT2	0.00006	0	01Jan2008, 00:00	0
L02	0.00002	0	01Jan2008, 12:00	0
L01	0.00002	0	01Jan2008, 12:12	0
RR 1	0.00002	0	01Jan2008, 15:20	0

Lammers Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
COMB2	0.00004	0	01Jan2008, 15:16	0
RR 2	0.00004	0	01Jan2008, 18:24	0
OUT5	0.00004	0	01Jan2008, 00:00	0
L07	0.00002	0	01Jan2008, 12:16	0
OUT3	0.00002	0	01Jan2008, 00:00	0
L08	0.00002	0	01Jan2008, 13:12	0
OUT4	0.00002	0	01Jan2008, 00:00	0

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW1

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	100-yr
Compute Time:	29Oct2010, 09:37:46	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	158.259 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 12:32
Peak Outflow :	3.000 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 12:20
Total Inflow :	94.86 (AC-FT)	Peak Storage :	89.83 (AC-FT)
Total Outflow :	14.12 (AC-FT)	Peak Elevation :	28.23 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW2

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 29Oct2010, 09:37:46 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	59.640 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:52
Peak Outflow :	2.000 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:40
Total Inflow :	29.84 (AC-FT)	Peak Storage :	26.72 (AC-FT)
Total Outflow :	9.55 (AC-FT)	Peak Elevation :	33.18 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW3

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	100-yr
Compute Time:	29Oct2010, 09:37:46	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	124.147 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 13:04
Peak Outflow :	4.000 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 12:32
Total Inflow :	93.97 (AC-FT)	Peak Storage :	87.00 (AC-FT)
Total Outflow :	18.79 (AC-FT)	Peak Elevation :	78.21 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW4

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 29Oct2010, 09:37:46 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	25.440 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:36
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:56
Total Inflow :	11.32 (AC-FT)	Peak Storage :	10.58 (AC-FT)
Total Outflow :	2.36 (AC-FT)	Peak Elevation :	52.58 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW5

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	100-yr
Compute Time:	29Oct2010, 09:37:46	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	18.386 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:32
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:36
Total Inflow :	8.17 (AC-FT)	Peak Storage :	7.43 (AC-FT)
Total Outflow :	2.37 (AC-FT)	Peak Elevation :	68.00 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW6

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 21Feb2012, 13:23:16 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	217.65 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:52
Peak Outflow :	4.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 10:48
Total Inflow :	107.56 (AC-FT)	Peak Storage :	100.68 (AC-FT)
Total Outflow :	19.55 (AC-FT)	Peak Elevation :	105.27 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW7

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	27.18 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:16
Peak Outflow :	0.50 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:04
Total Inflow :	10.31 (AC-FT)	Peak Storage :	9.55 (AC-FT)
Total Outflow :	2.41 (AC-FT)	Peak Elevation :	108.18 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW8

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	90.57 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:20
Peak Outflow :	1.50 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:08
Total Inflow :	35.91 (AC-FT)	Peak Storage :	33.59 (AC-FT)
Total Outflow :	7.24 (AC-FT)	Peak Elevation :	108.34 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW9A

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	201.53 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:36
Peak Outflow :	10.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 09:40
Total Inflow :	135.17 (AC-FT)	Peak Storage :	88.53 (AC-FT)
Total Outflow :	49.76 (AC-FT)	Peak Elevation :	131.61 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW9B

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	51.56 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:20
Peak Outflow :	1.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:08
Total Inflow :	19.88 (AC-FT)	Peak Storage :	18.36 (AC-FT)
Total Outflow :	4.81 (AC-FT)	Peak Elevation :	128.13 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW10A

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	24.95 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:48
Peak Outflow :	0.33 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 10:40
Total Inflow :	6.94 (AC-FT)	Peak Storage :	6.45 (AC-FT)
Total Outflow :	1.60 (AC-FT)	Peak Elevation :	188.20 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW10B

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 17Feb2012, 15:45:31 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	41.98 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:08
Peak Outflow :	0.67 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 11:00
Total Inflow :	14.56 (AC-FT)	Peak Storage :	13.56 (AC-FT)
Total Outflow :	3.23 (AC-FT)	Peak Elevation :	188.15 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW11

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 29Oct2010, 09:37:46 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	60.758 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:44
Peak Outflow :	2.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 12:00
Total Inflow :	35.82 (AC-FT)	Peak Storage :	30.38 (AC-FT)
Total Outflow :	11.75 (AC-FT)	Peak Elevation :	128.44 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET LW12

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 29Oct2010, 09:37:46 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	43.542 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:56
Peak Outflow :	0.500 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 10:48
Total Inflow :	13.34 (AC-FT)	Peak Storage :	12.60 (AC-FT)
Total Outflow :	2.42 (AC-FT)	Peak Elevation :	188.12 (FT)

Project: lammers_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET-OFF2
Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 29Oct2010, 09:37:46 Control Specifications: Control 1
Volume Units: AC-FT

Computed Results

Peak Inflow :	386.762 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:32
Peak Outflow :	4.944 (CFS)	Date/Time of Peak Outflow :	02Jan2008, 02:52
Total Inflow :	186.96 (AC-FT)	Peak Storage :	182.04 (AC-FT)
Total Outflow :	20.35 (AC-FT)	Peak Elevation :	233.90 (FT)

Mountain House Watershed - 10-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
MH2	0.6275	134.94	01Jan2008, 11:28	56.47
OFS1	0.4039	3.02	01Jan2008, 11:24	3.25
Reach-3	0.4039	3.01	01Jan2008, 12:52	3.25
DET MHW2	1.0314	5.5	02Jan2008, 02:16	21.78
RR23	1.0314	5.5	02Jan2008, 02:36	21.61
MH3	0.4001	88.68	01Jan2008, 10:48	25.97
DET MHW3	0.4001	4.28	02Jan2008, 00:56	15.55
COMB19	1.4315	9.73	02Jan2008, 01:40	37.17
MH1	0.2687	47.96	01Jan2008, 10:56	15.17

Project: mountain_house_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET MHW2

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	10-yr
Compute Time:	14Mar2012, 14:40:23	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	135.50 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:28
Peak Outflow :	5.50 (CFS)	Date/Time of Peak Outflow :	02Jan2008, 02:16
Total Inflow :	59.72 (AC-FT)	Peak Storage :	53.10 (AC-FT)
Total Outflow :	21.78 (AC-FT)	Peak Elevation :	186.28 (FT)

Project: mountain_house_watershed
Simulation Run: 10-yr 24-hr Reservoir: DET MHW3

Start of Run:	01Jan2008, 00:00	Basin Model:	Basin 1
End of Run:	03Jan2008, 18:36	Meteorologic Model:	10-yr
Compute Time:	14Mar2012, 14:40:23	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	88.68 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:48
Peak Outflow :	4.28 (CFS)	Date/Time of Peak Outflow :	02Jan2008, 00:56
Total Inflow :	25.97 (AC-FT)	Peak Storage :	21.09 (AC-FT)
Total Outflow :	15.55 (AC-FT)	Peak Elevation :	178.43 (FT)

Mountain House Watershed - 100-yr Peak Flows

Hydrologic Element	Drainage Area (MI ²)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)

MH2	0.6275	198.71	01Jan2008, 11:28	83.1
OFS1	0.4039	12.69	01Jan2008, 11:56	9.56
Reach-3	0.4039	12.67	01Jan2008, 12:56	9.55
DET MHW2	1.0314	7	01Jan2008, 17:16	31.66
RR23	1.0314	7	01Jan2008, 17:40	31.43
MH3	0.4001	143.61	01Jan2008, 10:48	40.74
DET MHW3	0.4001	5	01Jan2008, 13:32	4.19
COMB19	1.4315	12	01Jan2008, 17:40	35.62
MH1	0.2687	70.22	01Jan2008, 10:56	23.28

Project: mountain_house_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET MHW2

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 14Mar2012, 14:41:13 Control Specifications: Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	199.60 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 11:28
Peak Outflow :	7.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 17:16
Total Inflow :	92.65 (AC-FT)	Peak Storage :	83.19 (AC-FT)
Total Outflow :	31.66 (AC-FT)	Peak Elevation :	189.84 (FT)

Project: mountain_house_watershed
Simulation Run: 100-yr 24-hr Reservoir: DET MHW3

Start of Run: 01Jan2008, 00:00 Basin Model: Basin 1
End of Run: 03Jan2008, 18:36 Meteorologic Model: 100-yr
Compute Time: 14Mar2012, 14:41:13 Control Specifications: Control 1

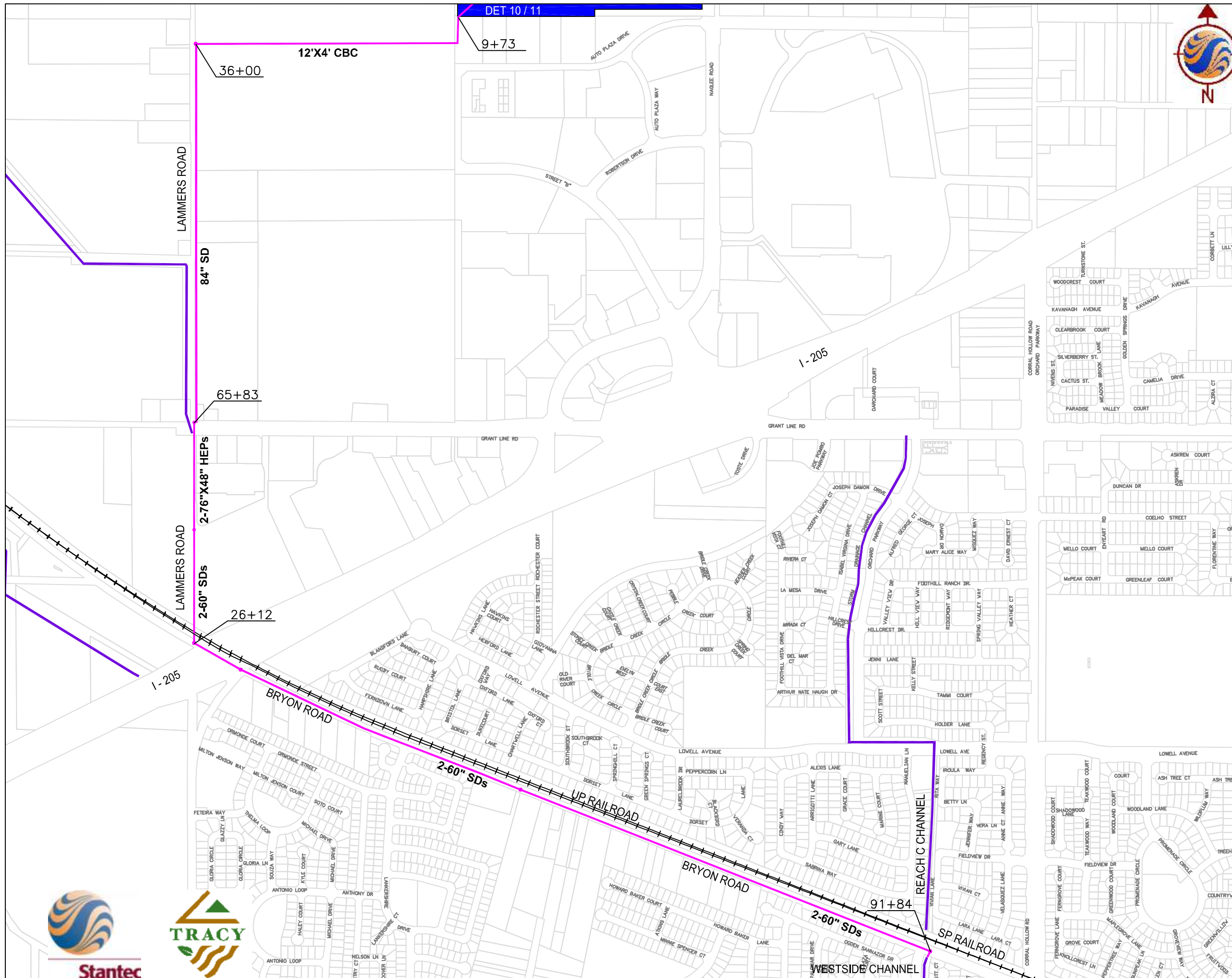
Volume Units: AC-FT

Computed Results

Peak Inflow :	143.61 (CFS)	Date/Time of Peak Inflow :	01Jan2008, 10:48
Peak Outflow :	5.00 (CFS)	Date/Time of Peak Outflow :	01Jan2008, 13:32
Total Inflow :	40.74 (AC-FT)	Peak Storage :	36.55 (AC-FT)
Total Outflow :	4.19 (AC-FT)	Peak Elevation :	179.99 (FT)

APPENDIX C

HYDRAULIC GRADE LINE CALCULATIONS



DET 10 / 11

12'X4' CBC

9+73

36+00

84" SD

65+83

2-76" X 48" HEPS

2-60" SDs

26+12

2-60" SDs

91+84

2-60" SDs

TRACY



Client/Project
 CITY OF TRACY
 CITYWIDE STORM DRAINAGE
 MASTER PLAN

APPENDIX C

Title
**HGL KEY MAP
 WESTSIDE CHANNEL
 OUTFALL SYSTEM**

DECEMBER 2010
 184010207
 SCALE: 1" = 750'

City of Tracy
 Citywide Storm Drainage Master Plan
 Hydraulic Grade Line Calculations
 Westside Channel Outfall System
 100-year 24-hour Storm
 2.6 feet has been added to all elevations to convert to NAVD 88

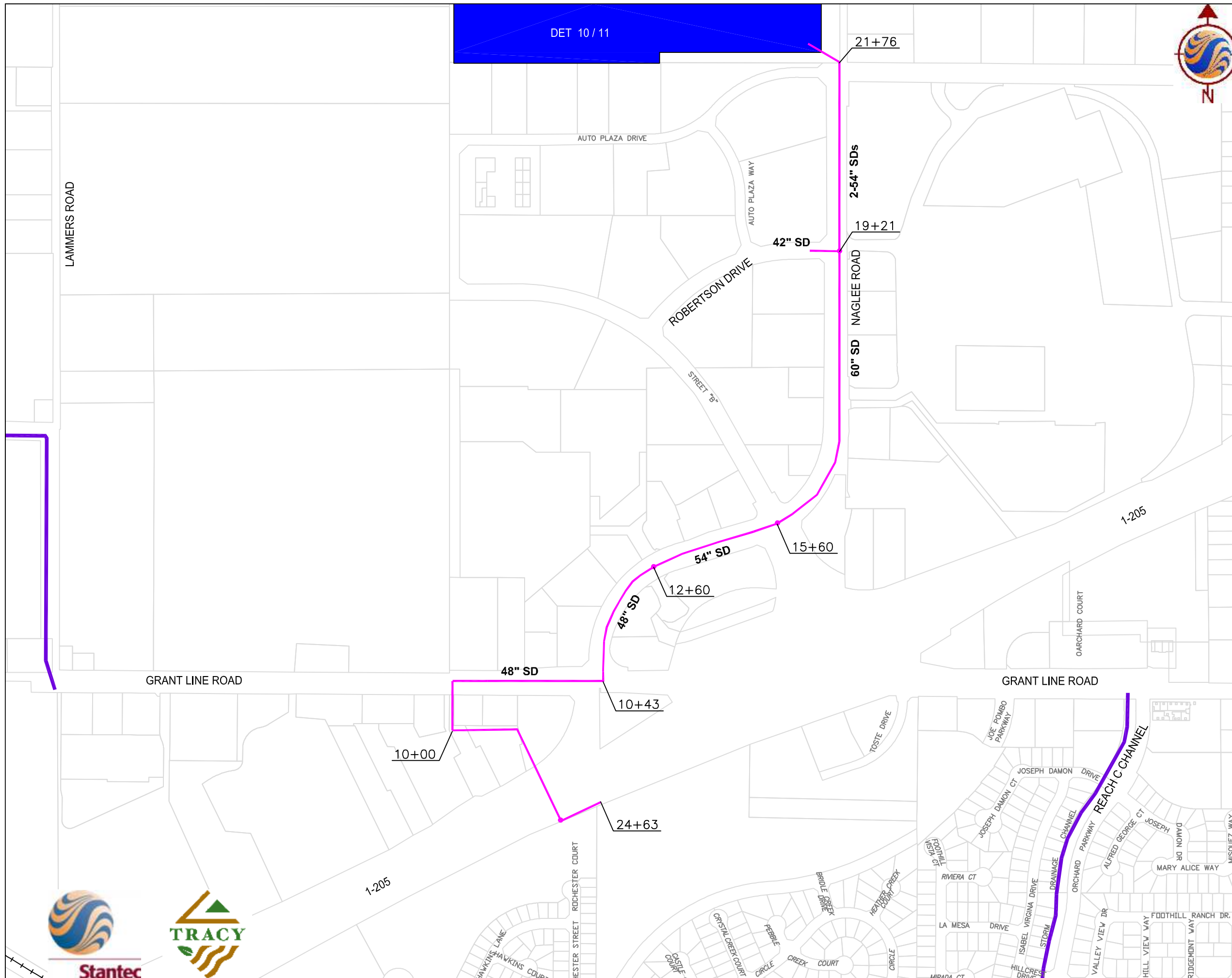
Manning's n= 0.013

Tailwater= 13.1 ft
 from DET 10/11 HEC-HMS 100-yr WSE

Station	Invert (ft)	Dia (in)	n	Length (ft)	Pipe Slope ft/ft	Flow (cfs)	Area (ft ²)	Wetted P (ft)	Hyd. Rad. (ft)	R ^{2/3} (ft)	Velocity (ft/s)	S _f (ft)	H _f (ft)	K _{minor}	H _{minor} (ft)	H _{total} (ft)	HGL (ft)	Top of Curb (ft)	Freeboard* (ft)	Note
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)
						$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$	$\frac{4.71}{\pi \cdot (1)^2} \cdot 4$
9+73	2.6																13.07	10.69		bolt
12x4'		94	0.013	205	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.19							
11+78	2.7													0.30	0.185	0.379	13.4	10.62	-2.8	bolt
12x4'		94	0.013	251	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.24							
14+29	2.83													0.30	0.185	0.422	13.9	11.60	-2.3	bolt
12x4'		94	0.013	22	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.02							
14+51	2.84													0.30	0.185	0.206	14.1	11.40	-2.7	bolt
12x4'		94	0.013	449	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.43							
19+00	3.06													0.05	0.031	0.456	14.5	11.90	-2.6	bolt
12x4'		94	0.013	550	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.52							
24+50	3.34													0.05	0.031	0.551	15.1	12.80	-2.3	bolt
12x4'		94	0.013	550	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.52							
30+00	3.61													0.05	0.031	0.551	15.6	12.20	-3.4	bolt
12x4'		94	0.013	570	0.0005	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.54							
35+70	3.90													0.05	0.031	0.570	16.2	13.80	-2.4	bolt
12x4'		94	0.013	30	0.0007	265.3	48.19	24.61	1.96	1.57	6.30	0.0009	0.03							
Lammers Road																				
36+00	3.92													0.05	0.031	0.059	16.3	14.10	-2.2	bolt
		84	0.013	532	0.0005	265.3	38.48	21.99	1.75	1.45	7.55	0.0017	0.92							
41+32	4.18													0.05	0.044	0.962	17.2	13.60	-3.6	bolt
		84	0.013	594	0.0014	265.3	38.48	21.99	1.75	1.45	7.55	0.0017	1.02							
47+25	5.01													0.05	0.044	1.069	18.3	18.60	0.3	
		84	0.013	625	0.0015	265.3	38.48	21.99	1.75	1.45	7.55	0.0017	1.08							ADD11
53+50	5.96													0.05	0.044	1.122	19.4	21.10	1.7	
		84	0.013	400	0.0110	256.1	38.48	21.99	1.75	1.45	7.55	0.0016	0.64							
57+50	10.34													0.05	0.044	0.687	20.1	22.70	2.6	
		84	0.013	416	0.0014	256.1	38.48	21.99	1.75	1.45	7.55	0.0016	0.67							
61+66	10.93													0.05	0.044	0.713	20.8	24.30	3.5	
		84	0.013	416	0.0014	256.1	38.48	21.99	1.75	1.45	7.55	0.0016	0.67							
Grant Line Road																				
65+83	11.51													0.05	0.044	0.713	21.5	26.0	4.5	
		84	0.013	115	0.0698	256.1	38.48	21.99	1.75	1.45	7.55	0.0016	0.18							ADD10
10+54	19.54													0.05	0.044	0.229	21.8	26.60	4.8	
		60	0.013	432	0.0005	122.2	19.63	15.71	1.25	1.16	7.43	0.0022	0.95							
14+87	19.75													0.05	0.043	0.994	22.8	28.90	6.1	
		60	0.013	455	0.0005	122.2	19.63	15.71	1.25	1.16	7.43	0.0022	1.00							
I-205																				
19+55	19.96													0.05	0.043	1.045	23.8	30.30	6.5	

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard*	Note	
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)		
	(1)	(2)	(3)	(4)	(5)	(6)	$\frac{\pi \cdot (1)^2}{4}$	$\pi \cdot (1)$	(4)/(5)	$(\frac{7}{6})^{0.67}$	(3)/(4)	(9)	(10)	(11)	(12)	(14)	(10)+(12)+(13)	(15)	(16)	(17)	
2x60"		60	0.013	342	0.0019	122.2	19.63	15.71	1.25	1.16	7.43	0.0022	0.75								
22+80	20.62													0.05	0.043	0.796	25.0	31.80	6.8		
2x60"		60	0.013	332	0.0020	122.2	19.63	15.71	1.25	1.16	7.43	0.0022	0.73								
Byron Road																					
26+12	21.28													0.05	0.043	0.774	25.8	33.20	7.4		
2x60"		60	0.013	114	0.0015	122.2	19.63	15.71	1.25	1.16	7.43	0.0022	0.25								CW86
27+38	21.45													0.05	0.043	0.294	26.1	34.44	8.3		
2x60"		60	0.013	431	0.0015	112.3	19.63	15.71	1.25	1.16	7.43	0.0019	0.80								
31+90	22.10													0.05	0.043	0.843	26.9	36.21	9.3		
2x60"		60	0.013	590	0.0015	112.3	19.63	15.71	1.25	1.16	5.81	0.0019	1.10								
37+72	22.97													0.05	0.026	1.122	28.1	35.90	7.8		
2x60"		60	0.013	430	0.0015	112.3	19.63	15.71	1.25	1.16	5.81	0.0019	0.80								
42+02	23.61													0.05	0.026	0.825	28.9	37.00	8.1		
2x60"		60	0.013	421	0.0015	112.3	19.63	15.71	1.25	1.16	5.81	0.0019	0.78								
46+48	24.24													0.05	0.026	0.808	29.7	36.00	6.3		
2x60"		60	0.013	537	0.0015	112.3	19.63	15.71	1.25	1.16	5.81	0.0019	1.00								
51+68	25.04													0.05	0.026	1.024	30.7	37.60	6.9		
2x60"		60	0.013	587	0.0015	112.3	19.63	15.71	1.25	1.16	5.81	0.0019	1.09								BERG
57+30	25.92													0.05	0.026	1.117	31.8	39.10	7.3		
2x60"		60	0.013	553	0.0015	105.2	19.63	15.71	1.25	1.16	5.81	0.0016	0.90								
62+86	26.75													0.05	0.026	0.928	32.8	38.04	5.3		
2x60"		60	0.013	563	0.0015	105.2	19.63	15.71	1.25	1.16	5.81	0.0016	0.92								
68.48	27.59													0.05	0.026	0.945	33.7	39.73	6.0		
2x60"		60	0.013	563	0.0015	105.2	19.63	15.71	1.25	1.16	5.81	0.0016	0.92								
74+11	28.43													0.05	0.026	0.945	34.7	41.45	6.8		
2x60"		60	0.013	573	0.0016	105.2	19.63	15.71	1.25	1.16	5.81	0.0016	0.93								BYRN
80+19	29.33													0.05	0.026	0.961	35.6	41.35	5.7		
2x60"		60	0.013	585	0.0013	101.0	19.63	15.71	1.25	1.16	3.95	0.0015	0.88								
85+77	30.10													0.05	0.012	0.892	36.5	39.70	3.2		
2x60"		60	0.013	607	0.0016	101.0	19.63	15.71	1.25	1.16	3.95	0.0015	0.91								
91+84	31.07													0.05	0.012	0.925	37.4	39.80	2.4	Belconte	

*In the lower reach conduits where negative freeboard is shown, the system is under pressure with bolt manhole covers.



Client/Project
 CITY OF TRACY
 CITYWIDE STORM DRAINAGE
 MASTER PLAN

APPENDIX C

Title
**HGL KEY MAP
 I-205 SPECIFIC PLAN AREA**

DECEMBER 2010
 184010207
 SCALE: 1" = 500'



City of Tracy
Citywide Storm Drainage Master Plan
Hydraulic Grade Line Calculations
I-205 Specific Plan Area
10-year 24-hour Storm
3.1 feet has been added to all elevations to convert to NAVD 88

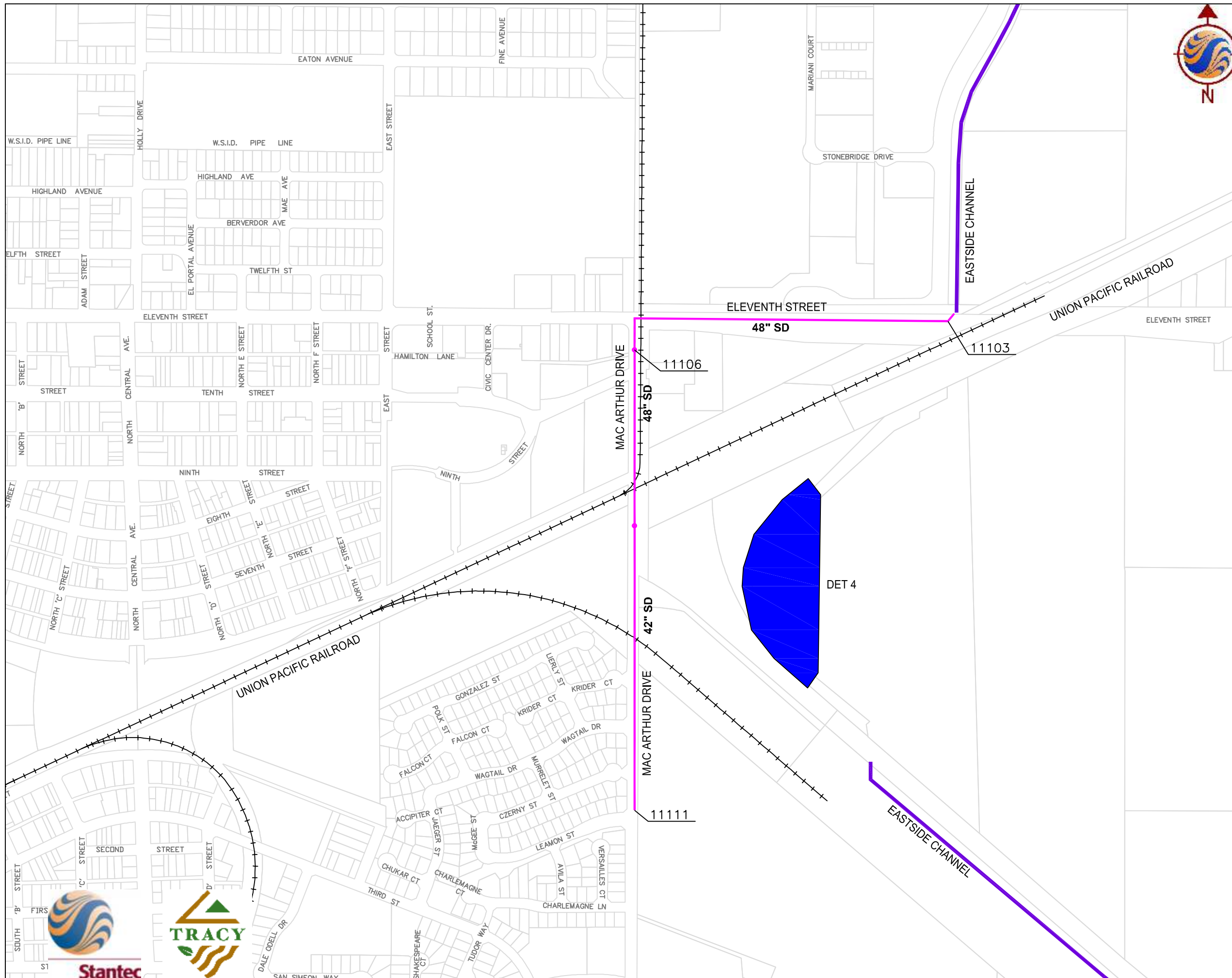
Manning's n= 0.013

Tailwater= 13.57 ft
from DET 10/11 HEC-HMS 100-year WSE

Station	Invert (ft)	Dia (in)	n	Length (ft)	Pipe Slope ft/ft	Flow (cfs)	Area (ft ²)	Wetted P (ft)	Hyd. Rad. (ft)	R ^{2/3} (ft)	Velocity (ft/s)	S _f (ft)	H _f (ft)	K _{minor}	H _{minor} (ft)	H _{total} (ft)	HGL (ft)	Top of Curb (ft)	Freeboard (ft)	Note
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)
Outlet	4.20																			
2-54"		54	0.013	17.6	0.0006	76.4	15.90	14.14	1.13	1.08	4.80	0.0015	0.03				13.57	12.18	-1.4	Det 10/11
22+22	4.21													0.05	0.018	0.044	13.6	12.26	-1.4	W93 + W90
2-54"		54	0.013	147	0.0004	76.4	15.90	14.14	1.13	1.08	4.80	0.0015	0.22							
Auto Plaza Drive																				
21+76	4.27													0.05	0.018	0.239	13.9	13.12	-0.7	Q10=126
2-54"		54	0.013	289	0.0004	63.0	15.90	14.14	1.13	1.08	3.96	0.0010	0.30							
20+86	4.39													0.05	0.012	0.309	14.2	14.33	0.2	
2-54"		54	0.013	374	0.0004	63.0	15.90	14.14	1.13	1.08	3.96	0.0010	0.38							
19+71	4.54													0.05	0.012	0.396	14.6	15.73	1.2	
2-54"		54	0.013	160	0.0004	63.0	15.90	14.14	1.13	1.08	3.96	0.0010	0.16							
Robertson Drive																				
19+21	4.60													0.05	0.012	0.176	14.7	15.97	1.2	Q10=94
		60	0.013	111	0.0212	94.0	19.63	15.71	1.25	1.16	4.79	0.0013	0.14							
18.86	6.95													0.05	0.018	0.162	14.9	16.98	2.1	
		60	0.013	367	0.0038	94.0	19.63	15.71	1.25	1.16	4.79	0.0013	0.48							
Yee Driveway																				
17+72	8.36													0.05	0.018	0.496	15.4	18.80	3.4	
		60	0.013	419	0.0039	94.0	19.63	15.71	1.25	1.16	4.79	0.0013	0.55							
16+43	9.98													0.05	0.018	0.564	16.0	20.47	4.5	
		60	0.013	230	0.0038	94.0	19.63	15.71	1.25	1.16	4.79	0.0013	0.30							
Pavillion Parkway																				
15+60	10.85													0.05	0.018	0.317	16.3	22.23	6.0	Q10=62
		60	0.013	373	0.0027	62.0	19.63	15.71	1.25	1.16	3.16	0.0006	0.21							
14+54	11.85													0.05	0.008	0.219	16.5	22.65	6.2	
		60	0.013	23	0.0004	62.0	19.63	15.71	1.25	1.16	3.16	0.0006	0.01							
14+48	11.86													0.05	0.008	0.021	16.5	22.58	6.1	
		54	0.013	113	0.0022	62.0	15.90	14.14	1.13	1.08	3.90	0.0010	0.11							
14+16	12.11													0.05	0.012	0.124	16.6	22.44	5.8	
		54	0.013	523	0.0023	62.0	15.90	14.14	1.13	1.08	3.90	0.0010	0.52							
12+60	13.32													0.05	0.012	0.532	17.2	21.35	4.2	
		48	0.013	423	0.0023	50.0	12.57	12.57	1.00	1.00	3.98	0.0012	0.51							Q10=50
11+20	14.29													0.05	0.012	0.525	17.7	22.73	5.0	
		48	0.013	251	0.0032	50.0	12.57	12.57	1.00	1.00	3.98	0.0012	0.30							
Grant Line Road																				
10+43	15.09													0.05	0.012	0.316	18.0	24.50	6.5	
		48	0.013	574	0.0028	50.0	12.57	12.57	1.00	1.00	3.98	0.0012	0.70							
5+00	16.67													0.05	0.012	0.708	18.7	26.40	7.7	
		48	0.013	500	0.0021	50.0	12.57	12.57	1.00	1.00	3.98	0.0012	0.61							
1+00	17.70													0.05	0.012	0.618	19.3	27.10	7.8	

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note	
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(14)	(15)	(16)	(17)		
							$\text{PI} \cdot (1)^{2/4}$	$\text{PI} \cdot (1)$	(4)/(5)	(6) ^{0.67}	(3)/(4)	$\text{PI} \cdot 1.49 \cdot (4) \cdot (7)$	(2) ^{1/9}		$(11) \cdot (8)^{2/2g}$	$(10) + (12) + (13)$	$\text{TW} + \Sigma(14)$		(16)-(15)		
Across Grant Line Road 3-36" Storm Drain																					
0+00	18.70																19.3	27.10	7.8		
3-36"		36	0.013	110	0.0068	11.5	7.07	9.42	0.75	0.83	1.62	0.0003	0.03							Q10=34.4	
South of Grant Line Road																					
10+00	19.45													0.05	0.002	0.035	19.4	27.10	7.7		
		48	0.013	162	0.0010	34.4	12.57	12.57	1.00	1.00	2.74	0.0006	0.09								
11+62	19.61													0.05	0.006	0.099	19.5	28.30	8.8		
		48	0.013	327	0.0009	34.4	12.57	12.57	1.00	1.00	2.74	0.0006	0.19								
14+90	19.92													0.05	0.006	0.193	19.7	26.20	6.5		
		48	0.013	317	0.0008	34.4	12.57	12.57	1.00	1.00	2.74	0.0006	0.18								
18+06	20.18													0.05	0.006	0.188	19.9	26.30	6.4		
		48	0.013	315	0.0007	34.4	12.57	12.57	1.00	1.00	2.74	0.0006	0.18								
21+22	20.40													0.05	0.006	0.186	20.2	28.40	8.2		
		42	0.013	341	0.0012	34.4	9.62	11.00	0.88	0.91	3.58	0.0012	0.40								
24+63	20.81													0.05	0.010	0.409	20.6	27.30	6.7	W90	

* For design purposes, where the freeboard shown is negative, assume the HGL is equivalent to the top of curb plus 0.5 feet



Client/Project
 CITY OF TRACY
 CITYWIDE STORM DRAINAGE
 MASTER PLAN

APPENDIX C
HGL KEY MAP
MACARTHUR / 11TH STREET

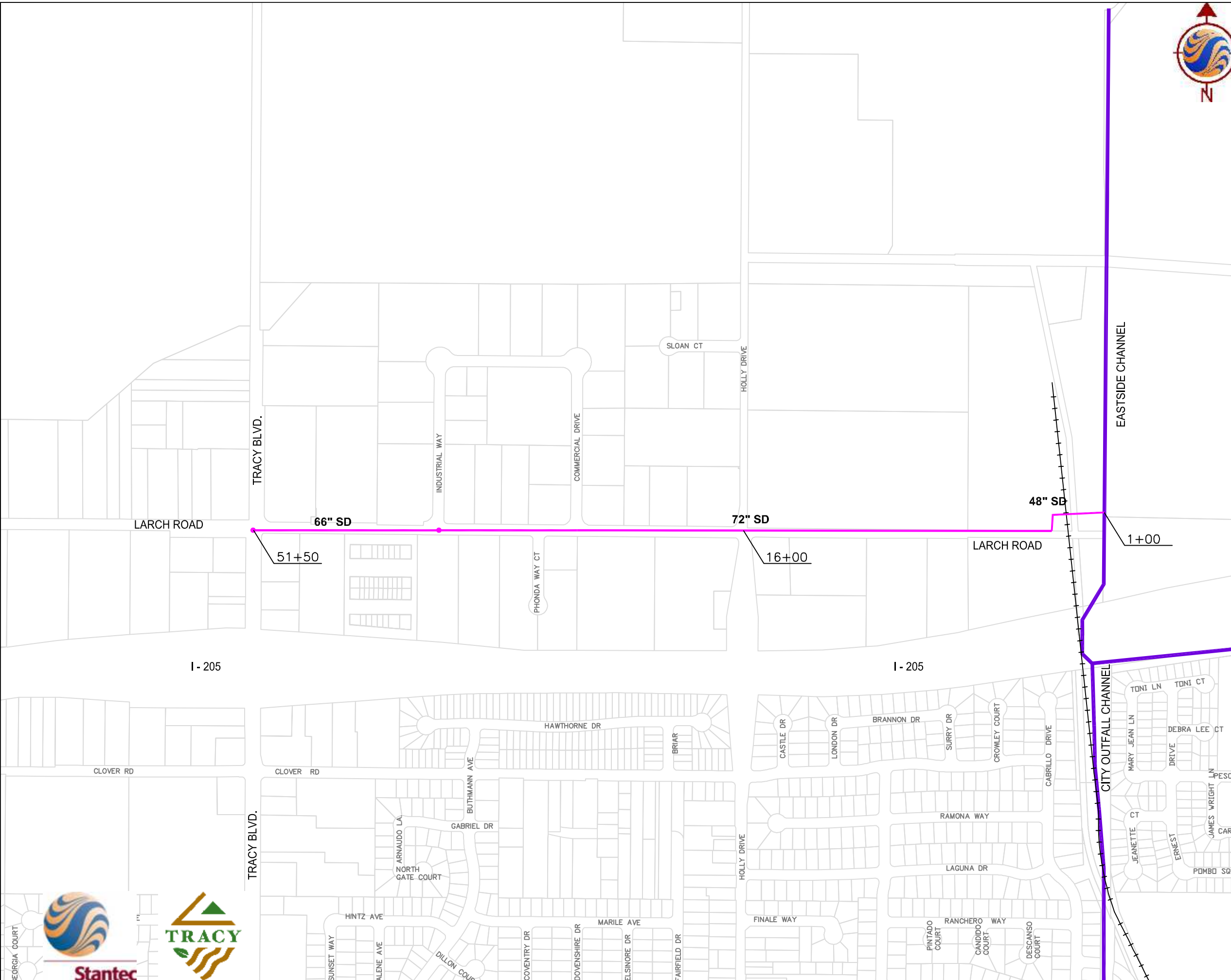
DECEMBER 2010
 184010207
 SCALE: 1" = 500'

City of Tracy
Citywide Storm Drainage Master Plan
Hydraulic Grade Line Calculations
MacArthur Drive to Eleventh Street Storm Drain
10-year 24-hour Storm

Manning's n= 0.013

Tailwater= 46.36 ft
from crown of 48" pipe

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note
	(ft)	(in)		(ft)	f/ft	(cfs)	(ft ²)	(ft)	(ft)	(ft)	(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	PI*(1) ² /4	PI*(1)	(4)/(5)	(7) (6) ^{0.67}	(8) (3)/(4)	(9))*n/1.49*(4) ^{1/3}	(10) (2)*(9)	(11)	(12) (11)*(8) ² /2g	(14) (10)+(12)+(13)	(15) TW+Σ(14)	(16)	(17) (16)-(15)	
Outfall	42.36																46.36	52.12		ES Channel
ES Channel		48	0.013	400	0.0012	67.3	12.57	12.57	1.00	1.00	6.27	0.0022	0.88							
11110	42.84													0.05	0.031	0.909	47.3	52.54	5.3	
		48	0.013	430	0.0014	57.1	12.57	12.57	1.00	1.00	4.81	0.0016	0.68							
11109	43.43													0.05	0.018	0.697	48.0	51.93	4.0	
		48	0.013	1340	0.0014	57.1	12.57	12.57	1.00	1.00	4.93	0.0016	2.12							Comb 10
11th Street																				
11103	45.37													0.05	0.019	2.136	50.1	51.42	1.3	
		48	0.013	685	0.0027	54.3	12.57	12.57	1.00	1.00	6.45	0.0014	0.98							
11108	47.20													0.05	0.032	1.011	51.1	55.20	4.1	Comb 9A
		48	0.013	820	0.0010	54.3	12.57	12.57	1.00	1.00	6.88	0.0014	1.17							
11107	47.99													0.05	0.037	1.208	52.3	57.69	5.4	
		48	0.013	330	0.0024	44.4	12.57	12.57	1.00	1.00	6.04	0.0010	0.32							
Railroad Crossing																				
11106	48.77													0.05	0.028	0.344	52.7	58.87	6.2	Comb 9
		48	0.013	260	0.0035	44.4	12.57	12.57	1.00	1.00	6.81	0.0010	0.25							
11105	49.67													0.05	0.036	0.284	53.0	59.97	7.0	
		42	0.013	285	0.0048	38.8	9.62	11.00	0.88	0.91	7.42	0.0015	0.42							
11104	51.03													0.05	0.043	0.467	53.4	61.33	7.9	
		42	0.013	220	0.0057	38.8	9.62	11.00	0.88	0.91	7.93	0.0015	0.33							
11111	52.28													0.05	0.049	0.376	54.1	62.43	8.4	Comb 8
Leamon St.																				



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Client/Project
CITY OF TRACY
CITYWIDE STORM DRAINAGE
MASTER PLAN

APPENDIX C

Title
**HGL KEY MAP
LARCH RD, TRACY BLVD
TO EASTSIDE CHANNEL**
DECEMBER 2010
184010207
SCALE: 1" = 500'

City of Tracy
 Citywide Storm Drainage Master Plan
 Hydraulic Grade Line Calculations
 Larch Road between Tracy Blvd and Eastside Channel

100-year 24-hour Storm

2.57 feet has been added to all elevations to convert to NAVD 88

Manning's n= 0.013

Tailwater= 8.38 ft

from top of crown of 72" pipe

Station	Invert (ft)	Dia (in)	n	Length (ft)	Pipe Slope ft/ft	Flow (cfs)	Area (ft ²)	Wetted P (ft)	Hyd. Rad. (ft)	R ^{2/3} (ft) ^{0.67}	Velocity (ft/s)	S _f (ft)	H _f (ft)	K _{minor} (11)	H _{minor} (ft)	H _{total} (ft)	HGL (ft)	Top of Curb (ft)	Freeboard (ft)	Note
	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)
						$PI \cdot (1)^{2/4}$	$PI \cdot (1)$	$(4)/(5)$	$(6)^{0.67}$	$(3)/(4)$	$n/1.49 \cdot (4)^{0.04}$	$(2) \cdot (9)$	$(11) \cdot (8)^{2/2g}$	$(10) + (12) + (13)$	$TW + \Sigma(14)$	(16)	$(16) - (15)$			
1+00	2.71	72	0.013	570	0.0009	214.4	28.27	18.85	1.50	1.31	8.25	0.0026	1.46				8.38	22.97	14.6	Pump Sta
5+70	3.24	72	0.013	930	0.0012	174.4	28.27	18.85	1.50	1.31	7.68	0.0017	1.58	0.05	0.053	1.514	9.9	21.14	11.2	Comb 40
Holly Drive																				
16+00	4.32	72	0.013	860	0.0010	153.0	28.27	18.85	1.50	1.31	7.67	0.0013	1.12	0.05	0.046	1.623	11.5	21.37	9.9	
27+00	5.15	72	0.013	175	0.0007	153.0	28.27	18.85	1.50	1.31	7.28	0.0013	0.23	0.05	0.046	1.168	12.7	19.77	7.1	
28+50	5.27	72	0.013	550	0.0010	126.6	28.27	18.85	1.50	1.31	6.88	0.0009	0.49	0.05	0.041	0.269	13.0	19.82	6.9	Comb 38
41.+50	5.80	66	0.013	1050	0.0009	126.6	23.76	17.28	1.38	1.24	5.29	0.0014	1.49	0.05	0.037	0.528	13.5	19.37	5.9	
Tracy Blvd.																				
51+50	6.79													0.05	0.022	1.514	15.0	15.07	0.1	Comb 37



Client/Project
CITY OF TRACY
CITYWIDE STORM DRAINAGE
MASTER PLAN

APPENDIX C

Title
**HGL KEY MAP
GRANT LINE RD, PARKER AVE
TO CITY OUTFALL**

DECEMBER 2010
184010207
SCALE: 1" = 500'



City of Tracy
 Citywide Storm Drainage Master Plan
 Hydraulic Grade Line Calculations
 Grant Line Road from Parker Avenue to City Outfall Channel
 10-year 24-hour Storm
 2.6 feet has been added to all elevations to convert to NAVD 88

Manning's n= 0.013

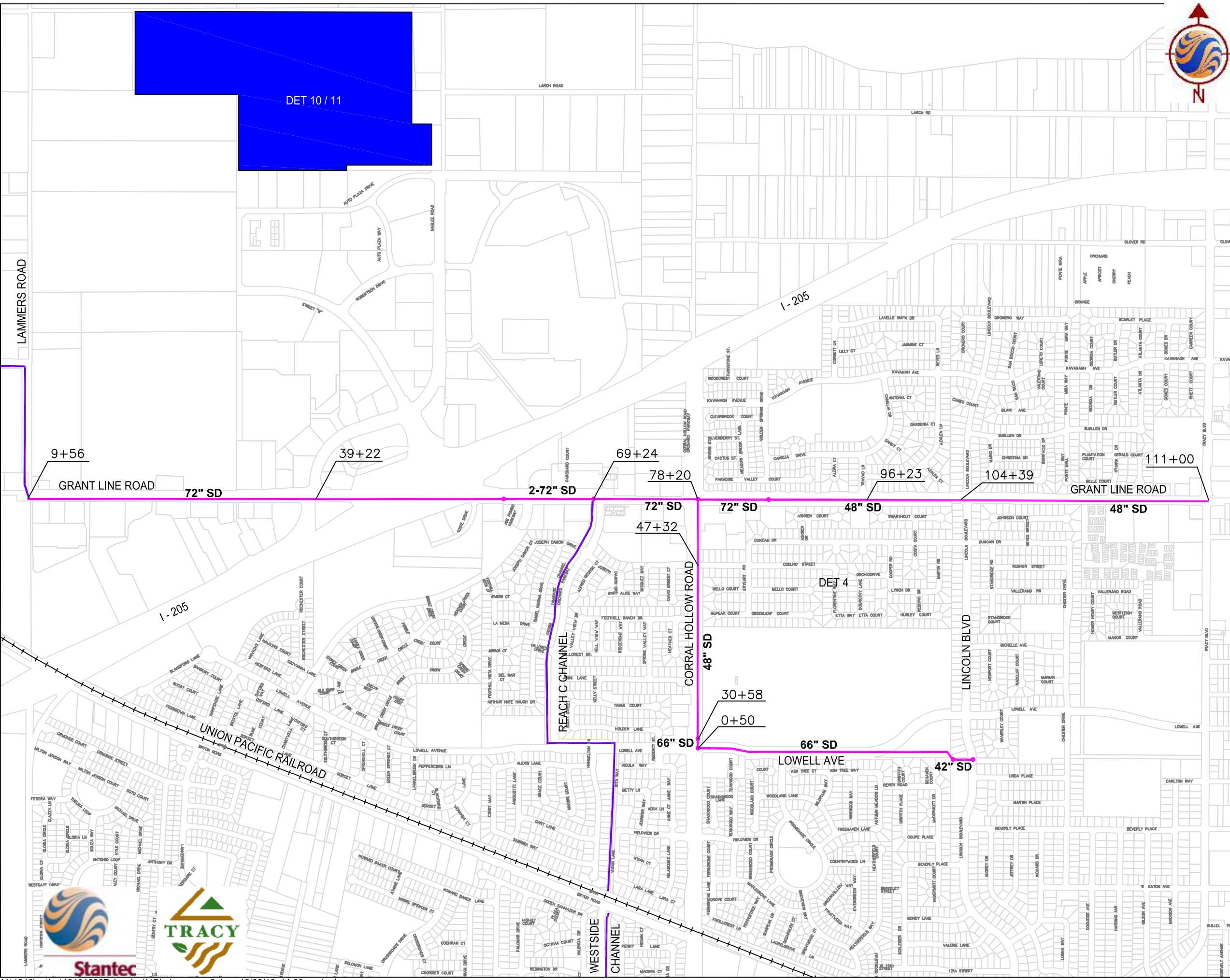
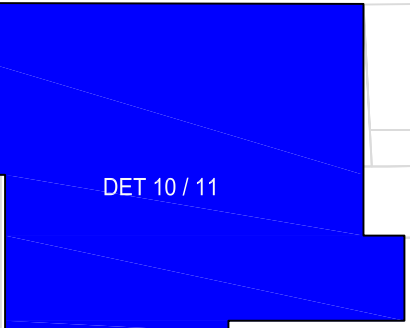
Tailwater= 21.57 ft
 from City Outfall Channel HEC-RAS

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note	
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	PI*(1) ² /4	PI*(1)	(4)/(5)	(6) ^{0.67}	(3)/(4)	(9)	(10)	(11)	(12)	(14)	(10)+(12)+(13)	TW+Σ(14)	(16)	(17)	
35+54 (out)	17.67																21.57	31.18	9.6	City Outfall	
		54	0.013	48	0.0021	106.0	15.90	14.14	1.13	1.08	6.66	0.0029	0.14								
35+06	17.77													0.05	0.034	0.174	21.74	31.18	9.4		
2-36"		36	0.013	26	0.0012	53.0	7.07	9.42	0.75	0.83	7.50	0.0063	0.16								
34+79	17.80													0.05	0.044	0.208	21.95	31.17	9.2		
		54	0.013	70	0.0010	106.0	15.90	14.14	1.13	1.08	6.66	0.0029	0.20								
34+10	17.87													0.05	0.034	0.238	22.19	31.30	9.1		
		54	0.013	31	0.0010	106.0	15.90	14.14	1.13	1.08	6.66	0.0029	0.09								
33+90	17.90													0.05	0.034	0.125	22.31	31.50	9.2		
		54	0.013	203	0.0010	106.0	15.90	14.14	1.13	1.08	6.66	0.0029	0.59								
31+87	18.10													0.05	0.034	0.624	22.94	31.00	8.1		
		54	0.013	227	0.0010	106.0	15.90	14.14	1.13	1.08	6.66	0.0029	0.66								
East Street																					
29+59	18.33													0.05	0.034	0.694	23.63	30.97	7.3	Comb 32	
		54	0.013	40	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.09								
29+20	18.37													0.05	0.028	0.121	23.75	30.90	7.1		
		54	0.013	223	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.52								
26+97	18.59													0.05	0.028	0.548	24.30	31.17	6.9		
		54	0.013	186	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.43								
25+11	18.78													0.05	0.028	0.462	24.76	31.64	6.9		
		54	0.013	78	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.18								
24+34	18.86													0.05	0.028	0.210	24.97	31.73	6.8		
		54	0.013	243	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.57								
21+91	19.10													0.05	0.028	0.595	25.57	31.20	5.6		
		54	0.013	87	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.20								
21+04	19.19													0.05	0.028	0.231	25.80	31.05	5.3		
		54	0.013	416	0.0010	95.0	15.90	14.14	1.13	1.08	5.97	0.0023	0.97								
Holly Drive																					
16+88	19.60													0.05	0.028	0.999	26.80	30.70	3.9	Comb 29	
29x45		36	0.013	79	0.0010	47.5	7.40	9.42	0.79	0.85	6.42	0.0044	0.34								
16+09	19.68													0.05	0.032	0.376	27.17	30.50	3.3		
29x45		36	0.013	84	0.0010	26.1	7.40	9.42	0.79	0.85	3.53	0.0013	0.11								
15+25	19.76													0.05	0.010	0.120	27.29	30.12	2.8		
		36	0.013	300	0.0010	26.1	7.07	9.42	0.75	0.83	3.69	0.0015	0.46								
12+25	20.06													0.05	0.011	0.470	27.76	30.35	2.6		
		36	0.013	194	0.0010	26.1	7.07	9.42	0.75	0.83	3.69	0.0015	0.30								
Court Drive																					
10+31	20.25													0.05	0.011	0.308	28.07	30.30	2.2	Comb 30	
2-24"		24	0.013	143	0.0010	8	3.14	6.28	0.50	0.63	2.55	0.0013	0.18								
8+88	20.39													0.05	0.005	0.184	28.25	30.56	2.3		
2-24"		24	0.013	42	0.0010	8	3.14	6.28	0.50	0.63	2.55	0.0013	0.05								

Grant Line Road between Parker Ave and City Outfall Channel

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	$R^{2/3}$	Velocity	S_f	H_f	K_{minor}	H_{minor}	H_{total}	HGL	Top of Curb	Freeboard	Note
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(14)	(15)	(16)	(17)	
							$PI*(1)^2/4$	$PI*(1)$	(4)/(5)	$(6)^{0.67}$	(3)/(4)	$n/1.49*(4)^{0.045}$	(2)*(9)	(11)	$(11)*(8)^2/2g$	$10+(12)+(13)$	$TW+\Sigma(14)$		(16)-(15)	
8+61	20.43													0.05	0.005	0.058	28.31	30.66	2.3	
2-24"		24	0.013	435	0.0010	8	3.14	6.28	0.50	0.63	2.55	0.0013	0.54							
4+26	20.86													0.05	0.005	0.549	28.86	29.78	0.9	
2-24"		24	0.013	23	0.0013	8	3.14	6.28	0.50	0.63	2.55	0.0013	0.03							
Parker Avenue																				
4+10	20.89													0.05	0.005	0.034	28.90	29.80	0.9	E42

Grant Line Road between Parker Ave and City Outfall Channel



LAMMERS ROAD

DET 10 / 11

9+56

39+22

69+24

78+20

96+23

104+39

111+00

GRANT LINE ROAD

72" SD

2-72" SD

72" SD

47+32

72" SD

48" SD

72" SD

DET 4

30+58

0+50

66" SD

66" SD

42" SD

GRANT LINE ROAD

GRANT LINE ROAD

I-205

UNION PACIFIC RAILROAD

REACH C CHANNEL

CORRAL HOLLOW ROAD

LINCOLN BLVD



Client/Project
CITY OF TRACY
CITYWIDE STORM DRAINAGE
MASTER PLAN

APPENDIX C

Title
**HGL KEY MAP
GRANT LINE RD, WSID MAIN
DRAIN TO TRACY BLVD**

DECEMBER 2010
184010207
SCALE: 1" = 950'

City of Tracy
 Citywide Storm Drainage Master Plan
 Hydraulic Grade Line Calculations
 Grant Line Road between WSID Main Drain and Tracy Blvd, Including Corral Hollow Road and Lowell Avenue

2-Square Mile Area Served by WSID Main Drain
 10-year 24-hour Storm

Tailwater= 19.3 ft

from Main Drain HEC-2 model for 175 cfs,
 FEMA tailwater

All elevations are adjusted for NAVD 88 (For Grant Line Road 3.8', for Corral Hollow 3.0', and for Lowell Ave 2.7')

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _r	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note	
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)	(ft)	(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(14)	(15)	(16)	(17)		
						PI*(1) ² /4	PI*(1)	(4)/(5)	(6) ^{0.67}		(3)/(4))*n*1.49*(4) ^{1/3}	(2) ^{1/9}		(11)*(8) ² /2g	10)+(12)+(13)	TW+Σ(14)		(16)-(15)		
Outfall	11.34		72	160	0.0000	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.29				19.3	26.8	7.5	Main Drain	
Grant Line Road & Lammers Road																					
9+56	11.34		72	394	0.0000	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.71	0.05	0.021	0.309	19.6	27.8	8.2		
13+50	11.34		72	455	0.0000	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.82	0.05	0.021	0.730	20.3	27.0	6.7		
18+05	11.34		72	95	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.17	0.05	0.021	0.840	21.2	27.8	6.6		
19+00	11.41		72	190	0.0008	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.34	0.05	0.021	0.192	21.4	27.8	6.4		
20+90	11.56		72	140	0.0009	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.25	0.05	0.021	0.363	21.7	28.8	7.1		
22+30	11.68		72	423	0.0008	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.76	0.05	0.021	0.273	22.0	28.4	6.3		
26+53	12.01		72	282	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.51	0.05	0.021	0.782	22.8	28.8	6.0		
29+35	12.22		72	500	0.0008	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.90	0.05	0.021	0.528	23.3	28.30	5.0		
34+35	12.60		72	487	0.0008	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.88	0.05	0.021	0.921	24.2	27.10	2.9		
Grant Line Road & Naglee Road																					
39+22	12.98		72	400	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.72	0.05	0.021	0.897	25.1	25.65	0.5		
43+22	13.26		72	400	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.72	0.05	0.021	0.741	25.9	25.80	-0.1		
47+22	13.54		72	400	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.72	0.05	0.021	0.741	26.6	26.70	0.1		
51+22	13.82		72	400	0.0007	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.72	0.05	0.021	0.741	27.4	25.90	-1.5		
55+22	14.10		72	244	0.0019	146.0	28.27	18.85	1.50	1.31	5.16	0.0018	0.44	0.05	0.021	0.741	28.1	26.65	-1.4	CP11MD	
Carl's Jr.																					
57+66	14.56		72	404	0.0009	68.1	28.27	18.85	1.50	1.31	2.41	0.0004	0.16	0.05	0.005	0.460	28.6	26.60	-2.0		
2-72"																					
61+70	14.91		72	95	0.0006	68.1	28.27	18.85	1.50	1.31	2.41	0.0004	0.04	0.05	0.005	0.163	28.7	27.02	-1.7		
2-72"																					
62+65	14.97													0.05	0.005	0.042	28.8	27.60	-1.2		

Grant Line Road between WSID Main Drain and Tracy Blvd
 Including Corral Hollow Road and Lowell Ave to Lincoln Blvd

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note	
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	(4) PI*(1) ² /4	(5) PI*(1)	(6) (4)/(5)	(7) (6) ^{0.67}	(8) (3)/(4)	(9) 1/n*1.49*(4) ^{1/3}	(10) (2) ^{1/3}		(11)	(12) (11) ² /2g	(14) (10)+(12)+(13)	(15) TW+Σ(14)	(16)	(17) (16)-(15)	
2-72"		72	0.016	256	0.0002	68.1	28.27	18.85	1.50	1.31	2.41	0.0004	0.10								
65+21	15.01													0.05	0.005	0.105	28.9	26.67	-2.2		
2-72"		72	0.016	244	0.0003	68.1	28.27	18.85	1.50	1.31	2.41	0.0004	0.10								
67+65	15.08													0.05	0.005	0.100	29.0	27.40	-1.6		
		72	0.016	159	0.0002	68.1	28.27	18.85	1.50	1.31	2.41	0.0004	0.06								31MD
Grant Line Road & Orchard Parkway																					
69+24	15.11													0.05	0.005	0.067	29.0	27.42	-1.6		
		72	0.016	294	0.0003	109	28.27	18.85	1.50	1.31	3.86	0.0010	0.30								
72+18	15.20													0.05	0.012	0.307	29.3	27.34	-2.0		
		72	0.016	480	0.0003	109	28.27	18.85	1.50	1.31	3.86	0.0010	0.48								
77+00	15.34													0.05	0.012	0.493	29.8	27.40	-2.4		
		72	0.016	120	0.0001	109	28.27	18.85	1.50	1.31	3.86	0.0010	0.12								9MD
Grant Line Road & Corral Hollow Road																					
78+20	15.35													0.05	0.012	0.132	30.0	27.30	-2.7		
		72	0.016	257	0.0003	39.5	28.27	18.85	1.50	1.31	1.40	0.0001	0.03								
80+77	15.43													0.05	0.002	0.035	30.0	27.30	-2.7		
		72	0.016	253	0.0005	39.5	28.27	18.85	1.50	1.31	1.40	0.0001	0.03								
83+30	15.55													0.05	0.002	0.035	30.0	27.00	-3.0		
		72	0.016	211	0.0058	39.5	28.27	18.85	1.50	1.31	1.40	0.0001	0.03								
85+41	16.77													0.05	0.002	0.029	30.1	26.25	-3.8		
85+41	17.03																30.1	26.25	-3.8		
		48	0.013	187	0.0014	39.5	12.57	12.57	1.00	1.00	3.14	0.0008	0.14								
87+28	17.29													0.05	0.008	0.149	30.2	27.23	-3.0		
		48	0.013	151	0.0014	39.5	12.57	12.57	1.00	1.00	3.14	0.0008	0.11								
88+79	17.50													0.05	0.008	0.122	30.3	26.81	-3.5		
		48	0.013	338	0.0014	39.5	12.57	12.57	1.00	1.00	3.14	0.0008	0.26								10MD
92+17	17.97													0.05	0.008	0.263	30.6	27.16	-3.4		
		48	0.013	406	0.0013	32	12.57	12.57	1.00	1.00	2.55	0.0005	0.20								
Grant Line Road, About 100 Feet East of Travao Lane																					
96+23	18.5													0.05	0.005	0.207	30.8	27.58	-3.2		
		48	0.013	287	0.0015	32	12.57	12.57	1.00	1.00	2.55	0.0005	0.14								CW77
99+10	18.94													0.05	0.005	0.147	31.0	27.48	-3.5		
		48	0.013	350	0.0014	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.04								
102+60	19.43													0.05	0.001	0.045	31.0	28.53	-2.5		
		48	0.013	179	0.0021	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.02								CW68A
200 Feet West of Lincoln Blvd.																					
104+39	19.80													0.05	0.001	0.023	31.0	29.55	-1.5		
		48	0.013	200	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.02								
105+00	20.05													0.05	0.001	0.026	31.0	28.66	-2.4		
		48	0.013	300	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.04								
106+00	20.43													0.05	0.001	0.038	31.1	29.13	-2.0		
		48	0.013	386	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.05								

**Grant Line Road between WSID Main Drain and Tracy Blvd
Including Corral Hollow Road and Lowell Ave to Lincoln Blvd**

Station	Invert (ft)	Dia (in)	n	Length (ft)	Pipe Slope ft/ft	Flow (cfs)	Area (ft ²)	Wetted P (ft)	Hyd. Rad. (ft)	R ^{2/3} (7) (6) ^{0.67}	Velocity (ft/s)	S _f (9)	H _f (10) (2)*(9)	K _{minor} (11)	H _{minor} (ft) (12) (11)*(8) ² /2g	H _{total} (ft) (14) (10)+(12)+(13)	HGL (ft) (15) TW+Σ(14)	Top of Curb (ft) (16)	Freeboard (ft) (17) (16)-(15)	Note
107+00	20.91	48	0.013	483	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.06	0.05	0.001	0.049	31.1	28.9	-2.2	
108+00	21.52	48	0.013	200	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.02	0.05	0.001	0.061	31.2	29.02	-2.2	
109+00	21.77	48	0.013	532	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.07	0.05	0.001	0.026	31.2	28.73	-2.5	
110+00	22.45	48	0.013	199	0.0013	16	12.57	12.57	1.00	1.00	1.27	0.0001	0.02	0.05	0.001	0.067	31.3	27.67	-3.6	
111+00	22.70													0.05	0.001	0.026	31.3	28.22	-3.1	
Corral Hollow Road South of Grant Line Road																				
54+30	16.20	48	0.013	432	0.0022	40.9	12.57	12.57	1.00	1.00	3.25	0.0008	0.35				29.8	27.00	-2.8	
50+00	17.15	48	0.013	269	0.0017	40.9	12.57	12.57	1.00	1.00	3.25	0.0008	0.22	0.05	0.008	0.358	30.2	27.57	-2.6	
Corral Hollow Road and Alegre Drive																				
47+32	17.60	48	0.013	583	0.0015	26.6	12.57	12.57	1.00	1.00	2.12	0.0003	0.20	0.05	0.008	0.226	30.4	28.8	-1.6	8MD
41+50	18.46	48	0.013	400	0.0016	26.6	12.57	12.57	1.00	1.00	2.12	0.0003	0.14	0.05	0.003	0.203	30.6	30.77	0.1	
37+56	19.09	48	0.013	379	0.0016	26.6	12.57	12.57	1.00	1.00	2.12	0.0003	0.13	0.05	0.003	0.141	30.8	31.95	1.2	
33+70	19.69	48	0.013	312	0.0015	26.6	12.57	12.57	1.00	1.00	2.12	0.0003	0.11	0.05	0.003	0.133	30.9	32.56	1.7	
30+58	20.15	66	0.013	69	0.0051	26.6	23.76	17.28	1.38	1.24	1.12	0.0001	0.00	0.05	0.003	0.110	31.0	32.80	1.8	
29+89	20.50													0.05	0.001	0.005	31.0	33.70	2.7	
Intersection of Corral Hollow Road and Lowell Ave.																				
0+50	21.61	66	0.013	44	0.0020	25.5	23.76	17.28	1.38	1.24	1.07	0.0001	0.00				31.0	32.00	1.0	7MD
0+94	21.70	66	0.013	520	0.0019	25.5	23.76	17.28	1.38	1.24	1.07	0.0001	0.03	0.05	0.001	0.003	31.0	32.52	1.5	
6+14	22.69	66	0.013	386	0.0019	25.5	23.76	17.28	1.38	1.24	1.07	0.0001	0.02	0.05	0.001	0.031	31.0	33.97	2.9	6MD
10+00	23.41	66	0.013	350	0.0019	25.5	23.76	17.28	1.38	1.24	1.07	0.0001	0.02	0.05	0.001	0.023	31.1	32.82	1.8	
13+50	24.08	66	0.013	540	0.0020	22.7	23.76	17.28	1.38	1.24	0.96	0.0000	0.02	0.05	0.001	0.021	31.1	33.87	2.8	
Lowell Ave Across from High School																				
18+80	25.16	66	0.013	300	0.0019	22.7	23.76	17.28	1.38	1.24	0.96	0.0000	0.01	0.05	0.001	0.025	31.1	34.24	3.1	
4+37	25.74													0.05	0.001	0.014	31.1	33.54	2.4	5MD

**Grant Line Road between WSID Main Drain and Tracy Blvd
Including Corral Hollow Road and Lowell Ave to Lincoln Blvd**

Station	Invert	Dia	n	Length	Pipe Slope	Flow	Area	Wetted P	Hyd. Rad.	R ^{2/3}	Velocity	S _f	H _f	K _{minor}	H _{minor}	H _{total}	HGL	Top of Curb	Freeboard	Note
	(ft)	(in)		(ft)	ft/ft	(cfs)	(ft ²)	(ft)	(ft)		(ft/s)		(ft)		(ft)	(ft)	(ft)	(ft)	(ft)	
		(1)		(2)		(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(14)	(15)	(16)	(17)	
							$\pi \cdot (1)^2 / 4$	$\pi \cdot (1)$	(4)/(5)	$(6)^{0.67}$	(3)/(4)	$(9) \cdot \pi \cdot 1.49 \cdot (4) \cdot (7)$	(2) \cdot (9)		$(11) \cdot (8)^2 / 2g$	$(10) + (12) + (13)$	$TW + \Sigma(14)$		(16)-(15)	
		66	0.013	455	0.0016	37.3	23.76	17.28	1.38	1.24	1.57	0.0001	0.06							
Lowell Ave and Lincoln Blvd.																				
8+92	26.45													0.05	0.002	0.058	31.2	33.70	2.5	
		66	0.013	40	0.0010	37.3	23.76	17.28	1.38	1.24	1.57	0.0001	0.00							
9+32	26.49													0.05	0.002	0.007	31.2	33.70	2.5	
		42	0.013	10	0.0010	37.3	9.62	11.00	0.88	0.91	3.88	0.0014	0.01							
9+42	26.50													0.05	0.012	0.025	31.2	33.70	2.5	4MD

* For design purposes, where the freeboard shown is negative, assume the HGL is equivalent to the top of curb plus 0.5 feet

**Grant Line Road between WSID Main Drain and Tracy Blvd
Including Corral Hollow Road and Lowell Ave to Lincoln Blvd**

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1*****
* HEC-2 WATER SURFACE PROFILES *
* * * * *
* Version 4.6.2; May 1991 *
* * * * *
* RUN DATE 02DEC10 TIME 12:37:17 *
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET, SUITE D *
* DAVIS, CALIFORNIA 95616-4687 *
* (916) 756-1104 *
*****

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1 02DEC10 12:37:17 PAGE 1

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THIS RUN EXECUTED 02DEC10 12:37:17

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*****
HEC-2 WATER SURFACE PROFILES
Version 4.6.2; May 1991
*****

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T1 CITY OF TRACY
T2 STORM DRAINAGE MASTER PLAN
T3 EASTSIDE CHANNEL
T4 HEC-2 FILE (ESIDECH1) DECEMBER 2010
T5 STANTEC PROJECT NO. 184010207
T6 100-YEAR 24-HOUR STORM ANALYSIS

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***** FILE ESIDECH1 IS COPIED FROM 6ESCHANL *****

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***** Feb 19, 2010 *****
***** ELEVATIONS ARE ADJUSTED FOR NAVD 88 *****
***** 2.7 FEET IS ADDED TO ALL PLAN ELEVATIONS *****

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15
652.21 EXAMPLE: X1652.21 = PLAN STATION 6+52.2 LINE A. (2= LINE B, 3= LINE C
652.21 AND 4=LINE D)
14001 CROSS SECTION GEOMETRY WAS OBTAINED FROM REIMER ASSOC. X-SECTION
14001 SHEETS CAX-1 THROUGH CDX-5.
19761 ARBOR AVE. CULVERT CROSSING
36541 FREEWAY UNDERPASS
36541 PORTION OF REACH A AND ALL OF REACH B IS CONCRETE SIDE SLOPE
22522 MACARTHUR DR. CROSSING W/LARGE SIZE CULVERT 6#24 FT. ARCH-PIPE
19363 PESCADERO AVE. CROSSING W/LARGE SIZE CULVERT 6#24 FT. ARCH-PIPE
10254 GRANT LINE RD. CROSSING (DUE TO LONG LENGTH AND 4-45 DEGREE BENDS
10254 THE BOX CULVERTS WERE NOT INCLUDED IN THE HEC-2 MODEL. THE 100 YR.
10254 WATER SURFACE ELEVS. THROUGH THE BOX CULVERTS WERE CALCULATED
10254 BY HAND AND ADDED TO THE D/S W.S.E.)
16394 RAILROAD CROSSING
65004 11TH STREET
2.7 FEET IS ADDED TO ALL ELEVATIONS TO GET TO NAVD 88

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J1 ICHECK INQ NINV IDIR STRT METRIC HVINS Q WSEL FQ
      2 6.60
J2 NPROF IPLOT PRFVS XSECV XSECH FN ALLDC IBW CHNIM ITRACE
      -1 -1
J3 VARIABLE CODES FOR SUMMARY PRINTOUT
      38 1 26 42 8 4 3 43 23 24

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1 02DEC10 12:37:17 PAGE 2

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53 54
J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****
      -10 -10

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QT 1 784
NC .05 .05 0.035 .1 .3

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***** SUGAR CUT OUTFALL *****										
X1	652.21	7	974	1021.5	0	0	0		2.7	
GR	8.0	965.5	3.0	974.0	2.0	981.5	0.0	1000	2.0	1018.5
GR	3.0	1021.5	10.0	1036.0						
NC	.05	.05	.02							
X1	658.21	6	981.5	1018.5	6	6	6		2.7	
GR	11.0	964.5	5.0	981.5	1.0	995.0	1.0	1005.0	5.0	1018.5
GR	11.0	1035.0								
X1	673.21	6	981.5	1018.5	15	15	15		2.7	
GR	15.25	980.5	15.25	981.5	6.25	995.0	6.25	1005.0	15.25	1018.5
GR	15.25	1019.5								
NC	.05	.05	.035							
X1	748.21	6	981.5	1018.5	75	75	75		2.7	
GR	19.0	970.0	15.4	981.5	6.4	995.0	6.4	1005.0	15.4	1018.5
GR	18.0	1028.0								
X1	8401	7	982.0	1018.0	91.8	91.8	91.8		2.7	
GR	15.5	965.0	15.0	982.0	6.58	995.0	6.58	1005.0	15.0	1018.0
GR	16.0	1030.0	17.5	1055.0						
X1	14001	7	983.0	1018.0	560	560	560		2.7	
GR	16.4	976.0	16.3	983.0	7.7	995.0	7.7	1005.0	16.0	1018.0
GR	17.7	1023.0	18.0	1034.0						
X1	19221	6	978	1020	522	522	522		2.7	
GR	17.2	940.0	17.0	978.0	8.7	995.0	8.7	1005.0	17.6	1020.0
GR	18.8	1034.0								

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NC	.05	.05	.016	0.3	0.5					
X1	19621	4	977.0	1023.0	40	40	40		2.7	
GR	17.0	977.0	8.8	989.5	8.8	1011.0	17.0	1023.0		
X1	19711	13	988.0	1012.0	9	9	9		2.7	
X3	10							17.7	17.7	
GR	16.8	988.0	8.8	988.0	8.8	989.0	8.8	991.0	8.8	993.0
GR	8.8	996.0	8.8	1000.0	8.8	1004.0	8.8	1007.0	8.8	1009.0
GR	8.8	1011.0	8.8	1012.0	16.8	1012.0				

***** ARBOR AVENUE CULVERT CROSSING *****										
X1	19761	0	0	0	5	5	5			
BT	11	988.0	19.5	13.5	989.0	19.5	15.0	991.0	19.5	16.3
BT	993.0	19.5	16.7	996.0	19.5	17.3	1000.0	19.5	17.5	1004.0
BT	19.5	17.3	1007.0	19.5	16.7	1009.0	19.5	16.3	1011.0	19.5
BT	15.0	1012.0	19.5	13.5						
X1	20231	0	0	0	47	47	47		0.1	
X2							1			
X1	20281	0	0	0	5	5	5			
X3	10							18.2	17.7	
X1	20371	4	970.0	1021.0	9	9	9		2.7	
GR	18.0	970.0	8.9	988.5	8.9	1011.5	18.0	1021.0		
QT	1	759								
NC	.05	.05	.035	0.1	0.3					
X1	20781	10	971.0	1025.0	41	41	41		2.7	
GR	19.3	964.0	18.8	971.0	9.0	991.0	9.0	1009.0	11.0	1011.0
GR	11.0	1013.0	17.0	1025.0	17.3	1030.0	18.0	1035.0	18.2	1040.0
X1	30001	8	969.0	1024.0	922	922	922		2.7	
GR	21.1	940.0	20.5	954.0	20.8	969.0	9.9	991.0	9.9	1009.0
GR	14.5	1014.0	20.0	1024.0	20.5	1030.0				
NC	.02	.02	0.02							
QT	1	574								

***** I-205 OVERPASS, NO IMPACT ON CHANNEL HYDRAULICS *****										
X1	36541	4	991.0	1009.0	654	654	654		2.7	
GR	26.1	960.0	10.5	991.0	10.5	1009.0	20.3	1019.0		

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NC	0.013	0.013	0.013							
X1	37741	5	991.0	1009.0	110	122	130		2.7	
GR	26.0	977.0	22.8	980.0	10.6	991.0	10.6	1009.0	20.8	1020.0
NC				0.3	0.5					
X1	38501	6	994.0	1006.0	76	76	76		2.7	
GR	23.3	940.0	21.6	983.0	10.7	994.0	10.7	1006.0	16.8	1012.0
GR	23.5	1026.0								
X1	39271	6	994.0	1006.0	87	77	68		2.7	
GR	21.9	979.0	19.7	985.0	10.8	994.0	10.8	1006.0	19.7	1015.0
GR	26.3	1037.0								
X1	41471	7	994.0	1006.0	220	220	220		2.7	
GR	24.4	967.0	17.2	988.0	11.0	994.0	11.0	1006.0	20.4	1018.0
GR	24.1	1036.0	24.3	1040.0						

NC	0.013	0.013	0.013							
QT	1	256								
***** UPSTREAM OF CITY OUTFALL CHANNEL *****										
X1	42641	4	0	26	150	90	117		2.7	
GR	19.2	0	11.2	8	11.2	18	19.2	26		
X1	16002	4	0	26	636	636	636		2.7	
GR	19.8	0	11.8	8	11.8	18	19.8	26		
X1	21982	4	0	26	598	598	598		2.7	
GR	20.4	0	12.4	8	12.4	18	20.4	26		
NC				0.3	0.5					
X1	22372	4	984.5	1015.5	39	39	39		2.7	
GR	20.5	984.5	12.4	992.5	12.4	1007.5	20.5	1015.5		
X1	22472	13	992.0	1016.0	10	10	10		2.7	
X3	10							20.7	20.7	
GR	21.5	992.0	12.4	992.0	12.4	993.0	12.4	995.0	12.4	997.0
GR	12.4	1000.0	12.4	1004.0	12.4	1008.0	12.4	1011.0	12.4	1013.0
GR	12.4	1015.0	12.4	1016.0	21.5	1016.0				

***** MACARTHUR DRIVE CULVERT CROSSING *****

1 02DEC10 12:37:17 PAGE 5

X1	22522	0	0	0	5	5	5			
BT	11	992.0	24.2	17.1	993.0	24.2	18.6	995.0	24.2	19.9
BT	997.0	24.2	20.3	1000.0	24.2	20.7	1004.0	24.2	21.1	1008.0
BT	24.2	20.7	1011.0	24.2	20.3	1013.0	24.2	19.9	1015.0	24.2
BT	18.6	1016.0	24.2	17.1						
X1	23442	0	0	0	92	92	92		0.1	
X2							1			
X1	23492	0	0	0	5	5	5		21.2	21.2
X3	10									
X1	23592	4	0	26	101	10	10		2.7	
GR	20.5	0	12.5	8	12.5	18	20.5	26		
NC				0.1	0.3					
X1	23992	4	0	26	40	40	40		2.7	
GR	20.6	0	12.6	8	12.6	18	20.6	26		
X1	30002	4	0	26	601	601	601		2.7	
GR	21.2	0	13.2	8	13.2	18	21.2	26		
X1	35552	4	0	26	555	555	555		2.7	
GR	21.3	0	13.3	8	13.3	18	21.3	26		

***** SCHACK DESIGN CHANNEL AROUND FUTURE OUTLET MALL EXPANSION SITE *****

NC	0.027	0.027	0.027							
STA.10+00										
X1	1000	4	0	27.4	1	1	1		2.7	
GR	22.01	0	13.3	8.7	13.3	18.7	22.01	27.4		
STA.11+50										

X1	1150	4	0	40	150	150	150		2.7
GR	22.12	0	13.4	12	13.4	28	22.12	40	

STA.13+00

X1	1300	4	0	55	150	150	150		2.7
GR	22.22	0	13.51	17	13.51	38	22.22	55	

STA.18+00

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X1	1800	4	0	55	500	500	500		2.7
GR	22.57	0	13.86	17	13.86	38	22.57	55	

SRA.19+22

X1	1922	4	0	55	101	144	122		2.7
GR	22.68	0	13.95	17	13.95	38	22.68	55	

STA.20+50

X1	2050	4	0	55	106	149	127		2.7
GR	22.78	0	14.05	17	14.05	38	22.77	55	

STA.23+00

X1	2300	4	0	55	250	250	250		2.7
GR	22.94	0	14.23	17	14.23	38	22.94	55	

STA.27+50

X1	2750	4	0	55	450	450	450		2.7
GR	23.26	0	14.55	17	14.55	38	23.26	55	

STA.28+55

X1	2855	4	0	55	84	128	105		2.7
GR	23.33	0	14.62	17	14.62	38	23.33	55	

STA.29+57

X1	2957	4	0	55	80	124	102		2.7
GR	23.40	0	14.69	17	14.69	38	23.40	55	

STA.33+50

X1	3350	4	0	55	393	393	393		2.7
GR	23.69	0	14.98	17	14.98	38	23.69	55	

STA.36+00

X1	3600	4	0	55	250	250	250		2.7
GR	23.86	0	15.15	17	15.15	38	23.86	55	

STA.36+75

X1	3675	4	0	62	75	75	75		2.7
GR	23.91	0	15.2	26	15.2	49	23.91	62	

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QT	1	275								
NC	0.05	0.05	0.016	0.3	0.5					
X1	19313	13	992	1008	42	42	42		2.7	
X3	10									
GR	23.23	992	15.25	992	15.23	993	15.23	995	15.23	997
GR	15.23	1000	15.23	1004	15.23	1008	15.23	1011	15.23	1013
GR	15.23	1015	15.23	1016	23.23	1016				

***** PESCADERO AVENUE CULVERT CROSSING *****

X1	19363				5	5	5			
BT	-11	992	25.93	19.93	993	25.93	21.43	995	25.93	22.73
BT		997	25.93	23.13	1000	25.93	23.73	1004	25.93	23.93
BT		1008	25.93	23.73	1011	25.93	23.43	1013	25.93	22.73
BT		1015	25.93	21.43	1016	25.93	19.93			

X1	19913				55	55	55		
X2							1		

X1	19963				5	5	5		
X3	10								

X1	20113	4	979	1021	15	15	15		2.7
GR	22.23	979	15.23	993	15.23	1007	22.23	1021	

NC	0.05	0.05	0.035	0.1	0.3						
X1	20443	6	982	1030	33	33	33			2.7	
GR	22.33	982	15.23	996	15.23	1004	18.43	1010.5	18.43		1014
GR	23.23	1030									

X1	29003	7	978.0	1020.0	856	856	856			2.7	
GR	24.7	978.0	18.1	989.0	18.0	992.0	16.4	996.0	16.4		1004.0
GR	24.7	1020.0	26.0	1022.0							

QT	1	232									
X1	33003	4	978.0	1019.0	400	400	400			2.7	
GR	26.1	978.0	17.2	996.0	17.2	1004.0	24.4	1019.0			

X1	39003	6	976.0	1024.0	600	600	600			2.7	
GR	28.5	965.0	28.6	976.0	18.5	996.0	18.5	1004.0	29.1		1024.0
GR	29.2	1034.0									

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X1	44703	4	974	1026	570	570	570			2.7	
GR	30.9	974	19.7	996	19.7	1004	30.9	1026			

X1	45003	4	971	1029	35	35	35			2.7	
GR	30.9	971	19.8	993.5	19.8	1006.5	30.9	1029			

***** GRANT LINE ROAD CULVERT CROSSING *****
 ***** HEAD LOSS THROUGH CULVERT CALCULATED BY HAND AND ADDED T *****

QT	1	207									
X1	10254	4	971.5	1028.5	406	406	406			2.7	
X5	1	27.00									
GR	33	971.5	22	993.5	22	1006.5	33	1028.5			

X1	11554	7	977	1020	140	140	140			2.7	
GR	33.3	973	33	977	23.6	996	22.9	1000	23.6		1004
GR	31.8	1020	32.5	1033							

X1	14454	7	975	1020	290	290	290			2.7	
GR	33.5	967	33.4	975	24	996	23.3	1000	24		1004
GR	32.2	1020	33	1033							

NC	.05	.05	.02	.3	.5						
X1	14794	4	973	1027	34	34	34			2.7	
GR	33	973	23.4	992	23.4	1008	33	1027			

X1	14964	4	991	1009	17	17	17			2.7	
X3	10										
GR	33.8	991	23.4	991.1	23.4	1008.5	33.8	1009			

***** RAILROAD CULVERT CROSSING *****

SB	1.25	1.56	3	0	18	2	80	0	26.3	26.1	
X1	16394	0	0	0	143	143	143				
X2	0	0	1	31.1	36.5						
X3	10										
BT	2	991	36.5	31.1	1009	36.5	31.1				

NC	.05	.05	.035	.1	.3						
X1	16564	4	973	1027	17	17	17			2.7	
GR	33	973	23.6	992	23.6	1008	33	1027			

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X1	16904	5	977.5	1018.5	34	34	34			2.7	
GR	33.6	977.5	24.3	996	23.6	1000	24.3	1004	33.6		1018.5

X1	24044	5	977.2	1022.8	714	714	714			2.7	
GR	34.6	977.2	25.2	996	24.5	1000	25.2	1004	34.6		1022.8

X1	25504	5	977.2	1022.8	265	320	146			2.7	
GR	34.7	977.2	25.3	996	24.6	1000	25.3	1004	34.6		1022.8

X1	25524	5	977.2	1022.8	2	2	2			2.7	
GR	36.6	977.2	27.2	996	26.5	1000	27.2	1004	36.3		1022.8

QT	1	116									
NC	0.05	0.05	0.05								
X1	26604	7	964.4	1022.5	195	245	108		2.7		
GR	36	964.4	32.5	974.9	32.3	982.9	27.6	997	27.1	1000	
GR	27.6	1003	35.4	1022.5							
X1	33804	7	966	1020.8	680	720	750		2.7		
GR	47.2	966	33.5	977	33.3	985	29.3	997	28.8	1000	
GR	29.3	1003	36.4	1020.8							
X1	40004	7	969	1021.3	620	620	620		2.7		
GR	37.6	969	34.9	977	34.7	985	30.7	997	30.2	1000	
GR	30.7	1003	38.2	1021.3							
X1	41504	7	966.6	1023.3	150	150	150		2.7		
GR	38.2	966.6	36	973.2	35.8	981.2	31.2	995	30.3	1000	
GR	31.2	1005	38.5	1023.3							
X1	44504	7	966.3	1022.3	300	300	300		2.7		
GR	38.7	966.3	36.5	972.9	36.3	980.9	31.6	995	30.8	1000	
GR	31.6	1005	38.5	1022.3							
X1	46004	7	965.6	1023.3	150	150	150		2.7		
GR	39.3	965.6	36.4	974.3	36.2	982.3	32.2	994.3	31.2	1000	
GR	32.2	1005.8	39.2	1023.3							

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X1	49004	7	966.8	1022.6	296	304	300		2.7		
GR	40.2	966.8	37.7	974.3	37.5	982.3	33.5	994.3	32.5	1000	
GR	33.5	1005.8	40.2	1022.6							
X1	52504	7	968	1022.1	350	350	350		2.7		
GR	41	968	38.9	974.3	38.7	982.3	34.7	994.3	33.7	1000	
GR	34.7	1005.8	41.2	1022.1							
X1	56004	7	968.9	1021.1	368	340	350		2.7		
GR	42.4	968.9	41.3	972.2	41.1	980.2	36.4	994.3	35.4	1000	
GR	36.4	1005.8	42.5	1021.1							
X1	64404	7	970.7	1020.6	840	840	840		2.7		
GR	45.2	970.7	43.9	974.6	43.7	982.6	39.8	994.3	38.8	1000	
GR	39.8	1005.8	45.7	1020.6							
X1	65004	7	970.7	1020.6	60	60	60		2.7		
GR	45.5	970.7	44.2	974.6	44	982.6	40.1	994.3	39.1	1000	
GR	40.1	1005.8	46	1020.6							

1 ***** 11TH STREET OVERPASS *****
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THIS RUN EXECUTED 02DEC10 12:37:17

 HEC-2 WATER SURFACE PROFILES
 Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

ASTSIDE CHANNEL
 SUMMARY PRINTOUT

SECNO	CWSEL	VCH	ELMIN	DEPTH	TOPWID	EG	Q	XLBEL	RBEL	SSTA	ENDST
652.210	6.60	6.41	2.70	3.90	50.89	7.24	784.00	5.70	5.70	972.47	1023.36
* 658.210	7.52	8.96	3.70	3.82	35.79	8.77	784.00	7.70	7.70	982.10	1017.90
* 673.210	13.49	10.28	8.95	4.54	23.62	15.13	784.00	17.95	17.95	988.19	1011.81
748.210	14.70	7.61	9.10	5.60	26.79	15.60	784.00	18.10	18.10	986.60	1013.40

8401.000	15.43	6.54	9.28	6.15	29.00	16.09	784.00	17.70	17.70	985.50	1014.50
14001.000	17.44	5.45	10.40	7.04	30.85	17.90	784.00	19.00	18.70	985.18	1016.03
19221.000	18.67	4.58	11.40	7.27	37.12	18.99	784.00	19.70	20.30	980.12	1017.24
* 19621.000	18.89	3.26	11.50	7.39	43.58	19.05	784.00	19.70	19.70	978.24	1021.81
* 19711.000	18.82	4.46	11.50	7.32	24.00	19.13	784.00	19.50	19.50	988.00	1012.00
* 19761.000	18.66	6.41	11.50	7.16	24.00	19.30	784.00	19.50	19.50	988.00	1012.00
20231.000	18.73	6.41	11.60	7.13	24.00	19.37	784.00	19.60	19.60	988.00	1012.00
* 20281.000	19.19	4.30	11.60	7.59	24.00	19.48	784.00	19.60	19.60	988.00	1012.00
* 20371.000	19.40	2.87	11.60	7.80	47.00	19.53	784.00	20.70	20.70	972.64	1019.64
* 20781.000	19.40	2.96	11.70	7.70	49.15	19.54	759.00	21.50	19.70	975.27	1024.42
30001.000	20.01	3.42	12.60	7.41	43.08	20.20	759.00	23.50	22.70	976.04	1019.12
* 36541.000	20.27	3.39	13.20	7.07	39.27	20.42	574.00	13.20	13.20	976.95	1016.22
37741.000	20.25	3.86	13.30	6.95	31.76	20.45	574.00	13.30	13.30	984.74	1016.49

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SECNO	CWSEL	VCH	ELMIN	DEPTH	TOPWID	EG	Q	XLBEL	RBEL	SSTA	ENDST
* 38501.000	20.16	5.49	13.40	6.76	26.21	20.55	574.00	13.40	13.40	987.18	1013.38
39271.000	20.17	5.59	13.50	6.67	25.50	20.57	574.00	13.50	13.50	987.25	1012.75
41471.000	20.22	5.56	13.70	6.52	27.24	20.62	574.00	13.70	13.70	987.08	1014.32
42641.000	20.65	2.26	13.90	6.75	23.50	20.73	256.00	21.90	21.90	1.25	24.75
16002.000	20.69	2.56	14.50	6.19	22.37	20.79	256.00	22.50	22.50	1.81	24.19
21982.000	20.73	2.91	15.10	5.63	21.26	20.87	256.00	23.10	23.10	2.37	23.63
* 22372.000	20.81	2.17	15.10	5.71	26.29	20.89	256.00	23.20	23.20	986.86	1013.14
22472.000	20.84	1.86	15.10	5.74	24.00	20.89	256.00	24.20	24.20	992.00	1016.00
* 22522.000	20.83	2.13	15.10	5.73	24.00	20.90	256.00	24.20	24.20	992.00	1016.00
23442.000	20.84	2.14	15.20	5.64	24.00	20.91	256.00	24.30	24.30	992.00	1016.00
* 23492.000	20.86	1.88	15.20	5.66	24.00	20.92	256.00	24.30	24.30	992.00	1016.00
* 23592.000	20.82	2.91	15.20	5.62	21.25	20.96	256.00	23.20	23.20	2.38	23.62
23992.000	20.82	2.98	15.30	5.52	21.05	20.96	256.00	23.30	23.30	2.47	23.53
30002.000	20.89	3.42	15.90	4.99	20.00	21.07	256.00	23.90	23.90	3.00	23.00
35552.000	21.00	3.42	16.00	5.00	19.99	21.18	256.00	24.00	24.00	3.01	22.99
* 1000.000	21.00	3.41	16.00	5.00	19.99	21.18	256.00	24.71	24.71	3.70	23.70
* 1150.000	21.19	2.19	16.10	5.09	30.01	21.26	256.00	24.82	24.82	5.00	35.00
1300.000	21.26	1.64	16.21	5.05	40.74	21.30	256.00	24.92	24.92	7.13	47.87
1800.000	21.34	1.77	16.56	4.78	39.66	21.39	256.00	25.27	25.27	7.67	47.33
1922.000	21.36	1.80	16.65	4.71	39.36	21.41	256.00	25.38	25.38	7.82	47.18
2050.000	21.39	1.84	16.75	4.64	39.08	21.44	256.00	25.48	25.47	7.96	47.05
2300.000	21.45	1.90	16.93	4.52	38.62	21.50	256.00	25.64	25.64	8.19	46.81
2750.000	21.56	2.03	17.25	4.31	37.78	21.62	256.00	25.96	25.96	8.61	46.39
2855.000	21.59	2.05	17.32	4.27	37.66	21.65	256.00	26.03	26.03	8.67	46.33
2957.000	21.62	2.07	17.39	4.23	37.51	21.69	256.00	26.10	26.10	8.74	46.26
3350.000	21.75	2.18	17.68	4.07	36.84	21.82	256.00	26.39	26.39	9.08	45.92

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SECNO	CWSEL	VCH	ELMIN	DEPTH	TOPWID	EG	Q	XLBEL	RBEL	SSTA	ENDST
3600.000	21.84	2.24	17.85	3.99	36.53	21.91	256.00	26.56	26.56	9.23	45.77
3675.000	21.88	2.02	17.90	3.98	40.78	21.94	256.00	26.61	26.61	14.15	54.93
19313.000	21.83	3.77	17.93	3.90	24.00	22.02	275.00	25.93	17.93	992.00	1016.00

19363.000	21.82	3.86	17.93	3.89	24.00	22.03	275.00	25.93	17.93	992.00	1016.00
19913.000	21.85	3.85	17.93	3.92	24.00	22.05	275.00	25.93	17.93	992.00	1016.00
19963.000	21.86	3.74	17.93	3.93	24.00	22.05	275.00	25.93	17.93	992.00	1016.00
20113.000	21.92	3.14	17.93	3.99	29.95	22.07	275.00	24.93	24.93	985.03	1014.97
* 20443.000	21.85	4.23	17.93	3.92	28.15	22.13	275.00	25.03	25.93	988.26	1016.41
* 29003.000	23.72	3.09	19.10	4.62	28.78	23.87	275.00	27.40	27.40	984.13	1012.91
33003.000	24.27	3.13	19.90	4.37	25.94	24.42	232.00	28.80	27.10	987.16	1013.11
39003.000	25.28	3.58	21.20	4.08	23.78	25.48	232.00	31.30	31.80	987.92	1011.70
44703.000	26.47	3.57	22.40	4.07	23.97	26.66	232.00	33.60	33.60	988.02	1011.98
* 45003.000	26.61	2.64	22.50	4.11	29.67	26.72	232.00	33.60	33.60	985.16	1014.84
* 10254.000	27.00	5.11	24.70	2.30	22.20	27.41	207.00	35.70	35.70	988.90	1011.10
* 11554.000	28.35	7.49	25.60	2.75	16.16	29.23	207.00	35.70	34.50	991.85	1008.01
* 14454.000	30.39	3.40	26.00	4.39	23.46	30.57	207.00	36.10	34.90	987.75	1011.21
* 14794.000	30.56	1.87	26.10	4.46	33.67	30.62	207.00	35.70	35.70	983.17	1016.83
* 14964.000	30.54	2.66	26.10	4.44	17.66	30.65	207.00	36.50	36.50	991.06	1008.71
16394.000	30.57	2.64	26.10	4.47	17.66	30.67	207.00	36.50	36.50	991.06	1008.71
16564.000	30.63	1.93	26.30	4.33	33.50	30.69	207.00	35.70	35.70	983.25	1016.75
* 16904.000	30.54	3.88	26.30	4.24	20.55	30.77	207.00	36.30	36.30	988.96	1009.52
24044.000	31.98	3.01	27.20	4.78	24.32	32.12	207.00	37.30	37.30	987.84	1012.16
25504.000	32.19	2.89	27.30	4.89	24.85	32.32	207.00	37.40	37.30	987.62	1012.47
* 25524.000	31.96	7.43	29.20	2.76	16.36	32.81	207.00	39.30	39.00	991.89	1008.25
* 26604.000	33.53	2.33	29.80	3.73	23.79	33.61	116.00	38.70	38.10	987.30	1011.09
33804.000	35.31	2.26	31.50	3.81	24.20	35.39	116.00	49.90	39.10	987.08	1011.29

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SECNO	CWSEL	VCH	ELMIN	DEPTH	TOPWID	EG	Q	XLBEL	RBEL	SSTA	ENDST
40004.000	36.71	2.27	32.90	3.81	24.01	36.79	116.00	40.30	40.90	987.07	1011.08
41504.000	37.00	1.87	33.00	4.00	27.08	37.06	116.00	40.90	41.20	985.70	1012.77
44504.000	37.42	1.87	33.50	3.92	27.19	37.48	116.00	41.40	41.20	985.64	1012.82
46004.000	37.65	2.00	33.90	3.75	26.56	37.71	116.00	42.00	41.90	986.08	1012.65
* 49004.000	38.35	2.69	35.20	3.15	23.31	38.46	116.00	42.90	42.90	987.87	1011.18
52504.000	39.62	2.59	36.40	3.22	23.71	39.72	116.00	43.70	43.90	987.65	1011.36
56004.000	41.05	3.01	38.10	2.95	22.21	41.19	116.00	45.10	45.20	988.46	1010.68
64404.000	44.68	2.64	41.50	3.18	23.53	44.79	116.00	47.90	48.40	987.75	1011.28
65004.000	44.91	2.75	41.80	3.11	23.10	45.02	116.00	48.20	48.70	987.98	1011.08

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SUMMARY OF ERRORS AND SPECIAL NOTES

CAUTION SECNO= 658.210 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 658.210 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 658.210 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

CAUTION SECNO= 673.210 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 673.210 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 673.210 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

WARNING SECNO= 19621.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 19711.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 19761.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 20281.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 20371.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 20781.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 36541.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 38501.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 22372.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 22522.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 23492.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 23592.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 1000.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 1150.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 20443.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 29003.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 45003.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 NOTE SECNO= 10254.000 PROFILE= 1 WSEL BASED ON X5 CARD
 WARNING SECNO= 10254.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 CAUTION SECNO= 11554.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
 CAUTION SECNO= 11554.000 PROFILE= 1 MINIMUM SPECIFIC ENERGY
 WARNING SECNO= 14454.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 14794.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

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 02DEC10 12:37:17

WARNING SECNO= 14964.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 16904.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 CAUTION SECNO= 25524.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
 CAUTION SECNO= 25524.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
 CAUTION SECNO= 25524.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL
 WARNING SECNO= 26604.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
 WARNING SECNO= 49004.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

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1*****
* HEC-2 WATER SURFACE PROFILES *
* * * * *
* Version 4.6.2; May 1991 *
* * * * *
* RUN DATE 02DEC10 TIME 12:37:29 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET, SUITE D *
* DAVIS, CALIFORNIA 95616-4687 *
* (916) 756-1104 *
*****

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X X XXXXXXXX XXXXX XXXXX
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X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXXXXXXX

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THIS RUN EXECUTED 02DEC10 12:37:29

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*****
HEC-2 WATER SURFACE PROFILES
Version 4.6.2; May 1991
*****

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T1 CITY OF TRACY
T2 STORM DRAINAGE MASTER PLAN
T3 WESTSIDE CHANNEL SYSTEM (OPEN CHANNEL SEGMENTS)
T4 HEC2 FILE (WSIDECH1) DECEMBER 2010
T5 STANTEC PROJECT NO. 184010207
T6 100-YEAR 24-HOUR STORM ANALYSIS

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*****
***** FEB 26, 2010*****
***** ELEVATIONS ARE ADJUSTED FOR NAVD 88 *****
***** 2.75 IS ADDED TO ALL ELEVATIONS *****

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J1 ICHECK INQ NINV IDIR STRT METRIC HVINS Q WSEL FQ
      2 37.5
J2 NPROF IPLOT PRFVS XSECV XSECH FN ALLDC IBW CHNIM ITRACE
      -1 -1
J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****
      -10 -10

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QT 1 202
NC 0.05 0.05 0.05 0.1 0.3
!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!
***** OCTOBER 30, 2009 *****
***** CHANNEL REACH "C" BYRON ROAD TO DET 5 *****
STA 60+50
X1 50 7 0 59 200 200 200 2.75
GR 35.37 0 32.37 11 32.37 21 29.37 29 29.37 40.5
GR 35.37 59 35.37 60
STA 62+00
X1 150 7 0 59 150 150 150 2.75
GR 36.17 0 33.17 11 33.17 21 30.17 29 30.17 40.5
GR 36.17 59 36.17 60

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X1 350 7 0 59 200 200 200 2.75
GR 36.93 0 33.93 11 33.93 21 36.93 29 30.93 40.5
GR 36.93 59 36.93 60
STA 65+50
X1 500 7 0 59 150 150 150 2.75
GR 37.22 0 34.22 11 34.22 21 31.22 29 31.22 40.5
GR 37.22 59 37.22 60

```

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STA 65+60
***** REDDINGTON DRIVE CULVERT 2-7'x 3' BOX *****

```


X1	976		6	3	57.0	176	176	176		2.75	
X5	1	39.9									
GR	43.0		0	43.3	3	36.71	24.0	36.71	36.0	43.0	57.0
GR	43.0		60								

CHANNEL REACH "A" STATION 12+30.81

X1	1230		6	3	57	254.76	254.76	254.76		2.75	
GR	44.3		0	44.3	3	37.25	24	37.25	36.0	44.3	57.0
GR	44.3		60								

CHANNEL REACH "A" STATION 15+00

X1	1500		7	1	60	270	270	270		2.75	
GR	44.0		0	44.0	1	38.07	19.5	37.8	31	40.8	52
GR	40.8		58	45.0	60						

CHANNEL REACH "A" STATION 20.+00.00

X1	2000					500	500	500		1.01	
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CHANNEL REACH "A" STATION 21+10

X1	2110		6	26	37.5	114	114	114		2.75	
GR	46.0		0	46.0	1	40.20	19.5	40.2	31.0	46.0	52
GR	46.0		60								

CHANNEL REACH "B" STATION 9+00

X1	900		5	0	59	26	26	26		2.75	
GR	47.0		0	40.44	22	40.44	37	47.0	59	47.0	60

QT 1 233
 *****CORRAL HOLLOW ROAD CULVERT DOWNSTREAM SECTION *****
 2-CELL 7'*5' CBC

CHANNEL REACH "B" STATION 9+10.50

NC	0.05	0.05	0.013	0.3	0.5						
X1	910	6	22	37	10	10				2.75	
X3	10										
GR	46.6	0	46.6	22	40.48	22	40.48	37	46.6	37	
GR	46.6	60									

SB	0.9	2.25	3.0		15	1	70		43.62	43.23	
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CHANNEL REACH "B" STATION 10+65.50

X1	1065					155	155	155		0.39	
X2			1	48.62	49.74						
X3	10										

CHANNEL REACH "B" STATION 11+05.50

NC	0.05	0.05	0.05	0.1	0.3						
X1	1105	5	0	59	40	40	40		59	47.0	60
GR	47.0	0	41.0	22	41.0	37	47.0				

CHANNEL REACH "B" STATION 12+45.50

X1	1245	7	17	59	140	140	140		26	2.75	37.5
GR	48.0	0	45.0	9	45.0	17	41.75			41.4	
GR	48.0	59	48.0	60							

CHANNEL REACH "B" STATION 14+00.50

X1	1400	6	3	59	155	155	155		37.5	2.75	59
GR	50.0	0	50.0	3	42.25	26	41.9			48.0	
GR	48.0	60									

CHANNEL REACH "B" STATION 15+13.50

X1	1513	5	0	59	113	113	113		59	2.75	60
GR	50.0	0	42.2	22	42.2	37	50.0			50.0	

CHANNEL REACH "B" STATION 15+30
 FOX HOLLOW WAY 14'10"*4'10" ARCH CMP
 DOWNSTREAM SECTION

NC			0.024	0.3	0.5						
X1	1530	11	0	14.83	17	17	17			2.75	
X3	10										
GR	50.3	0	42.26	0	42.26	1	42.26	3	42.26	42.26	6
GR	42.26	7.42	42.26	9.8	42.26	11.8	42.26	13.8	42.26	42.26	14.83
GR	50.3	14.83									

X1	1531					1	1	1			
BT	9	0	53.05	47.04	1	53.05	48.54	3	53.05	49.14	
BT	6	53.05	49.64	7.42	53.05	49.84	9.8	53.05	49.64	11.8	

BT	53.05	49.14	13.8	53.05	48.54	14.83	53.05	47.04		
X1	1641				110	110	110		0.33	
X2							1			
CHANNEL REACH "B" STATION 1642.50										
X1	1642				1	1	1			
X3	10									
1	02DEC10	12:37:29							PAGE	6
NC	0.05	0.05	0.05	0.1	0.3					
X1	1655	5	0	59	13	13	13		2.75	
GR	50.7	0	42.60	22	42.60	37	50.7	59	50.7	60
CHANNEL REACH "B" STATION 16+82.50										
X1	1682	6	0	59	27	27	27		2.75	
GR	50.0	0	50.0	8	42.8	26	42.8	37.5	50.0	59
GR	50.0	60								
CHANNEL REACH "B" STATION 19+82.50										
X1	1982	6	0	59	245	287	300		2.75	
GR	50.80	0	50.80	3	43.64	26	43.64	37.5	52.00	59
GR	52.00	60								
CHANNEL REACH "B" STATION 22+82.50										
X1	2282	6	0	59	303	295	300		2.75	
GR	50.5	0	50.5	8	44.48	26	44.48	37.5	54.0	59
GR	54.0	60								
CHANNEL REACH "B" STATION 25+82.50										
X1	2582	6	3	59	320	285	300		2.75	
GR	52.00	0	52.00	3	45.32	26	45.32	37.5	54.00	59
GR	54.00	60								
CHANNEL REACH "B" STATION 28+83.78										
X1	2883	5	0	59	301	301	301		2.75	
GR	54.5	0	46.1	22	46.1	37	52.0	59	52.0	60
NC	0.05	0.05	0.024	0.3	0.5					
CHANNEL REACH "C" STATION 10+10.23 CYPRESS DRIVE CULVERT DOWNSTREAM SECTION 14'10"*4'10" ARCH CMP										
X1	1010	11	0	14.83	40	40	40		2.75	
X3	10									
GR	55.0	0	46.18	0	46.18	1	46.18	3	46.18	5
GR	46.18	7.42	46.18	9.84	46.18	11.84	46.18	13.84	46.18	14.83
GR	55.0	14.83								
CHANNEL REACH "C" STATION 10+20.23										
X1	1011				1	1	1			
BT	9	0	57.15	50.96	1	57.15	52.45	3	57.15	53.05
BT	5	57.15	53.55	7.42	57.15	53.76	9.84	57.15	53.55	11.84
BT	57.15	53.05	13.84	57.15	52.45	14.83	57.15	50.96		
CHANNEL REACH "C" STATION 11+33										
1	02DEC10	12:37:29							PAGE	7
X1	1133				122	122	122		0.40	
X2							1			
CHANNEL REACH "C" STATION 11+33.88										
X1	1134				1	1	1			
X3	10									
CHANNEL REACH "C" STATION 11+48.88										
NC	0.05	0.05	0.05	0.1	0.3					
X1	1149	5	0	59	15	15	15		2.75	
GR	54.0	0	54.0	3	46.63	26	46.63	37.5	57.0	90
X1	1184	8	3	46.5	35	35	35		2.75	
GR	54.0	0	53.7	3	47.05	26	46.7	37.5	49.7	46.5
GR	49.7	54.5	57.0	89	57.0	90				

CHANNEL REACH "C" STATION 14+43.88

X1	1443	8	3	46.5	259	259	259		2.75	
GR	54.90	0	54.90	3	47.78	26	47.43	37.5	50.43	46.5
GR	50.43	54.5	57.00	89	57.00	90				

CHANNEL REACH "C" STATION 17+39.24

QT 1 219

***** TENNIS LANE 1-5'x3' BOX & 1-14'8"4'1" ARCH ARCH CULVERT *****

X1	1739	4	0	90	296	296	296		2.75	
GR	54.0	0	48.26	30	48.26	50.67	54	90		
NC	0.05	0.05	0.024	0.3	0.5					
X1	1779	17	0	90	40	40	40		2.75	
GR	54.0	0	48.55	30	48.55	35	48.39	36	48.39	36.5
GR	48.39	37.5	48.39	38.5	48.39	39.5	48.39	41.5	48.39	43.33
GR	48.39	45	48.39	47	48.39	48	48.39	49	48.39	50
GR	48.39	50.67	54.0	90						

CHANNEL REACH "C" STATION 17+89.25

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X1	1780				1	1	1			
BT	-17	0	56.75	56.75	30	56.75	54.30	35	56.75	54.30
BT		36	56.75	52.75	36.5	56.75	53.75	37.5	56.75	54.25
BT		38.5	56.75	54.65	39.5	56.75	54.75	41.5	56.75	54.95
BT		43.33	56.75	55.47	45	56.75	54.95	47	56.75	54.75
BT		48	56.75	54.65	49	56.75	54.25	50	56.75	53.75
BT		50.67	56.75	52.75	90	56.75	56.75			

X1	1875				95	95	95		0.24	
X2							1			

X1	1876				1	1	1			
NC	0.05	0.05	0.05	0.1	0.3					
X1	1886	5	0	59	10	10	10		2.75	
GR	54.7	0	48.66	30	48.66	51	56.0	59	56.0	60

CHANNEL REACH "C" STATION 19+16.05

X1	1916	6	0	59	30	30	30		2.75	
GR	54.77	0	54.77	8	49.04	29	48.77	40.5	54.77	59
GR	54.77	60								

CHANNEL REACH "C" STATION 21+91.05

X1	2191				275	275	275		.99	
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CHANNEL REACH "C" STATION 24+91.05

X1	2491	7	0	59	300	300	300		2.75	
GR	56.00	0	55.17	12	55.17	20	52.17	29	51.84	41.5
GR	56.00	59	56.0	60						

CHANNEL REACH "C" STATION 26+91.05

X1	2691	6	0	59	200	200	200		2.75	
GR	59.9	0	59.9	8	52.87	29	52.6	40.5	60.0	59
GR	60.0	60								

LOCATION OF 90 DEGREE CHANNEL BEND AT LATHROP PARKWAY AND SCHULTE ROAD
CHANNEL REACH "D" STATION 11+19.94

QT	1	211								
X1	1119	6	0	59	295	165	225		2.75	
GR	60.0	0	60.0	8	53.87	29	53.6	40.5	60.0	59
GR	60.0	60								

CHANNEL REACH "D" STATION 14+19.94

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STA 15+42

X1	1542	6	7	59	123	123	123		2.75	
GR	60.7	0	60.7	7	54.77	29	54.50	40.5	60.5	59
GR	60.5	60								

STA 18+80

X1	1880	6	2	59	338	338	338		2.75	
GR	61.5	0	62.3	2	55.17	29	54.90	40.5	61.5	59
GR	61.5	60								

STA 22+00

X1	2200	8	10	59	320	320	320		2.75	
GR	62.3	0	62.3	10	58.2	12	58.2	18	55.47	20
GR	55.2	40.5	62.3	59	62.3	60				

QT 1 208
STA 23+62

***** UPTC RAILROAD CULVERT, 12'x6' BOX *****

X1	2362	6	0	60	162	162	162		2.75	
GR	62.5	0	62.5	10	61.4	24	55.4	24	55.4	36
GR	62.5	60								

NC	0.05	0.05	0.013	0.3	0.5					
STA 23+73.86										
X1	2374	6	24	36	12	12	12		2.75	
X3	10									
GR	69.4	0	69.4	24	55.4	24	55.4	36	69.4	36
GR	69.4	60								

SC	1.013	0.5	3		6	12	68	8.1	58.35	58.15
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STA 24+39.82

X1	2440				68	68	68		0.2	
X2			2	69.35	72.15					
X3	10									

NC	0.05	0.05	0.05	0.1	0.3					
STA 24+52										
X1	2452	6	0	60	12	12	12		2.75	
GR	64.1	0	64.1	10	61.0	24	55.6	24	55.6	36
GR	64.1	60								

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QT 1 144
STA 26+00

X1	2600	8	10	59	148	148	148		2.75	
GR	64.7	0	64.7	10	59.02	12	59.02	18	56.56	20
GR	56.02	40.5	64.5	59	64.5	60				

STA 30+00

X1	3000	6	7	59	400	400	400		2.75	
GR	65.6	0	65.6	7	56.87	29	56.6	40.5	65.8	59
GR	65.8	60								

STA 33+00

X1	3300	6	7	59	300	300	300		2.75	
GR	66.3	0	66.3	7	58.97	29	58.7	40.5	66.6	59
GR	66.6	60								

STA 34+00

X1	3400	6	7	59	100	100	100		2.75	
GR	66.8	0	66.8	7	59.37	29	59.1	40.5	67.6	59
GR	67.6	60								

QT 1 133
***** SCHULTE ROAD CULVERT AT SYCAMORE PARKWAY, 2-7'x5' BOX *****

STA 34+69.49

X1	3470	5	0	59	70	70	70		2.75	
GR	66.7	0	59.34	21	59.34	38	67.1	59	67.1	60

NC	0.05	0.05	0.013	0.3	0.5					
STA 10+30.07 REACH "E"										
X1	1030	6	21	38	30	30	30		2.75	
X3	10									
GR	66.7	0	66.7	21	59.46	21	59.46	38	67.2	59
GR	67.2	60								

SC	2.013	0.5	3		5	7	115	8.1	62.54	62.21
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STA 11+45

X1	1145				115	115	115		0.33	
X2			2	67.54	69.45					
X3	10									

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NC	0.05	0.05	0.05	0.1	0.3					
STA 13+00										
X1	1300	6	8	59	155	155	155		2.75	
GR	68.0	0	68.0	8	61.07	29	60.8	40.5	70.0	59
GR	70.0	60								

STA 13+49.25
 ***** BENE VAGIENNE ROAD CULVERT, 13'x7" X 4'x7" ARCH CMP *****

X1	1349	4	0	60	49	49	49		2.75	
GR	68.0	0	61.1	23	61.1	37	68.0	60		

NC	0.05	0.05	0.024	0.3	0.5					
STA 13+88										
X1	1388	13	23	60	39	39	39		2.75	
X3	10									
GR	69.7	0	69.7	23	61.39	23	61.39	24	61.39	26
GR	61.39	28	61.39	29.80	61.39	31.6	61.39	33.6	61.39	35.6
GR	61.39	36.6	69.7	36.6	69.7	60				

X1	1389				1	1	1			
BT	11	0	72.45	72.45	23	72.45	66.14	24	72.45	67.14
BT	26	72.45	68.14	28	72.45	68.55	29.8	72.45	68.72	31.6
BT	72.45	68.55	33.6	72.45	68.14	35.6	72.45	67.14	36.6	72.45
BT	66.14	60	72.45	72.45						

STA 14+91.75

X1	1492				103	103	103		0.68	
X2							1			

X1	1493				1	1	1			
X3	10									

NC	0.05	0.05	0.05	0.1	0.3					
STA 15+01.75										
X1	1502	4	0	60	10	10	10		2.75	
GR	68.0	0	62.1	23	62.1	37	68.0	60		

STA 17+00

X1	1700	7	12	59	202	202	202		2.75	
GR	70.0	0	67.0	12	67.0	20	63.67	29	63.4	40.5
GR	70.0	59	70.0	60						

STA 18+20
 1 02DEC10 12:37:29

X1	1820	9	9	59	120	120	120		2.75	
GR	70.0	0	67.3	12	67.3	20	64.57	29	64.3	45.5
GR	69.3	46.5	69.3	51.5	70.0	52.5	70.0	60		

STA 20+00

X1	2000	8	8	53.5	180	180	180		2.75	
GR	71.5	0	71.5	8	65.77	29	65.5	45.5	70.5	46.5
GR	70.5	52.5	74.0	53.5	74.0	60				

STA 21+18.90
 ***** AMBERWOOD WAY CULVERT, 12'x 4' BOX *****

X1	2119	4	0	60	119	119	119		2.75	
GR	72.0	0	66.2	24	66.2	36	74.0	60		

NC	0.05	0.05	0.013	0.3	0.5					
STA 21+27										
X1	2127	6	24	36	8	8	8		2.75	
X3	10									
GR	72.0	0	72.0	24	66.2	24	66.2	36	74.0	36
GR	74.0	60								

SC	1.013	0.5	3		4.0	12.0	91	8.1	69.51	68.95
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STA 22+18

X1	2218				91	91	91		0.56	
X2			2	73.51	74.65					
X3	10									

NC	0.05	0.05	0.05	0.1	0.3					
STA 22+27.78										
X1	2228	4	0	60	10	10	10		2.75	
GR	72.0	0	66.76	24	66.76	36	74.2	60		

STA 24+00										
X1	2400	6	8	59	172	172	172		2.75	
GR	73.9	0	73.9	8	67.67	29	67.4	40.5	74.6	59
GR	74.6	60								
STA 26+00										
X1	2600	6	8	59	200	200	200		2.75	
GR	74.0	0	74.0	8	68.47	29	68.2	40.5	75.0	59
GR	75.0	60								
STA 30+00										
1	02DEC10	12:37:29								PAGE 13
X1	3000	7	7	59	400	400	400		2.75	
GR	76.0	0	73.7	12	73.7	20	70.97	29	70.70	40.5
GR	77.0	59	77.0	60						
QT	1	79								
STA 30+44										
***** MONUMENT DRIVE CULVERT, 8'x 4' BOX *****										
X1	3044	4	0	60	44	44	44		2.75	
GR	77.0	0	70.99	26	70.99	34	78.0	60		
NC	0.05	0.05	0.013	0.3	0.5					
STA 30+54										
X1	3054	6	26	34	10	10	10		2.75	
X3	10									
GR	77.0	0	77.0	26	71.00	26	71.00	34	78.0	34
GR	78.0	60								
SC	1.013	0.5	3		4.0	8.0	94	8.1	74.15	73.75
STA 31+48										
X1	3148				94	94	94		0.40	
X2			2	78.15	79.75					
X3	10									
NC	0.05	0.05	0.05	0.1	0.3					
STA 31+57.83										
X1	3158	4	0	60	10	10	10		2.75	
GR	77.0	0	71.5	26	71.5	34	78.0	60		
STA 35+00										
X1	3500	7	20	59	342	342	342		2.75	
GR	79.70	0	76.7	12	76.7	20	73.77	29	73.7	40.5
GR	80.0	59	80.0	60						
STA 39+00										
X1	3900	6	8	59	400	400	400		2.75	
GR	82.5	0	82.5	8	76.57	29	76.3	40.5	84.0	59
GR	84.0	60								
STA 39+52.40										
***** ALLEGHENY WAY CULVERT, 7'x4' BOX *****										
X1	3952	5	0	59	52	52	52		2.75	
GR	83.0	0	76.55	26	76.55	33	84.0	59	84.0	60
1	02DEC10	12:37:29								PAGE 14
NC	0.05	0.05	0.013	0.3	0.5					
STA 39+62										
X1	3962	6	26	33	10	10	10		2.75	
X3	10									
GR	83.0	0	84.0	26	76.7	26	76.7	33	84.0	33
GR	84.0	60								
SC	1.013	0.5	3		4.0	7.0	102	8.1	79.75	79.45
STA 40+64										
X1	4064				102	102	102		0.3	
X2			2	83.75	85.75					
X3	10									
NC	0.05	0.05	0.05	0.1	0.3					
STA 40+73.58										
X1	4074	5	0	59	10	10	10		2.75	
GR	84.0	0	77.00	26	77.00	33	84.0	59	84.0	60

STA 42+00
 X1 4200 6 8 59 126 126 126 2.75
 GR 84.0 0 84.0 8 77.99 29 77.72 40.5 85.00 59
 GR 85.00 60

STA 46+00
 X1 4600 6 8 59 400 400 400 2.75
 GR 86.20 0 86.20 8 80.27 29 80.00 40.5 88.00 59
 GR 88.00 60

STA 50+00
 X1 5000 7 20 59 400 400 400 2.75
 GR 88.60 0 85.60 12 85.60 20 83.15 29 82.88 40.5
 GR 90.00 59 90.00 60

51+98.10
 ***** CENTRAL AVE CULVERT, 7'x4' BOX *****

QT 1 66
 X1 5198 5 0 59 198 198 198 2.75
 GR 90.7 0 84.62 26 84.62 33 91.0 59 91.0 60

NC 0.05 0.05 0.013 0.3 0.5
 STA 92+08
 X1 9208 6 26 33 10 10 2.75
 X3 10
 GR 90.7 0 90.7 26 84.7 26 84.7 33 91.0 33
 GR 91.0 60

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SC 1.013 0.5 3 4.0 7.0 102 8.1 87.65 87.45

STA 53+18
 X1 5318 110 110 110 0.2
 X2 2 91.65 93.75
 X3 10

NC 0.05 0.05 0.05 0.1 0.3
 STA 53+27.81
 X1 5328 5 0 59 10 10 2.75
 GR 92.0 0 84.93 26 84.93 33 94.0 59 94.0 60

QT 1 55
 STA 55+00
 X1 5500 6 8 59 172 172 172 2.75
 GR 93.4 0 93.3 8 86.77 29 86.5 40.5 94.6 59
 GR 94.6 60

STA 58+00
 X1 5800 7 20 59 300 300 300 2.75
 GR 95.3 0 92.2 12 92.2 20 89.47 29 89.2 40.5
 GR 96.2 59 96.2 60

STA 59+94.54
 ***** CHAPLIN WAY CULVERT, 60"x30" BOX *****

X1 5995 5 0 60 195 195 195 2.75
 GR 96.70 0 90.88 27 90.88 32 97.00 59 97.80 60

NC 0.05 0.05 0.013 0.3 0.5
 STA 60+00
 X1 6000 6 27 32 10 10 2.75
 X3 10
 GR 97.70 0 97.70 27 90.91 27 90.91 32 98.00 32
 GR 98.00 60

SC 1.013 0.5 3 2.5 5 104 8.1 94.00 93.66

STA 61+04
 X1 6104 104 104 104 0.34
 X2 2 96.50 98.75
 X3 10

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NC 0.05 0.05 0.05 0.1 0.3
 STA 61+13.64
 X1 6114 5 0 60 10 10 2.75
 GR 97.70 0 91.28 27 91.28 32 98.00 59 98.00 60

STA 63+40										
X1	6340	6	8	59	226	226	226		2.75	
GR	99.4	0	99.7	8	93.67	29	93.4	40.5	100.0	59
GR	100.0	60								
QT	1	52								
STA 64+89.04										
***** VALPICO ROAD CULVERT, 60"x30" BOX *****										
X1	6489	5	0	60	149	149	149		2.75	
GR	100.00	0	94.74	27	94.74	32	100.00	59	100.00	60
GR										
NC	0.05	0.05	0.013	0.3	0.5					
STA 64+99										
X1	6499	6	27	32	10	10	10		2.75	
X3	10									
GR	100.00	0	100.00	27	94.80	27	94.80	32	100.00	32
GR	100.00	60								
SC	1.013	0.5	3		2.5	5	132	8.1	98.23	97.55
STA 66+31										
X1	6631				132	132	132		0.68	
X2			2	100.73	103.49					
X3	10									
NC	0.05	0.05	0.05	0.1	0.3					
STA 66+40.72										
X1	6641	5	0	60	10	10	10		2.75	
GR	102.00	0	95.54	27	95.54	32	104.00	59	104.00	60
GR										
STA 67+40										
X1	6740	7	20	59	99	99	99		2.75	
GR	102.0	0	99.4	12	99.4	20	96.67	29	96.4	40.5
GR	104.0	59	104.0	60						
STA 70+00										
X1	7000	6	8	59	260	260	260		2.75	
GR	104.6	0	104.6	8	99.07	29	98.8	40.5	105.0	59
GR	105.0	60								
STA 71+75										
1	02DEC10	12:37:29								PAGE 17
X1	7175	6	8	59	175	175	175		2.75	
GR	106.00	0	106.00	8	100.80	29	100.53	40.5	106.00	59
GR	106.00	60								
STA 75+00										
X1	7500	6	8	59	325	325	325		2.75	
GR	108.60	0	108.60	8	101.97	29	101.70	40.5	110.00	59
GR	110.00	60								
QT	1	35								
STA 76+99.59										
***** CAMBRIDGE PLACE CULVERT, 72"x 44" CMPA *****										
X1	7700	4	0	60	200	200	200		2.75	
GR	110.9	0	103.6	27	103.6	33	110.0	60		
GR										
NC	0.05	0.05	0.024	0.3	0.5					
STA 77+30.59										
X1	7731	11	27	33	31	31	31		2.75	
X3	10									
GR	112.0	0	112.0	27	104.0	27	104.0	28	104.0	29
GR	104.0	30	104.0	31	104.0	32	104.0	33	112.0	33
GR	112.0	60								
X1	7732				1	1	1			
BT	9	0	114.75	114.75	27	114.75	108.25	28	114.75	109.35
BT	29	114.75	110.20	30	114.75	110.42	31	114.75	110.20	32
BT	114.75	109.35	33	114.75	108.25	60	114.75	114.75		
STA 78+35.33										
X1	7835				103	103	103		0.9	
X2							1			
X1	7836				1	1	1			
X3	10									
NC	0.05	0.05	0.05	0.1	0.3					

STA 78+55.23
 X1 7855 4 0 60 19 19 19 2.75
 GR 112.0 0 105.2 27 105.2 33 112.0 60

STA 80+25
 ***** CONCRETE CUT- OFF WALL *****

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X1 8025 7 20 59 175 175 175 2.75
 GR 113.00 0 110.00 12 110.00 20 107.58 29 107.31 40.5
 GR 114.00 59 114.00 60

STA 80+26

X1 8026 7 20 59 1 1 1 2.75
 GR 113.00 0 110.00 12 110.00 20 109.31 29 109.04 40.5
 GR 114.00 59 114.00 60

STA 82+40

X1 8240 7 20 59 214 214 214 2.75
 GR 116.0 0 113.0 12 113.0 20 110.87 29 110.6 40.5
 GR 116.0 59 116.0 60

STA 86+00

X1 8600 6 8 59 360 360 360 2.75
 GR 119.0 0 119.0 8 113.17 29 112.9 40.5 118.0 59
 GR 118.0 60

STA 87+80

X1 8780 6 8 59 180 180 180 2.75
 GR 120.0 0 120.0 8 114.17 29 113.9 40.5 120.0 59
 GR 120.0 60

***** CENTRAL AVE CHANNEL *****

X1 -5328 5 0 59 10 10 10 2.75
 GR 92.0 0 84.93 26 84.93 33 94.0 59 94.0 60

QT 1 11
 STA 10+40

X1 1040 6 1 40 40 40 40 2.75
 GR 94.0 0 94.0 1 85.0 19.5 85.27 31 91.0 52
 GR 91.0 60

STA 13+00

X1 1300 6 1 40 260 260 260 2.75
 GR 93.00 0 93.00 1 85.36 19.5 85.63 31 91.20 52
 GR 91.20 60

1 STA 16+00 02DEC10 12:37:29 PAGE 19

X1 1600 6 1 40 300 300 300 2.75
 GR 95.00 0 95.00 1 85.78 19.5 86.13 31 92.00 52
 GR 92.00 60

STA 19+00

X1 1900 7 1 40 300 300 300 2.75
 GR 95.00 0 95.00 1 86.20 19.5 86.47 31 90.00 40
 GR 90.00 48 92.00 60

STA 22+00

X1 2200 7 20 59 300 300 300 2.75
 GR 96.00 0 96.00 1 86.62 19.5 86.97 31 92.00 40
 GR 92.00 48 93.00 60

STA 22+83.45

X1 2283 5 0 59 83 83 83 2.75
 GR 96.0 0 86.7 27 86.7 33 93.0 59 93.0 60

***** TRACY BLVD CULVERT 1-6'X4' CBC *****

NC 0.05 0.05 0.013 0.3 0.5
 STA 22+93.45
 X1 2293 6 27 33 10 10 10 2.75
 X3 10
 GR 96.0 0 96.0 27 86.7 27 86.7 33 96.0 59
 GR 96.0 60

SC	1.013	0.5	3	4	6	235	8.1	87.34	86.71
STA 25+18.45									
X1	2518				235	235		0.37	
X2			2	91.34	94.2				
X3	10								
NC	0.05	0.05	0.05	0.1	0.3				
STA 2528.45									
X1	2528	5	0	59	10	10		2.75	
GR	94.1	0	87.34	27	87.34	33	93.3	59	93.3 60

***** CHANNEL EAST OF TRACY BLVD. *****
 ***** X-SECTIONAL DATA BASED ON CURRENT FIELD SURVEY, 2.75' IS NOT ADDED ***

1 STA 30+28
 02DEC10 12:37:29 PAGE 20

X1	3028	4	0	45	500	500	500		
GR	98.97	0	90.15	20	90.55	25	95.47	45	

***** CULVERT 1-15'X 5.0' ACMP *****

STA 30+98									
X1	3098	4	0	45	70	70	70		
GR	98.17	0	90.17	15	90.17	30	98.17	45	

NC	0.05	0.05	0.023	0.3	0.5				
STA 31+00									
X1	3100	11	0	45	2	2	2		
GR	98.17	0	90.17	15	90.17	16	90.17	18	90.17 21
GR	90.17	22.5	90.17	24	90.17	27	90.17	29	90.17 30
GR	98.17	45							

STA 31+01									
X1	3101				1	1	1		
BT	-11	0	98.17	98.17	15	98.17	90.17	16	98.17 91.40
BT		18	98.17	93.17	21	98.17	94.47	22.5	98.17 95.17
BT		24	98.17	94.47	27	98.17	93.17	29	98.17 91.40
BT		30	98.17	90.17	45	98.17	98.17		

STA 31+58									
X1	3157				57	57	57		0.47
X2							1		

X1	3158				1	1	1		
X3	10								

NC	0.05	0.05	0.05	0.1	0.3				
STA 34+38									
X1	3438	4	0	45	280	280	280		
GR	98.38	0	90.50	20	91.20	25	94.19	45	

***** CULVERT 1-15'X 5.5' ACMP *****

STA 37.21									
X1	3721	4	0	45	283	283	283		
GR	97.41	0	89.91	15	89.91	30	97.41	45	

1 02DEC10 12:37:29 PAGE 21

NC	0.05	0.05	0.023	0.3	0.5				
STA 37+23									
X1	3723	11	0	45	2	2	2		
GR	97.41	0	89.91	15	89.91	16	89.91	18	89.91 21
GR	89.91	22.5	89.91	24	89.91	27	89.91	29	89.91 30
GR	97.41	45							

STA 37+24									
X1	3724				1	1	1		
BT	-11	0	97.41	97.41	15	97.41	89.91	16	97.41 91.64
BT		18	97.41	93.41	21	97.41	94.71	22.5	97.41 95.41
BT		24	97.41	94.71	27	97.41	93.41	29	97.41 91.64
BT		30	97.41	89.91	45	97.41	97.41		

STA 37+82

X1	3782			58	58	58		0.09
X2						1		
X1	3783			1	1	1		
X3	10							
NC	0.05	0.05	0.05	0.1	0.3			
STA	39+13							
X1	3913	4	0	50	130	130	130	
GR	99.19	0	90.75	20	90.85	25	96.29	50
STA	41+43							
X1	4143	4	0	45	230	230	230	
GR	99.32	0	90.18	20	90.13	25	96.06	45
STA	42+33							
X1	4233	4	0	45	90	90	90	
GR	97.73	0	90.30	20	90.05	25	97.65	26
STA	44+13							
X1	4413	4	0	45	180	180	180	
GR	97.45	0	90.40	25	90.33	32	97.59	33

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THIS RUN EXECUTED 02DEC10 12:37:29

 HEC-2 WATER SURFACE PROFILES
 Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

WESTSIDE CHANNEL SYSTEM
 SUMMARY PRINTOUT TABLE 150

SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10*KS	VCH	AREA	.01K
50.000	.00	.00	.00	32.12	202.00	37.50	.00	37.52	3.56	1.18	171.72	107.05
* 150.000	150.00	.00	.00	32.92	202.00	37.56	.00	37.60	7.34	1.52	132.94	74.54
* 350.000	200.00	.00	.00	33.68	202.00	37.81	.00	38.00	83.54	3.49	57.92	22.10
* 500.000	150.00	.00	.00	33.97	202.00	38.30	.00	38.35	10.33	1.71	117.98	62.86
510.000	10.00	.00	.00	33.99	202.00	38.32	.00	38.36	6.56	1.51	133.82	78.86
* 521.000	11.00	.00	.00	34.01	202.00	38.27	.00	38.42	2.02	3.16	63.85	142.05
624.000	103.00	39.70	37.33	34.33	202.00	38.84	.00	38.98	1.72	2.99	67.61	154.11
* 650.000	26.00	.00	.00	34.38	202.00	38.96	.00	38.99	7.85	1.56	129.88	72.10
* 800.000	150.00	.00	.00	35.55	202.00	39.12	.00	39.21	27.91	2.42	83.60	38.24
950.000	150.00	.00	.00	36.45	202.00	39.64	.00	39.78	51.07	2.97	68.02	28.27
980.000	30.00	.00	.00	37.25	202.00	39.76	.00	40.02	90.76	4.07	49.61	21.20
* 982.000	3.00	.00	.00	36.82	202.00	39.87	.00	40.04	3.19	3.33	60.58	113.04
* 1262.000	280.00	42.99	41.81	37.81	202.00	41.72	.00	41.81	1.27	2.41	83.96	179.51
* 1338.000	76.00	.00	.00	37.75	240.00	42.40	.00	42.40	.01	.08	3557.33	2174.07
1468.000	130.00	.00	.00	38.19	240.00	42.40	.00	42.40	.02	.09	3293.20	1902.56
* 1968.000	500.00	.00	.00	39.25	240.00	42.40	.00	42.40	.13	.20	1181.28	675.10
* 976.000	176.00	.00	.00	39.46	240.00	42.23	.00	42.49	82.75	4.12	58.19	26.38

1 02DEC10 12:37:29 PAGE 23

SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10*KS	VCH	AREA	.01K
-------	------	-------	------	-------	---	-------	------	----	-------	-----	------	------

*	1230.000	254.76	.00	.00	40.00	240.00	43.59	.00	43.73	31.34	2.95	81.44	42.87
	1500.000	270.00	.00	.00	40.55	240.00	44.29	.00	44.36	18.14	2.14	112.07	56.36
	2000.000	500.00	.00	.00	41.56	240.00	45.24	.00	45.32	19.96	2.21	108.64	53.72
*	2110.000	114.00	.00	.00	42.95	240.00	45.45	.00	45.86	108.67	5.72	50.26	23.02
	900.000	26.00	.00	.00	43.19	240.00	45.88	.00	46.10	64.97	3.70	64.94	29.78
*	910.000	10.00	.00	.00	43.23	233.00	45.71	.00	46.32	12.97	6.24	37.31	64.69
	1065.000	155.00	49.74	48.62	43.62	233.00	45.91	.00	46.63	16.57	6.77	34.40	57.23
*	1105.000	40.00	.00	.00	43.75	233.00	46.63	.00	46.78	45.49	3.17	73.41	34.55
	1245.000	140.00	.00	.00	44.15	233.00	47.33	.00	47.55	62.54	3.76	61.98	29.46
	1400.000	155.00	.00	.00	44.65	233.00	48.15	.00	48.30	38.77	3.12	74.79	37.42
	1513.000	113.00	.00	.00	44.95	233.00	48.53	.00	48.64	22.74	2.59	90.00	48.87
	1530.000	17.00	.00	.00	45.01	233.00	48.46	.00	48.78	17.28	4.55	51.16	56.06
	1531.000	1.00	53.05	49.84	45.01	233.00	48.45	.00	48.79	19.57	4.68	49.81	52.67
	1641.000	110.00	53.05	50.17	45.34	233.00	48.67	.00	49.03	21.56	4.84	48.11	50.18
	1642.000	1.00	.00	.00	45.34	233.00	48.70	.00	49.04	18.65	4.68	49.83	53.95
	1655.000	13.00	.00	.00	45.35	233.00	48.99	.00	49.09	21.98	2.57	90.53	49.70
	1682.000	27.00	.00	.00	45.55	233.00	49.02	.00	49.18	37.57	3.18	73.34	38.01
	1982.000	300.00	.00	.00	46.39	233.00	50.05	.00	50.18	29.54	2.88	80.84	42.87
	2282.000	300.00	.00	.00	47.23	233.00	50.94	.00	51.08	30.19	2.96	78.82	42.40
	2582.000	300.00	.00	.00	48.07	233.00	51.81	.00	51.93	26.56	2.76	84.42	45.21
	2883.000	301.00	.00	.00	48.85	233.00	52.52	.00	52.61	19.20	2.38	97.72	53.18
	1010.000	40.00	.00	.00	48.93	233.00	52.48	.00	52.78	15.89	4.43	52.65	58.46
	1011.000	1.00	57.15	53.76	48.93	233.00	52.47	.00	52.80	18.34	4.56	51.13	54.40
	1133.000	122.00	57.15	54.16	49.33	233.00	52.69	.00	53.05	20.91	4.79	48.62	50.95
	1134.000	1.00	.00	.00	49.33	233.00	52.73	.00	53.06	18.04	4.62	50.40	54.86
	1149.000	15.00	.00	.00	49.38	233.00	53.02	.00	53.11	21.34	2.64	96.08	50.44

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	SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10*KS	VCH	AREA	.01K
	1184.000	35.00	.00	.00	49.45	233.00	53.08	.00	53.21	30.35	2.91	83.79	42.30
	1443.000	259.00	.00	.00	50.18	233.00	53.86	.00	53.99	29.39	2.90	84.39	42.98
*	1739.000	296.00	.00	.00	51.01	219.00	54.42	.00	54.45	9.36	1.56	140.27	71.58
*	1779.000	40.00	.00	.00	51.14	219.00	54.43	.00	54.48	2.58	1.66	132.18	136.27
*	1780.000	1.00	56.75	56.75	51.14	219.00	54.43	.00	54.48	7.19	1.92	114.27	81.65
	1875.000	95.00	56.75	56.99	51.38	219.00	54.49	.00	54.56	7.33	2.04	107.46	80.91
*	1876.000	1.00	.00	.00	51.38	219.00	54.51	.00	54.56	3.23	1.80	121.88	121.93
*	1886.000	10.00	.00	.00	51.41	219.00	54.49	.00	54.58	20.81	2.34	93.49	48.00
*	1916.000	30.00	.00	.00	51.52	219.00	54.51	.00	54.71	62.53	3.62	60.46	27.70
	2191.000	275.00	.00	.00	52.51	219.00	55.89	.00	56.03	37.70	3.01	72.71	35.67
	2491.000	300.00	.00	.00	54.59	219.00	57.37	.00	57.59	73.40	3.77	58.14	25.56
	2691.000	200.00	.00	.00	55.35	219.00	58.62	.00	58.81	50.84	3.49	62.80	30.71
	1119.000	225.00	.00	.00	56.35	211.00	59.67	.00	59.82	39.82	3.08	68.50	33.44
	1542.000	123.00	.00	.00	57.25	211.00	60.24	.00	60.43	59.19	3.51	60.10	27.43
*	1880.000	338.00	.00	.00	57.65	211.00	61.47	.00	61.57	21.47	2.44	86.55	45.54
*	2200.000	320.00	.00	.00	57.95	211.00	61.99	.00	62.04	10.78	1.87	112.61	64.27
*	2362.000	162.00	.00	.00	58.15	208.00	62.20	.00	62.31	24.59	2.73	76.29	41.94
*	2374.000	12.00	.00	.00	58.15	208.00	62.12	.00	62.42	4.57	4.37	47.63	97.29
	2440.000	68.00	72.15	69.35	58.35	208.00	62.08	.00	62.42	5.44	4.65	44.76	89.16

*	2452.000	12.00	.00	.00	58.35	208.00	62.31	.00	62.45	29.67	2.99	69.67	38.18
*	2600.000	148.00	.00	.00	58.77	144.00	62.64	.00	62.67	6.97	1.45	99.65	54.53
*	3000.000	400.00	.00	.00	59.35	144.00	63.02	.00	63.08	15.76	2.10	68.63	36.28
*	3300.000	300.00	.00	.00	61.45	144.00	63.83	.00	64.04	81.26	3.68	39.08	15.97
	3400.000	100.00	.00	.00	61.85	144.00	64.53	.00	64.68	51.70	3.15	45.64	20.03
*	3470.000	70.00	.00	.00	62.09	133.00	64.83	.00	64.89	17.10	1.97	67.45	32.17
*	1030.000	30.00	.00	.00	62.21	133.00	64.81	.00	64.94	2.18	2.90	53.41	90.10

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	SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS	EG	10*KS	VCH	AREA	.01K
	1145.000	115.00	69.45	67.54	62.54	133.00	65.10	.00	65.23	2.28	2.95	52.51	87.99
*	1300.000	155.00	.00	.00	63.55	133.00	65.12	65.12	65.71	367.38	6.16	21.59	6.94
*	1349.000	49.00	.00	.00	63.85	133.00	66.11	.00	66.23	43.28	2.72	48.88	20.22
	1388.000	39.00	.00	.00	64.14	133.00	66.14	.00	66.51	34.51	4.87	27.30	22.64
	1389.000	1.00	72.45	72.45	64.14	133.00	66.14	.00	66.51	34.81	4.89	27.23	22.54
	1492.000	103.00	72.45	73.13	64.82	133.00	66.52	.00	67.03	57.12	5.75	23.14	17.60
	1493.000	1.00	.00	.00	64.82	133.00	66.53	.00	67.04	56.65	5.73	23.20	17.67
	1502.000	10.00	.00	.00	64.85	133.00	67.02	.00	67.13	47.49	2.74	48.57	19.30
*	1700.000	202.00	.00	.00	66.15	133.00	68.29	.00	68.53	102.36	3.89	34.21	13.15
	1820.000	120.00	.00	.00	67.05	133.00	69.30	.00	69.45	59.70	3.18	41.78	17.21
	2000.000	180.00	.00	.00	68.25	133.00	70.43	.00	70.59	66.77	3.28	40.60	16.28
	2119.000	119.00	.00	.00	68.95	133.00	71.18	.00	71.32	55.69	2.98	44.65	17.82
*	2127.000	8.00	.00	.00	68.95	133.00	71.05	.00	71.48	11.74	5.26	25.26	38.82
	2218.000	91.00	74.65	73.51	69.51	133.00	71.85	.00	72.19	8.48	4.72	28.18	45.66
*	2228.000	10.00	.00	.00	69.51	133.00	72.16	.00	72.24	26.53	2.24	59.26	25.82
	2400.000	172.00	.00	.00	70.15	133.00	72.72	.00	72.85	47.95	2.93	45.35	19.21
	2600.000	200.00	.00	.00	70.95	133.00	73.61	.00	73.72	39.40	2.69	49.46	21.19
	3000.000	400.00	.00	.00	73.45	133.00	75.71	.00	75.90	77.80	3.46	38.45	15.08
	3044.000	44.00	.00	.00	73.74	79.00	76.06	.00	76.12	26.04	1.97	40.17	15.48
*	3054.000	10.00	.00	.00	73.75	79.00	75.95	.00	76.26	9.68	4.49	17.59	25.40
	3148.000	94.00	79.75	78.15	74.15	79.00	76.34	.00	76.66	9.64	4.48	17.62	25.45
*	3158.000	10.00	.00	.00	74.25	79.00	76.65	.00	76.70	20.84	1.78	44.36	17.30
*	3500.000	342.00	.00	.00	76.45	79.00	77.95	.00	78.13	113.35	3.41	23.18	7.42
*	3900.000	400.00	.00	.00	79.05	79.00	80.99	.00	81.10	52.07	2.60	30.37	10.95
	3952.000	52.00	.00	.00	79.30	79.00	81.28	.00	81.40	62.67	2.77	28.56	9.98
*	3962.000	10.00	.00	.00	79.45	79.00	81.03	81.03	81.82	35.15	7.16	11.03	13.33

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	SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS	EG	10*KS	VCH	AREA	.01K
*	4064.000	102.00	85.75	83.75	79.75	79.00	82.14	.00	82.48	10.60	4.71	16.78	24.27
	4074.000	10.00	.00	.00	79.75	79.00	82.48	.00	82.53	16.16	1.68	46.91	19.65
	4200.000	126.00	.00	.00	80.47	79.00	82.73	.00	82.80	28.30	2.09	37.82	14.85
*	4600.000	400.00	.00	.00	82.75	79.00	84.48	.00	84.63	83.68	3.08	25.64	8.64
	5000.000	400.00	.00	.00	85.63	79.00	87.46	.00	87.58	65.33	2.79	28.26	9.77
*	5198.000	198.00	.00	.00	87.37	66.00	88.97	.00	89.11	95.75	3.01	21.89	6.74
*	9208.000	10.00	.00	.00	87.45	66.00	88.85	88.85	89.55	34.82	6.74	9.79	11.19
*	5318.000	110.00	93.75	91.65	87.65	66.00	89.75	.00	90.06	10.62	4.47	14.76	20.25

*	5328.000	10.00	.00	.00	87.68	66.00	90.05	.00	90.10	23.43	1.89	34.94	13.64
*	5500.000	172.00	.00	.00	89.25	55.00	90.67	.00	90.79	92.15	2.88	19.12	5.73
	5800.000	300.00	.00	.00	91.95	55.00	93.37	.00	93.50	88.08	2.80	19.62	5.86
	5995.000	195.00	.00	.00	93.63	55.00	95.19	.00	95.32	99.54	2.91	18.93	5.51
*	6000.000	10.00	.00	.00	93.66	55.00	95.21	95.21	95.99	41.06	7.11	7.74	8.58
*	6104.000	104.00	98.75	96.50	94.00	55.00	96.34	.00	96.68	13.03	4.69	11.74	15.24
	6114.000	10.00	.00	.00	94.03	55.00	96.70	.00	96.72	10.56	1.29	42.61	16.93
*	6340.000	226.00	.00	.00	96.15	55.00	97.20	.00	97.48	301.93	4.24	12.96	3.17
*	6489.000	149.00	.00	.00	97.49	52.00	99.16	.00	99.24	59.74	2.31	22.55	6.73
*	6499.000	10.00	.00	.00	97.55	52.00	99.04	99.04	99.80	40.69	6.97	7.46	8.15
*	6631.000	132.00	103.49	100.73	98.23	52.00	100.54	.00	100.85	12.09	4.49	11.58	14.96
	6641.000	10.00	.00	.00	98.29	52.00	100.86	.00	100.89	12.71	1.40	37.27	14.59
*	6740.000	99.00	.00	.00	99.15	52.00	101.02	.00	101.07	26.67	1.83	28.48	10.07
*	7000.000	260.00	.00	.00	101.55	52.00	102.54	.00	102.82	324.98	4.24	12.27	2.88
*	7175.000	175.00	.00	.00	103.28	52.00	104.77	.00	104.86	58.51	2.31	22.47	6.80
	7500.000	325.00	.00	.00	104.45	52.00	106.22	.00	106.28	34.10	2.01	25.90	8.90
*	7700.000	200.00	.00	.00	106.35	35.00	107.34	.00	107.54	236.06	3.58	9.77	2.28
*	7731.000	31.00	.00	.00	106.75	35.00	107.76	107.76	108.28	124.59	5.75	6.09	3.14

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SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10*KS	VCH	AREA	.01K	
7732.000	1.00	114.75	110.42	106.75	35.00	107.99	.00	108.34	68.86	4.71	7.44	4.22	
7835.000	103.00	114.75	111.32	107.65	35.00	108.77	.00	109.19	92.22	5.20	6.74	3.64	
7836.000	1.00	.00	.00	107.65	35.00	108.79	.00	109.20	90.35	5.16	6.78	3.68	
7855.000	19.00	.00	.00	107.95	35.00	109.30	.00	109.38	67.46	2.28	15.38	4.26	
*	8025.000	175.00	.00	.00	110.06	35.00	111.03	.00	111.17	167.73	3.00	11.68	2.70
*	8026.000	1.00	.00	.00	111.79	35.00	112.51	112.51	112.74	489.15	3.85	9.08	1.58
*	8240.000	214.00	.00	.00	113.35	35.00	114.71	.00	114.76	38.23	1.76	19.85	5.66
*	8600.000	360.00	.00	.00	115.65	35.00	116.74	.00	116.83	94.84	2.43	14.39	3.59
*	8780.000	180.00	.00	.00	116.65	35.00	117.96	.00	118.01	47.85	1.95	17.99	5.06
-5328.000	10.00	.00	.00	87.68	35.00	90.05	.00	90.06	6.63	1.00	34.86	13.59	
1040.000	40.00	.00	.00	87.75	11.00	90.07	.00	90.07	.48	.31	38.40	15.90	
1300.000	260.00	.00	.00	88.11	11.00	90.09	.00	90.09	.87	.37	31.40	11.76	
*	1600.000	300.00	.00	.00	88.53	11.00	90.13	.00	90.13	2.45	.54	21.50	7.03
*	1900.000	300.00	.00	.00	88.95	11.00	90.23	.00	90.24	5.67	.68	16.21	4.62
*	2200.000	300.00	.00	.00	89.37	11.00	90.47	.00	90.48	12.01	.92	12.26	3.17
2283.000	83.00	.00	.00	89.45	11.00	90.58	.00	90.59	15.00	.98	11.21	2.84	
*	2293.000	10.00	.00	.00	89.45	11.00	90.57	.00	90.61	1.97	1.55	8.56	7.84
*	2518.000	235.00	94.20	91.34	89.82	11.00	90.50	.00	90.61	10.08	2.62	4.74	3.46
*	2528.000	10.00	.00	.00	90.09	11.00	90.51	90.51	90.69	534.74	3.35	3.28	.48
*	3028.000	500.00	.00	.00	90.15	11.00	91.84	.00	91.85	7.14	.78	14.17	4.12
*	3098.000	70.00	.00	.00	90.17	11.00	91.86	.00	91.87	.95	.36	30.64	11.30
*	3100.000	2.00	.00	.00	90.17	11.00	91.86	.00	91.87	.20	.36	30.80	24.75
*	3101.000	1.00	98.17	98.17	90.17	11.00	91.86	.00	91.87	.44	.48	23.01	16.56
*	3157.000	57.00	98.17	98.64	90.64	11.00	91.87	.00	91.87	1.06	.64	17.15	10.67
3158.000	1.00	.00	.00	90.64	11.00	91.87	.00	91.87	.61	.52	21.19	14.14	
*	3438.000	280.00	.00	.00	90.50	11.00	91.91	.00	91.93	25.67	1.17	9.37	2.17

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	SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10*KS	VCH	AREA	.01K
*	3721.000	283.00	.00	.00	89.91	11.00	91.97	.00	91.97	.46	.28	39.34	16.17
*	3723.000	2.00	.00	.00	89.91	11.00	91.97	.00	91.97	.10	.28	39.45	35.28
*	3724.000	1.00	97.41	97.41	89.91	11.00	91.97	.00	91.97	.23	.39	28.39	23.17
	3782.000	58.00	97.41	97.50	90.00	11.00	91.97	.00	91.97	.25	.40	27.28	21.88
*	3783.000	1.00	.00	.00	90.00	11.00	91.97	.00	91.97	.11	.29	37.33	32.55
*	3913.000	130.00	.00	.00	90.75	11.00	91.97	.00	91.98	16.96	1.04	10.56	2.67
*	4143.000	230.00	.00	.00	90.13	11.00	92.10	.00	92.11	2.53	.54	20.32	6.92
*	4233.000	90.00	.00	.00	90.05	11.00	92.13	.00	92.14	5.19	.76	14.56	4.83
	4413.000	180.00	.00	.00	90.33	11.00	92.20	.00	92.21	2.99	.58	18.82	6.36

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WESTSIDE CHANNEL SYSTEM

SUMMARY PRINTOUT TABLE 150

	SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
	50.000	202.00	37.50	.00	.00	.00	54.82	.00
*	150.000	202.00	37.56	.00	.06	.00	49.81	150.00
*	350.000	202.00	37.81	.00	.24	.00	37.79	200.00
*	500.000	202.00	38.30	.00	.50	.00	47.74	150.00
	510.000	202.00	38.32	.00	.02	.00	46.77	10.00
*	521.000	202.00	38.27	.00	-.06	.00	15.00	11.00
	624.000	202.00	38.84	.00	.57	.00	15.00	103.00
*	650.000	202.00	38.96	.00	.12	.00	49.39	26.00
*	800.000	202.00	39.12	.00	.16	.00	42.60	150.00
	950.000	202.00	39.64	.00	.52	.00	40.06	150.00
	980.000	202.00	39.76	.00	.12	.00	28.03	30.00
*	982.000	202.00	39.87	.00	.11	.00	25.97	3.00
*	1262.000	202.00	41.72	.00	1.84	.00	28.76	280.00
*	1338.000	240.00	42.40	.00	.68	.00	1267.07	76.00
	1468.000	240.00	42.40	.00	.00	.00	1266.71	130.00
*	1968.000	240.00	42.40	.00	.00	.00	519.67	500.00
*	976.000	240.00	42.23	.00	-.18	.00	30.06	176.00
*	1230.000	240.00	43.59	.00	1.37	.00	33.38	254.76
	1500.000	240.00	44.29	.00	.70	.00	49.69	270.00
	2000.000	240.00	45.24	.00	.95	.00	49.44	500.00
*	2110.000	240.00	45.45	.00	.21	.00	28.58	114.00
	900.000	240.00	45.88	.00	.43	.00	33.11	26.00
*	910.000	233.00	45.71	.00	-.17	.00	15.00	10.00
	1065.000	233.00	45.91	.00	.20	.00	15.00	155.00

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	SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
*	1105.000	233.00	46.63	.00	.71	.00	36.08	40.00
	1245.000	233.00	47.33	.00	.70	.00	29.67	140.00
	1400.000	233.00	48.15	.00	.83	.00	33.23	155.00
	1513.000	233.00	48.53	.00	.38	.00	35.22	113.00

1530.000	233.00	48.46	.00	-.08	.00	14.83	17.00
1531.000	233.00	48.45	.00	-.01	.00	14.83	1.00
1641.000	233.00	48.67	.00	.21	.00	14.83	110.00
1642.000	233.00	48.70	.00	.03	.00	14.83	1.00
1655.000	233.00	48.99	.00	.29	.00	34.76	13.00
1682.000	233.00	49.02	.00	.04	.00	30.61	27.00
1982.000	233.00	50.05	.00	1.03	.00	32.67	300.00
2282.000	233.00	50.94	.00	.89	.00	30.98	300.00
2582.000	233.00	51.81	.00	.87	.00	33.64	300.00
2883.000	233.00	52.52	.00	.71	.00	38.28	301.00
1010.000	233.00	52.48	.00	-.04	.00	14.83	40.00
1011.000	233.00	52.47	.00	-.01	.00	14.83	1.00
1133.000	233.00	52.69	.00	.22	.00	14.83	122.00
1134.000	233.00	52.73	.00	.03	.00	14.83	1.00
1149.000	233.00	53.02	.00	.29	.00	41.29	15.00
1184.000	233.00	53.08	.00	.07	.00	42.88	35.00
1443.000	233.00	53.86	.00	.77	.00	42.81	259.00
* 1739.000	219.00	54.42	.00	.56	.00	61.77	296.00
* 1779.000	219.00	54.43	.00	.02	.00	61.00	40.00
* 1780.000	219.00	54.43	.00	-.01	.00	61.00	1.00
1875.000	219.00	54.49	.00	.07	.00	58.60	95.00
* 1876.000	219.00	54.51	.00	.02	.00	58.84	1.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
* 1886.000	219.00	54.49	.00	-.02	.00	39.67	10.00
* 1916.000	219.00	54.51	.00	.02	.00	30.75	30.00
2191.000	219.00	55.89	.00	1.38	.00	33.33	275.00
2491.000	219.00	57.37	.00	1.48	.00	31.59	300.00
2691.000	219.00	58.62	.00	1.25	.00	28.62	200.00
1119.000	211.00	59.67	.00	1.06	.00	31.55	225.00
1542.000	211.00	60.24	.00	.56	.00	30.76	123.00
* 1880.000	211.00	61.47	.00	1.24	.00	35.63	338.00
* 2200.000	211.00	61.99	.00	.51	.00	39.53	320.00
* 2362.000	208.00	62.20	.00	.21	.00	25.69	162.00
* 2374.000	208.00	62.12	.00	-.08	.00	12.00	12.00
2440.000	208.00	62.08	.00	-.04	.00	12.00	68.00
* 2452.000	208.00	62.31	.00	.23	.00	23.18	12.00
* 2600.000	144.00	62.64	.00	.33	.00	37.25	148.00
* 3000.000	144.00	63.02	.00	.38	.00	27.43	400.00
* 3300.000	144.00	63.83	.00	.81	.00	23.39	300.00
3400.000	144.00	64.53	.00	.70	.00	24.46	100.00
* 3470.000	133.00	64.83	.00	.30	.00	32.23	70.00
* 1030.000	133.00	64.81	.00	-.02	.00	24.06	30.00
1145.000	133.00	65.10	.00	.29	.00	23.96	115.00
* 1300.000	133.00	65.12	.00	.02	.00	18.61	155.00
* 1349.000	133.00	66.11	.00	.99	.00	29.12	49.00

1388.000	133.00	66.14	.00	.02	.00	13.60	39.00
1389.000	133.00	66.14	.00	.00	.00	13.60	1.00
1492.000	133.00	66.52	.00	.38	.00	13.60	103.00
1493.000	133.00	66.53	.00	.01	.00	13.60	1.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
1502.000	133.00	67.02	.00	.48	.00	30.88	10.00
* 1700.000	133.00	68.29	.00	1.27	.00	22.55	202.00
1820.000	133.00	69.30	.00	1.01	.00	23.47	120.00
2000.000	133.00	70.43	.00	1.13	.00	23.89	180.00
2119.000	133.00	71.18	.00	.76	.00	28.08	119.00
* 2127.000	133.00	71.05	.00	-.13	.00	12.00	8.00
2218.000	133.00	71.85	.00	.80	.00	12.00	91.00
* 2228.000	133.00	72.16	.00	.31	.00	32.70	10.00
2400.000	133.00	72.72	.00	.56	.00	25.84	172.00
2600.000	133.00	73.61	.00	.89	.00	27.80	200.00
3000.000	133.00	75.71	.00	2.10	.00	24.69	400.00
3044.000	79.00	76.06	.00	.35	.00	26.64	44.00
* 3054.000	79.00	75.95	.00	-.11	.00	8.00	10.00
3148.000	79.00	76.34	.00	.40	.00	8.00	94.00
* 3158.000	79.00	76.65	.00	.30	.00	28.95	10.00
* 3500.000	79.00	77.95	.00	1.30	.00	20.26	342.00
* 3900.000	79.00	80.99	.00	3.04	.00	22.12	400.00
3952.000	79.00	81.28	.00	.29	.00	21.88	52.00
* 3962.000	79.00	81.03	.00	-.26	.00	7.00	10.00
* 4064.000	79.00	82.14	.00	1.11	.00	7.00	102.00
4074.000	79.00	82.48	.00	.34	.00	27.31	10.00
4200.000	79.00	82.73	.00	.25	.00	24.19	126.00
* 4600.000	79.00	84.48	.00	1.75	.00	20.69	400.00
5000.000	79.00	87.46	.00	2.97	.00	21.97	400.00
* 5198.000	66.00	88.97	.00	1.51	.00	20.36	198.00
* 9208.000	66.00	88.85	.00	-.12	.00	7.00	10.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
* 5318.000	66.00	89.75	.00	.90	.00	7.00	110.00
* 5328.000	66.00	90.05	.00	.30	.00	22.50	10.00
* 5500.000	55.00	90.67	.00	.62	.00	18.42	172.00
5800.000	55.00	93.37	.00	2.71	.00	19.04	300.00
5995.000	55.00	95.19	.00	1.82	.00	19.17	195.00
* 6000.000	55.00	95.21	.00	.01	.00	5.00	10.00
* 6104.000	55.00	96.34	.00	1.13	.00	5.00	104.00
6114.000	55.00	96.70	.00	.36	.00	26.94	10.00
* 6340.000	55.00	97.20	.00	.50	.00	17.10	226.00
* 6489.000	52.00	99.16	.00	1.96	.00	22.09	149.00
* 6499.000	52.00	99.04	.00	-.11	.00	5.00	10.00
* 6631.000	52.00	100.54	.00	1.50	.00	5.00	132.00

6641.000	52.00	100.86	.00	.32	.00	23.97	10.00
* 6740.000	52.00	101.02	.00	.16	.00	21.34	99.00
* 7000.000	52.00	102.54	.00	1.52	.00	17.18	260.00
* 7175.000	52.00	104.77	.00	2.23	.00	21.51	175.00
7500.000	52.00	106.22	.00	1.45	.00	20.21	325.00
* 7700.000	35.00	107.34	.00	1.11	.00	13.81	200.00
* 7731.000	35.00	107.76	.00	.43	.00	6.00	31.00
7732.000	35.00	107.99	.00	.23	.00	6.00	1.00
7835.000	35.00	108.77	.00	.78	.00	6.00	103.00
7836.000	35.00	108.79	.00	.02	.00	6.00	1.00
7855.000	35.00	109.30	.00	.51	.00	16.74	19.00
* 8025.000	35.00	111.03	.00	1.73	.00	16.73	175.00
* 8026.000	35.00	112.51	.00	1.48	.00	20.11	1.00
* 8240.000	35.00	114.71	.00	2.20	.00	20.80	214.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
* 8600.000	35.00	116.74	.00	2.03	.00	18.42	360.00
* 8780.000	35.00	117.96	.00	1.21	.00	19.19	180.00
-5328.000	35.00	90.05	.00	-27.91	.00	22.48	10.00
1040.000	11.00	90.07	.00	.03	.00	23.79	40.00
1300.000	11.00	90.09	.00	.02	.00	22.72	260.00
* 1600.000	11.00	90.13	.00	.04	.00	19.11	300.00
* 1900.000	11.00	90.23	.00	.10	.00	16.77	300.00
* 2200.000	11.00	90.47	.00	.24	.00	14.99	300.00
2283.000	11.00	90.58	.00	.11	.00	13.91	83.00
* 2293.000	11.00	90.57	.00	-.01	.00	9.16	10.00
* 2518.000	11.00	90.50	.00	-.07	.00	7.91	235.00
* 2528.000	11.00	90.51	.00	.01	.00	9.53	10.00
* 3028.000	11.00	91.84	.00	1.33	.00	14.12	500.00
* 3098.000	11.00	91.86	.00	.02	.00	21.33	70.00
* 3100.000	11.00	91.86	.00	.00	.00	21.35	2.00
* 3101.000	11.00	91.86	.00	.00	.00	21.35	1.00
* 3157.000	11.00	91.87	.00	.00	.00	19.59	57.00
3158.000	11.00	91.87	.00	.00	.00	19.59	1.00
* 3438.000	11.00	91.91	.00	.04	.00	13.23	280.00
* 3721.000	11.00	91.97	.00	.06	.00	23.23	283.00
* 3723.000	11.00	91.97	.00	.00	.00	23.25	2.00
* 3724.000	11.00	91.97	.00	.00	.00	23.24	1.00
3782.000	11.00	91.97	.00	.00	.00	22.88	58.00
* 3783.000	11.00	91.97	.00	.00	.00	22.88	1.00
* 3913.000	11.00	91.97	.00	-.01	.00	13.07	130.00
* 4143.000	11.00	92.10	.00	.13	.00	15.85	230.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
* 4233.000	11.00	92.13	.00	.03	.00	10.20	90.00

4413.000 11.00 92.20 .00 .07 .00 13.64 180.00

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO= 150.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 350.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 500.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 521.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 650.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 800.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 982.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1262.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
NOTE SECNO= 1338.000 PROFILE= 1 WSEL BASED ON X5 CARD
WARNING SECNO= 1338.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1968.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 976.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1230.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2110.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 910.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1105.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1739.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1779.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1780.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1876.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1886.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1916.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1880.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2200.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2362.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2374.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 2452.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2600.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3000.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3300.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3470.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1030.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 1300.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 1300.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 1300.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO= 1349.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1700.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2127.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2228.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3054.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3158.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3500.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 3900.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 3962.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 3962.000 PROFILE= 1 MINIMUM SPECIFIC ENERGY
WARNING SECNO= 4064.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 4600.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 5198.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 9208.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 9208.000 PROFILE= 1 MINIMUM SPECIFIC ENERGY
WARNING SECNO= 5318.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 5328.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 5500.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 6000.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 6000.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 6000.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

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WARNING SECNO= 6104.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 6340.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 6489.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 6499.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 6499.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 6499.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO= 6631.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 6740.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 7000.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 7175.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 7700.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 7731.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 7731.000 PROFILE= 1 MINIMUM SPECIFIC ENERGY
WARNING SECNO= 8025.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 8026.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 8026.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 8026.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO= 8240.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 8600.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 8780.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1600.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 1900.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2200.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2293.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 2518.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO= 2528.000 PROFILE= 1 CRITICAL DEPTH ASSUMED
CAUTION SECNO= 2528.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO= 2528.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

WARNING SECNO= 3028.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3098.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3100.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

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WARNING SECNO= 3101.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3157.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3438.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3721.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 3723.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3724.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3783.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 3913.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 4143.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO= 4233.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

HEC-RAS Version 4.1.0 Jan 2010
 U.S. Army Corps of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

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X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X   X
X   X   X       X   X   X   X   X   X   X
XXXXXXXX XXXX   X       XXX XXXX   XXXXXX   XXXX
X   X   X       X       X   X   X   X       X
X   X   X       X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
  
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PROJECT DATA

Project Title: City of Tracy - Outfall Channel
 Project File : Outfall11.prj
 Run Date and Time: 12/1/2010 12:38:52 PM

Project in English units

Project Description:

2.6 feet has been added to all elevations to get NAVD 88 . Starting WSE=20.22
 ES CH 100-yr

PLAN DATA

Plan Title: City of Tracy - Outfall Channel
 Plan File : v:\1840\active\184010207\analysis\ras\Outfall11.p01

Geometry Title: City of tracy - Outfall Channel
 Geometry File : v:\1840\active\184010207\analysis\ras\Outfall11.g01

Flow Title : City of Tracy - Outfall Channel
 Flow File : v:\1840\active\184010207\analysis\ras\Outfall11.f01

Plan Summary Information:

Number of: Cross Sections = 13 Multiple Openings = 0
 Culverts = 2 Inline Structures = 0
 Bridges = 1 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: City of Tracy - Outfall Channel
 Flow File : v:\1840\active\184010207\analysis\ras\Outfall11.f01

Flow Data (cfs)

River	Reach	RS	10-Yr
City Outfall	City Outfall	4200	154.6
City Outfall	City Outfall	2562	160.5

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
City Outfall	City Outfall	10-Yr		Known WS = 20.22

GEOMETRY DATA

Geometry Title: City of tracy - Outfall Channel
 Geometry File : v:\1840\active\184010207\analysis\ras\Outfall11.g01

CROSS SECTION

RIVER: City Outfall

REACH: City Outfall RS: 4200

INPUT

Description: Grant Line Rd - 2.6 ft is added to all elev to get NAVD 88

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	30.63	13.5	21.63	17.5	17.63
43.5	29.63			27.5	17.63
				31.5	21.63

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	13.5	.013	31.5	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	13.5	31.5		500	500		.1	.3

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 3700

INPUT

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	30.03	13.5	21.03	17.5	17.03
43.5	29.03			27.5	17.03
				31.5	21.03

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	13.5	.013	31.5	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	13.5	31.5		992	992		.1	.3

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2708

INPUT

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	28.84	13.5	19.84	17.5	15.84
43.5	27.84			27.5	15.84
				31.5	19.84

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	13.5	.013	31.5	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	13.5	31.5		20	20		.3	.5

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2688

INPUT

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	28.82	13.5	19.82	17.5	15.82
43.5	27.82			27.5	15.82
				31.5	19.82

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	13.5	.013	31.5	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	13.5	31.5		78	78		.3	.5

CULVERT

RIVER: City Outfall
REACH: City Outfall RS: 2650

INPUT

Description: Colony Drive									
Distance from Upstream XS =	1								
Deck/Roadway Width =	76								
Weir Coefficient =	2.6								
Upstream Deck/Roadway Coordinates									
num=	2								
Sta	Hi	Cord	Lo	Cord	Sta	Hi	Cord	Lo	Cord
0	25.1				200	25.1			

Upstream Bridge Cross Section Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev

0 28.82 13.5 19.82 17.5 15.82 27.5 15.82 31.5 19.82
43.5 27.82

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .02 13.5 .013 31.5 .02

Bank Sta: Left Right Coeff Contr. Expan.
13.5 31.5 .3 .5

Downstream Deck/Roadway Coordinates
num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 25.1 200 25.1

Downstream Bridge Cross Section Data
Station Elevation Data num= 6
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 27.73 12 19.73 16 15.73 26 15.73 30 19.73
38 27.73

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .02 12 .013 30 .02

Bank Sta: Left Right Coeff Contr. Expan.
12 30 .3 .5

Upstream Embankment side slope = 1.5 horiz. to 1.0 vertical
Downstream Embankment side slope = 1.5 horiz. to 1.0 vertical
Maximum allowable submergence for weir flow = .95
Elevation at which weir flow begins = 25.1
Energy head used in spillway design =
Spillway height used in design =
Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
Colony Drive Circular 5
FHWA Chart # 2 - Corrugated Metal Pipe Culvert
FHWA Scale # 2 - Mitered to conform to slope
Solution Criteria = Highest U.S. EG
Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
1 76 .013 .013 0 .5 1

Number of Barrels = 2
Upstream Elevation = 15.82
Centerline Stations
Sta. Sta.
19 25
Downstream Elevation = 15.73
Centerline Stations
Sta. Sta.
18 24

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2612

INPUT
Description:
Station Elevation Data num= 6
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 27.73 12 19.73 16 15.73 26 15.73 30 19.73
38 27.73

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .02 12 .013 30 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
12 30 50 50 50 .3 .5

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2562

INPUT
Description:
Station Elevation Data num= 6
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 27.68 12 19.68 16 15.68 26 15.68 30 19.68
38 27.68

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .02 12 .013 30 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
12 30 112 112 112 .3 .5

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2450

INPUT

Description:

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	27.57	12	19.57	16	15.57
38	27.57			26	15.57
				30	19.57

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	12	.013	30	.02

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	12	30		75	75	75		.3	.5

BRIDGE

RIVER: City Outfall
REACH: City Outfall RS: 2400

INPUT

Description:

Distance from Upstream XS = 50
 Deck/Roadway Width = 2.75
 Weir Coefficient = 2.6
 Upstream Deck/Roadway Coordinates
 num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
5	23.17	20.42	35	23.12	20.37

Upstream Bridge Cross Section Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	27.57	12	19.57	16	15.57
38	27.57			26	15.57
				30	19.57

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	12	.013	30	.02

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	12	30		.3	.5

Downstream Deck/Roadway Coordinates
 num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
5	23.17	20.42	35	23.12	20.37

Downstream Bridge Cross Section Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	27.5	12	19.5	16	15.5
38	27.5			26	15.5
				30	19.5

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	12	.013	30	.02

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	12	30		.3	.5

Upstream Embankment side slope = 1.5 horiz. to 1.0 vertical
 Downstream Embankment side slope = 1.5 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins = 23.17
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Bridge Coefficient Sets = 1

Low Flow Methods and Data

Energy
Selected Low Flow Methods = Highest Energy Answer

High Flow Method

Energy Only

Additional Bridge Parameters

Add Friction component to Momentum
 Do not add Weight component to Momentum
 Class B flow critical depth computations use critical depth
 inside the bridge at the upstream end
 Criteria to check for pressure flow = Upstream energy grade line

CROSS SECTION

RIVER: City Outfall

REACH: City Outfall RS: 2375

INPUT

Description: upstream of bottleneck

Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev
0	27.5	12	19.5	16	15.5
38	27.5			26	15.5
				30	19.5

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	12	.013	30	.02

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	12	30		257	257		.3	.5

CULVERT

RIVER: City Outfall
REACH: City Outfall RS: 2247.5

INPUT

Description: Bottleneck
 Distance from Upstream XS = 1
 Deck/Roadway Width = 255
 Weir Coefficient = 2.6
 Upstream Deck/Roadway Coordinates

num=		2	
Sta	Hi Cord	Lo Cord	Sta
0	29.1		200
			29.1

Upstream Bridge Cross Section Data		Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	27.5	12	19.5	16	15.5	26	15.5
38	27.5			30	19.5		

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	12	.013	30	.02

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	12	30		.3	.5

Downstream Deck/Roadway Coordinates

num=		2	
Sta	Hi Cord	Lo Cord	Sta
0	32.6		200
			32.6

Downstream Bridge Cross Section Data		Station Elevation Data		num=		6	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.14	18	19.14	22	15.14	32	15.14
54	31.14			36	19.14		

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
0	.02	18	.013	36	.02

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	18	36		.3	.5

Upstream Embankment side slope = 1.5 horiz. to 1.0 vertical
 Downstream Embankment side slope = 1.5 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins = 29.1
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name	Shape	Rise	Span
Bottleneck	Circular	7	

FHWA Chart # 2 - Corrugated Metal Pipe Culvert
 FHWA Scale # 2 - Mitered to conform to slope
 Solution Criteria = Highest U.S. EG

Culvert Upstrm Dist	Length	Top n	Bottom n	Depth Blocked	Entrance Loss Coef	Exit Loss Coef
1	255	.013	.013	0	.5	1

Upstream Elevation = 15.5
 Centerline Station = 21
 Downstream Elevation = 15.12
 Centerline Station = 27

CROSS SECTION

RIVER: City Outfall
REACH: City Outfall RS: 2120

INPUT

Description:
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
-----	------	-----	------	-----	------	-----	------	-----	------

0	31.14	18	19.14	22	15.14	32	15.14	36	19.14
54	31.14								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .02 18 .013 36 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 18 36 70 70 70 .3 .5

CROSS SECTION

RIVER: City Outfall
 REACH: City Outfall RS: 2050

INPUT
 Description:
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.07	18	19.07	22	15.07	32	15.07	36	19.07
54	31.07								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .02 18 .013 36 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 18 36 450 450 450 .3 .5

CROSS SECTION

RIVER: City Outfall
 REACH: City Outfall RS: 1600

INPUT
 Description:
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.62	18	18.62	22	14.62	32	14.62	36	18.62
54	30.62								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .02 18 .013 36 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 18 36 500 500 500 .1 .3

CROSS SECTION

RIVER: City Outfall
 REACH: City Outfall RS: 1100

INPUT
 Description:
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.12	18	18.12	22	14.12	32	14.12	36	18.12
54	30.12								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .02 18 .013 36 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 18 36 25 25 25 .1 .3

CROSS SECTION

RIVER: City Outfall
 REACH: City Outfall RS: 1075

INPUT
 Description: City Outfall at East Side Channel
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.1	18	18.1	22	14.1	32	14.1	36	18.1
54	30.1								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .02 18 .013 36 .02

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 18 36 0 0 0 .1 .3

SUMMARY OF MANNING'S N VALUES

River:City Outfall

City Outfall	2375	10-Yr	160.50	15.50	21.09	17.37	21.14	0.000043	1.88	87.72	21.97
0.15											
City Outfall	2247.5		Culvert								
City Outfall	2120	10-Yr	160.50	15.14	20.25		20.32	0.000062	2.10	77.83	21.33
0.18											
City Outfall	2050	10-Yr	160.50	15.07	20.25		20.31	0.000059	2.07	79.27	21.53
0.18											
City Outfall	1600	10-Yr	160.50	14.62	20.23		20.28	0.000042	1.87	88.89	22.83
0.15											
City Outfall	1100	10-Yr	160.50	14.12	20.22		20.26	0.000030	1.68	100.43	24.30
0.13											
City Outfall	1075	10-Yr	160.50	14.10	20.22	15.97	20.26	0.000030	1.68	100.90	24.36
0.13											

Profile Output Table - Standard Table 2

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Vel Head (ft)	Frctn Loss (ft)	C & E Loss (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Top	Width (ft)
City Outfall	4200	10-Yr	21.69	21.57	0.12	0.06	0.01		154.60			17.88
City Outfall	3700	10-Yr	21.62	21.53	0.09	0.06	0.01	0.05	154.50	0.05		19.50
City Outfall	2708	10-Yr	21.55	21.50	0.05	0.00	0.00	0.74	153.12	0.74		22.98
City Outfall	2688	10-Yr	21.55	21.50	0.05			0.76	153.08	0.76		23.04
City Outfall	2650		Culvert									
City Outfall	2612	10-Yr	21.18	21.12	0.06	0.00	0.00	0.51	153.78	0.31		21.48
City Outfall	2562	10-Yr	21.17	21.11	0.06	0.01	0.00	0.57	159.60	0.34		21.59
City Outfall	2450	10-Yr	21.17	21.11	0.06	0.00	0.01	0.66	159.45	0.39		21.85
City Outfall	2400		Bridge									
City Outfall	2375	10-Yr	21.14	21.09	0.05			0.70	159.38	0.42		21.97
City Outfall	2247.5		Culvert									
City Outfall	2120	10-Yr	20.32	20.25	0.07	0.00	0.00	0.32	159.85	0.32		21.33
City Outfall	2050	10-Yr	20.31	20.25	0.07	0.02	0.01	0.37	159.76	0.37		21.53
City Outfall	1600	10-Yr	20.28	20.23	0.05	0.02	0.00	0.72	159.06	0.72		22.83
City Outfall	1100	10-Yr	20.26	20.22	0.04	0.00	0.00	1.23	158.03	1.23		24.30
City Outfall	1075	10-Yr	20.26	20.22	0.04			1.26	157.99	1.26		24.36

HEC-RAS Version 4.1.0 Jan 2010
 U.S. Army Corps of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X   X
X   X   X       X   X   X   X   X   X   X
XXXXXXXX XXXX   X       XXX XXXX   XXXXXX   XXXX
X   X   X       X       X   X   X   X       X
X   X   X       X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
  
```

PROJECT DATA

Project Title: City of Tracy - Orchard PKW Reach C
 Project File : Orchard1.prj
 Run Date and Time: 12/1/2010 12:09:21 PM

Project in English units

Project Description:

For Reach C Channel, Top of bank elevation Up/s of GLR culvert is set as Starting WSE

PLAN DATA

Plan Title: Orchard PKW Reach C Channel
 Plan File : v:\1840\active\184010207\analysis\ras\Orchard1.p01

Geometry Title: Orchard PKW Reach C Channel
 Geometry File : v:\1840\active\184010207\analysis\ras\Orchard1.g01

Flow Title : Orchard PKW Reach C Channel
 Flow File : v:\1840\active\184010207\analysis\ras\Orchard1.f01

Plan Summary Information:

Number of: Cross Sections = 31 Multiple Openings = 0
 Culverts = 5 Inline Structures = 0
 Bridges = 0 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Orchard PKW Reach C Channel
 Flow File : v:\1840\active\184010207\analysis\ras\Orchard1.f01

Flow Data (cfs)

River	Reach	RS	100-Year
Reach C	R-1	5750	19.3
Reach C	R-1	2719	42

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Reach C	R-1	100-Year		Known WS = 24.28

GEOMETRY DATA

Geometry Title: Orchard PKW Reach C Channel
 Geometry File : v:\1840\active\184010207\analysis\ras\Orchard1.g01

CROSS SECTION

RIVER: Reach C

REACH: R-1 RS: 5750

INPUT

Description: Sta 57+50 downstream Byron Road 36" Culvert

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	36	1	36	18.5	30	30	30	47.5	36
60	36								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		250	250		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 5500

INPUT

Description: Sta 55+00

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	34.96	1	34.96	18.5	28.96	30	28.96	47.5	34.96
60	34.96								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		200	200		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 5300

INPUT

Description: Sta 53+00

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	34.56	1	34.56	18.5	28.56	30	28.56	47.5	34.56
60	34.56								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		300	300		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 5000

INPUT

Description: Sta 50+00

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	33.96	1	33.96	18.5	27.96	30	27.96	47.5	33.96
60	33.96								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		300	300		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 4700

INPUT

Description: Sta 47+00

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	33.36	1	33.36	18.5	27.36	30	27.36	47.5	33.36
60	33.36								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		200	200		.1	.3

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 4500

INPUT

Description: Sta 45+00
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	32.97	1	32.97	18.5	26.97	30	26.97	47.5	32.97
60	32.97								

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 1 60 95 95 95 .1 .3

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 4395

INPUT

Description: Sta 43+95 Upstream of W. Lowell Ave 42" RCP
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	32.78	1	32.78	18.5	26.78	30	26.78	47.5	32.78
60	32.78								

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 1 60 122 122 122 .1 .3

CULVERT

RIVER: Reach C
 REACH: R-1 RS: 4350

INPUT

Description:
 Distance from Upstream XS = 1
 Deck/Roadway Width = 120
 Weir Coefficient = 2.6
 Upstream Deck/Roadway Coordinates num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	32.78		60	32.78	

Upstream Bridge Cross Section Data
 Station Elevation Data num= 6

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	32.78	1	32.78	18.5	26.78	30	26.78	47.5	32.78
60	32.78								

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Coeff Contr. Expan.
 1 60 .1 .3

Downstream Deck/Roadway Coordinates num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	32.53		60	32.53	

Downstream Bridge Cross Section Data
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	32.53	9	32.53	23.5	29.53	23.5	26.53	35	26.53
55	32.53	60	32.53						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	9	.05	60	.05

Bank Sta: Left Right Coeff Contr. Expan.
 9 60 .1 .3

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .98
 Elevation at which weir flow begins = 32.53
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
Culvert #1 Circular 3
FHWA Chart # 1 - Concrete Pipe Culvert
FHWA Scale # 1 - Square edge entrance with headwall
Solution Criteria = Highest U.S. EG
Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
 1 120 .013 .013 0 .5 1
Upstream Elevation = 26.78
 Centerline Station = 24.25
Downstream Elevation = 26.53
 Centerline Station = 29.25

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 4273

INPUT
Description: Sta 42+73 Downstream of W. Lowell Ave 42" RCP
Station Elevation Data num= 7
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 32.53 9 32.53 23.5 29.53 23.5 26.53 35 26.53
 55 32.53 60 32.53
Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 9 .05 60 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 9 60 73 73 73 .1 .3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 4200

INPUT
Description: Sta 42+00
Station Elevation Data num= 7
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 32.37 9 32.37 23.5 29.37 23.5 26.37 35 26.37
 55 32.37 60 32.37
Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 9 .05 60 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 9 60 200 200 200 .1 .3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 4000

INPUT
Description: Sta 40+00
Station Elevation Data num= 7
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 31.98 9 31.98 23.5 28.98 23.5 25.98 35 25.98
 55 31.98 60 31.98
Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 9 .05 55 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 9 55 150 150 150 .1 .3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 3850

INPUT
Description: Sta 38+50
Station Elevation Data num= 7
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 31.69 9 31.69 23.5 28.69 23.5 25.69 35 25.69
 55 31.66 60 31.69
Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 9 .05 55 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 9 55 100 100 100 .1 .3

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 3750

INPUT

Description: Sta 37+50 Uptream of W. Lowell Ave Culvert

Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.49	1	31.49	19.5	25.49	31	25.49	41	28.49
51	28.49	60	31.49						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

1	60	146	146	146	.1	.3
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CULVERT

RIVER: Reach C
 REACH: R-1 RS: 3700

INPUT

Description: West Lowell Ave 1-42" RCP

Distance from Upstream XS = 1
 Deck/Roadway Width = 144
 Weir Coefficient = 2.6
 Upstream Deck/Roadway Coordinates

num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	31.49		60	31.49	

Upstream Bridge Cross Section Data

Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.49	1	31.49	19.5	25.49	31	25.49	41	28.49
51	28.49	60	31.49						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Coeff Contr. Expan.

1	60	.1	.3
---	----	----	----

Downstream Deck/Roadway Coordinates

num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	31.16		60	31.16	

Downstream Bridge Cross Section Data

Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.16	1	31.16	19.5	25.16	31	25.16	41	28.16
51	28.16	60	31.16						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Coeff Contr. Expan.

1	60	.1	.3
---	----	----	----

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .98
 Elevation at which weir flow begins = 31.16
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
 Culvert #1 Circular 3.5
 FHWA Chart # 1 - Concrete Pipe Culvert
 FHWA Scale # 1 - Square edge entrance with headwall
 Solution Criteria = Highest U.S. EG
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
 1 144 .013 .013 0 .5 1
 Upstream Elevation = 25.49
 Centerline Station = 25.25
 Downstream Elevation = 25.16
 Centerline Station = 25.25

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 3604

INPUT
 Description: Sta 36+04 downstream of W. Lowell Ave Culvert
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.16	1	31.16	19.5	25.16	31	25.16	41	28.16
51	28.16	60	31.16						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1	60	104	104	104	.1	.3	

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 3500

INPUT
 Description: Sta 35+00
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.99	1	30.99	19.5	24.99	31	24.99	41	27.99
51	27.99	60	30.99						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1	60	200	200	200	.1	.3	

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 3300

INPUT
 Description: Sta 33+00
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.6	1	30.6	19.5	24.6	31	24.6	41	27.6
51	27.6	60	30.6						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1	60	300	300	300	.1	.3	

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 3000

INPUT
 Description: Sta 30+00 Jenni Lane
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30.01	1	30.01	19.5	24.01	31	24.01	41	27.01
51	27.01	60	30.01						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1	60	150	150	150	.1	.3	

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 2850

INPUT
 Description: Sta 28+50
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	29.71	1	29.71	19.5	23.71	31	23.71	41	26.71
51	26.71	60	29.71						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1	60	131	131	131	.1	.3	

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2719

INPUT
Description: Sta 27+19 Upstream of Hillcrest Drive Culvert
Station Elevation Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 29.45 1 29.45 19.5 23.45 31 23.45 41 26.45
51 26.45 60 29.45
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 60 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
1 60 92 92 92 .1 .3

CULVERT

RIVER: Reach C
REACH: R-1 RS: 2700

INPUT
Description: Hillcrest Drive 1-60" RCP
Distance from Upstream XS = 1
Deck/Roadway Width = 90
Weir Coefficient = 2.6
Upstream Deck/Roadway Coordinates num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 29.45 60 29.45
Upstream Bridge Cross Section Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 29.45 1 29.45 19.5 23.45 31 23.45 41 26.45
51 26.45 60 29.45
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 60 .05
Bank Sta: Left Right Coeff Contr. Expan.
1 60 .1 .3

Downstream Deck/Roadway Coordinates num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 29.34 60 29.34

Downstream Bridge Cross Section Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 29.34 1 29.34 19.5 23.34 31 23.34 41 26.34
51 26.34 60 29.34
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 60 .05
Bank Sta: Left Right Coeff Contr. Expan.
1 60 .1 .3

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
Downstream Embankment side slope = 0 horiz. to 1.0 vertical
Maximum allowable submergence for weir flow = .98
Elevation at which weir flow begins = 29.34
Energy head used in spillway design =
Spillway height used in design =
Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
Culvert #1 Circular 5
FHWA Chart # 1 - Concrete Pipe Culvert
FHWA Scale # 1 - Square edge entrance with headwall
Solution Criteria = Highest U.S. EG
Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
1 90 .013 .013 0 .5 1
Upstream Elevation = 23.45
Centerline Station = 25.25
Downstream Elevation = 23.34
Centerline Station = 25.25

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2627

INPUT

Description: Sta 26+27 Downstream of Hillcrest Drive Culvert

Station Elevation Data num= 7									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	29.34	1	29.34	19.5	23.34	31	23.34	41	26.34
51	26.34	60	29.34						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		114 114	114		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2450

INPUT
Description: Sta 24+50

Station Elevation Data num= 7									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	28.81	1	28.81	19.5	22.81	31	22.81	41	25.81
51	25.81	60	28.81						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		114 114	114		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2306

INPUT
Description: Sta 23+06 Top of cut-off wall

Station Elevation Data num= 7									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	28.37	1	28.37	19.5	22.37	31	22.37	41	25.37
51	25.37	60	28.37						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		1 1	1		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2305

INPUT
Description: Sta 23+05 downstream of cut-off wall

Station Elevation Data num= 7									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	26.99	1	26.99	19.5	20.99	31	20.99	41	23.99
51	23.99	60	26.99						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		105 105	105		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 2200

INPUT
Description: Sta 22+00

Station Elevation Data num= 7									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	26.79	1	26.79	19.5	20.79	31	20.79	41	23.79
51	23.79	60	26.79						

Manning's n Values num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	60	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	60		300 300	300		.1	.3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 1900

INPUT
Description: Sta 19+00
Station Elevation Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 26.19 1 26.19 19.5 20.19 31 20.19 41 23.19
51 23.19 60 26.19
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 60 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
1 60 200 200 200 .1 .3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 1700

INPUT
Description: Sta 17+00
Station Elevation Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 25.79 1 25.79 19.5 19.79 31 19.79 41 22.79
51 22.79 60 25.79
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 60 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
1 60 98 98 98 .1 .3

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 1602

INPUT
Description: Upstream of Joseph Damon Drive 2-60" RCP Culvert
Station Elevation Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 25.23 1 25.23 19.5 19.23 31 19.23 31 20.23
50 25.23 60 25.23
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 50 .05
Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
1 50 84 84 84 .1 .3

CULVERT

RIVER: Reach C
REACH: R-1 RS: 1550

INPUT
Description: Joseph Damon Drive 2-60" RCP
Distance from Upstream XS = 1
Deck/Roadway Width = 82
Weir Coefficient = 2.6
Upstream Deck/Roadway Coordinates num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 25.23 50 25.23
Upstream Bridge Cross Section Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 25.23 1 25.23 19.5 19.23 31 19.23 31 20.23
50 25.23 60 25.23
Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 50 .05
Bank Sta: Left Right Coeff Contr. Expan.
1 50 .1 .3
Downstream Deck/Roadway Coordinates num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 25.02 50 25.02
Downstream Bridge Cross Section Data num= 7
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev

0	25.02	1	25.02	19.5	19.02	31	19.02	31	20.02
50	25.02	60	25.02						

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 1 .05 50 .05

Bank Sta: Left Right Coeff Contr. Expan.
 1 50 .1 .3

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .98
 Elevation at which weir flow begins = 25.02
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
 Culvert #1 Circular 5
 FHWA Chart # 1 - Concrete Pipe Culvert
 FHWA Scale # 1 - Square edge entrance with headwall
 Solution Criteria = Highest U.S. EG
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
 1 82 .013 .013 0 .5 1

Number of Barrels = 2
 Upstream Elevation = 19.23

Centerline Stations
 Sta. Sta.
 22 28.5
 Downstream Elevation = 19.02

Centerline Stations
 Sta. Sta.
 22 28.5

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 1518

INPUT
 Description: Downstream of Joseph Damon Drive 2-60" RCP Culvert
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	25.02	1	25.02	19.5	19.02	31	19.02	31	20.02
50	25.02	60	25.02						

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 1 .05 50 .05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 1 50 118 118 118 .1 .3

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 1400

INPUT
 Description: Sta14+00
 Station Elevation Data num= 7

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	24.78	1	24.78	19.5	18.78	31	18.78	31	19.78
50	24.78	60	24.78						

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 1 .05 50 .05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 1 50 115 115 115 .1 .3

CROSS SECTION

RIVER: Reach C
 REACH: R-1 RS: 1285

INPUT
 Description: Sta 12+85 Downstream of 48" culvert
 Station Elevation Data num= 8

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	24.51	1	24.51	19.5	21.51	19.5	18.51	31	18.51
31	21.51	52	24.51	62	24.51				

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .05 1 .05 52 .05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

1 52 67 67 67 .1 .3

CULVERT

RIVER: Reach C
REACH: R-1 RS: 1250

INPUT
Description: 48" RCP Just Up/s of GLR
Distance from Upstream XS = 1
Deck/Roadway Width = 65
Weir Coefficient = 2.6
Upstream Deck/Roadway Coordinates
num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 24.51 52 24.51

Upstream Bridge Cross Section Data
Station Elevation Data num= 8
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 24.51 1 24.51 19.5 21.51 19.5 18.51 31 18.51
31 21.51 52 24.51 62 24.51

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 52 .05

Bank Sta: Left Right Coeff Contr. Expan.
1 52 .1 .3

Downstream Deck/Roadway Coordinates
num= 2
Sta Hi Cord Lo Cord Sta Hi Cord Lo Cord
0 24.38 52 24.38

Downstream Bridge Cross Section Data
Station Elevation Data num= 8
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 24.38 1 24.38 19.5 21.38 19.5 18.38 31 18.38
31 21.38 52 24.38 62 24.38

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 52 .05

Bank Sta: Left Right Coeff Contr. Expan.
1 52 .1 .3

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
Downstream Embankment side slope = 0 horiz. to 1.0 vertical
Maximum allowable submergence for weir flow = .98
Elevation at which weir flow begins = 24.38
Energy head used in spillway design =
Spillway height used in design =
Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
Culvert #1 Circular 4
FHWA Chart # 1 - Concrete Pipe Culvert
FHWA Scale # 1 - Square edge entrance with headwall
Solution Criteria = Highest U.S. EG
Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
1 65 .013 .013 0 .5 1
Upstream Elevation = 18.51
Centerline Station = 25.25
Downstream Elevation = 18.38
Centerline Station = 25.25

CROSS SECTION

RIVER: Reach C
REACH: R-1 RS: 1218

INPUT
Description: Sta 12+18 (2.6 ft is added to all elev to get NAVD 88)
Station Elevation Data num= 8
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
0 24.38 1 24.38 19.5 21.38 19.5 18.38 31 18.38
31 21.38 52 24.38 62 24.38

Manning's n Values num= 3
Sta n Val Sta n Val Sta n Val
0 .05 1 .05 52 .05

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
1 52 78 78 78 .1 .3

CROSS SECTION

RIVER: Reach C

REACH: R-1 RS: 1140

INPUT
 Description: Sta 11+40 Up/s of GLR, Top of bank set as Known WSE
 2.6 Feet is

added to all elevations to get to NAVD 88

Station Elevation Data		num= 8							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	24.28	1	24.28	19.5	21.28	19.5	18.28	31	18.28
31	21.28	52	24.28	62	24.28				

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
0	.05	1	.05	52	.05

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	1	52		0	0		.1	.3

SUMMARY OF MANNING'S N VALUES

River: Reach C

Reach	River Sta.	n1	n2	n3
R-1	5750	.05	.05	.05
R-1	5500	.05	.05	.05
R-1	5300	.05	.05	.05
R-1	5000	.05	.05	.05
R-1	4700	.05	.05	.05
R-1	4500	.05	.05	.05
R-1	4395	.05	.05	.05
R-1	4350	Culvert		
R-1	4273	.05	.05	.05
R-1	4200	.05	.05	.05
R-1	4000	.05	.05	.05
R-1	3850	.05	.05	.05
R-1	3750	.05	.05	.05
R-1	3700	Culvert		
R-1	3604	.05	.05	.05
R-1	3500	.05	.05	.05
R-1	3300	.05	.05	.05
R-1	3000	.05	.05	.05
R-1	2850	.05	.05	.05
R-1	2719	.05	.05	.05
R-1	2700	Culvert		
R-1	2627	.05	.05	.05
R-1	2450	.05	.05	.05
R-1	2306	.05	.05	.05
R-1	2305	.05	.05	.05
R-1	2200	.05	.05	.05
R-1	1900	.05	.05	.05
R-1	1700	.05	.05	.05
R-1	1602	.05	.05	.05
R-1	1550	Culvert		
R-1	1518	.05	.05	.05
R-1	1400	.05	.05	.05
R-1	1285	.05	.05	.05
R-1	1250	Culvert		
R-1	1218	.05	.05	.05
R-1	1140	.05	.05	.05

SUMMARY OF REACH LENGTHS

River: Reach C

Reach	River Sta.	Left	Channel	Right
R-1	5750	250	250	250
R-1	5500	200	200	200
R-1	5300	300	300	300
R-1	5000	300	300	300
R-1	4700	200	200	200
R-1	4500	95	95	95
R-1	4395	122	122	122
R-1	4350	Culvert		
R-1	4273	73	73	73
R-1	4200	200	200	200
R-1	4000	150	150	150
R-1	3850	100	100	100
R-1	3750	146	146	146
R-1	3700	Culvert		
R-1	3604	104	104	104
R-1	3500	200	200	200
R-1	3300	300	300	300
R-1	3000	150	150	150
R-1	2850	131	131	131
R-1	2719	92	92	92
R-1	2700	Culvert		
R-1	2627	114	114	114
R-1	2450	114	114	114

R-1	3500	100-Year	19.30	24.99	26.41		26.43	0.000720	0.84	22.90	20.64
0.14											
R-1	3300	100-Year	19.30	24.60	26.32		26.33	0.000359	0.66	29.25	22.53
0.10											
R-1	3000	100-Year	19.30	24.01	26.26		26.26	0.000129	0.46	42.15	25.95
0.06											
R-1	2850	100-Year	19.30	23.71	26.25		26.25	0.000081	0.39	49.82	27.78
0.05											
R-1	2719	100-Year	42.00	23.45	26.22	24.15	26.23	0.000272	0.74	56.40	29.26
0.09											
R-1	2700		Culvert								
R-1	2627	100-Year	42.00	23.34	24.89		24.93	0.002529	1.65	25.43	21.41
0.27											
R-1	2450	100-Year	42.00	22.81	24.71		24.73	0.001169	1.26	33.40	23.68
0.19											
R-1	2306	100-Year	42.00	22.37	24.62		24.64	0.000610	1.00	42.15	25.95
0.14											
R-1	2305	100-Year	42.00	20.99	24.63		24.63	0.000098	0.46	90.68	44.64
0.06											
R-1	2200	100-Year	42.00	20.79	24.62		24.62	0.000075	0.42	99.33	45.80
0.05											
R-1	1900	100-Year	42.00	20.19	24.61		24.61	0.000036	0.33	127.20	49.37
0.04											
R-1	1700	100-Year	42.00	19.79	24.60		24.60	0.000024	0.29	147.15	51.77
0.03											
R-1	1602	100-Year	42.00	19.23	24.60	19.95	24.60	0.000022	0.29	142.44	44.65
0.03											
R-1	1550		Culvert								
R-1	1518	100-Year	42.00	19.02	24.57		24.57	0.000019	0.28	150.56	45.89
0.03											
R-1	1400	100-Year	42.00	18.78	24.57		24.57	0.000016	0.26	161.68	47.53
0.02											
R-1	1285	100-Year	42.00	18.51	24.56	19.25	24.56	0.000039	0.32	131.52	62.00
0.04											
R-1	1250		Culvert								
R-1	1218	100-Year	42.00	18.38	24.28		24.29	0.000046	0.34	123.38	49.73
0.04											
R-1	1140	100-Year	42.00	18.28	24.28	19.02	24.28	0.000042	0.33	128.25	51.00
0.04											

Profile Output Table - Standard Table 2

Reach	River Sta	Profile	E.G. Elev (ft)	W.S. Elev (ft)	Vel Head (ft)	Frctn Loss (ft)	C & E Loss (ft)	Q Left (cfs)	Q Channel (cfs)	Q Right (cfs)	Top Width (ft)
R-1	5750	100-Year	30.85	30.81	0.05	0.78	0.01		19.30		16.21
R-1	5500	100-Year	30.07	30.05	0.02	0.39	0.00		19.30		17.85
R-1	5300	100-Year	29.68	29.66	0.02	0.43	0.00		19.30		17.93
R-1	5000	100-Year	29.25	29.23	0.02	0.19	0.00		19.30		18.92
R-1	4700	100-Year	29.05	29.05	0.01	0.06	0.00		19.30		21.34
R-1	4500	100-Year	29.00	28.99	0.00	0.02	0.00		19.30		23.30
R-1	4395	100-Year	28.98	28.98	0.00				19.30		24.32
R-1	4350		Culvert								
R-1	4273	100-Year	27.87	27.86	0.02	0.08	0.00		19.30		15.92
R-1	4200	100-Year	27.79	27.78	0.02	0.15	0.00		19.30		16.19
R-1	4000	100-Year	27.65	27.64	0.01	0.07	0.00		19.30		17.02
R-1	3850	100-Year	27.58	27.57	0.01	0.02	0.00		19.30		17.80
R-1	3750	100-Year	27.55	27.55	0.00				19.30		24.71
R-1	3700		Culvert								
R-1	3604	100-Year	26.51	26.50	0.01	0.08	0.00		19.30		20.08
R-1	3500	100-Year	26.43	26.41	0.01	0.10	0.00		19.30		20.64
R-1	3300	100-Year	26.33	26.32	0.01	0.06	0.00		19.30		22.53
R-1	3000	100-Year	26.26	26.26	0.00	0.02	0.00		19.30		25.95
R-1	2850	100-Year	26.25	26.25	0.00	0.02	0.00		19.30		27.78
R-1	2719	100-Year	26.23	26.22	0.01				42.00		29.26
R-1	2700		Culvert								
R-1	2627	100-Year	24.93	24.89	0.04	0.19	0.01		42.00		21.41
R-1	2450	100-Year	24.73	24.71	0.02	0.09	0.00		42.00		23.68
R-1	2306	100-Year	24.64	24.62	0.02	0.00	0.00		42.00		25.95
R-1	2305	100-Year	24.63	24.63	0.00	0.01	0.00		42.00		44.64
R-1	2200	100-Year	24.62	24.62	0.00	0.02	0.00		42.00		45.80
R-1	1900	100-Year	24.61	24.61	0.00	0.01	0.00		42.00		49.37
R-1	1700	100-Year	24.60	24.60	0.00	0.00	0.00		42.00		51.77
R-1	1602	100-Year	24.60	24.60	0.00				42.00		44.65
R-1	1550		Culvert								
R-1	1518	100-Year	24.57	24.57	0.00	0.00	0.00		42.00		45.89
R-1	1400	100-Year	24.57	24.57	0.00	0.00	0.00		42.00		47.53
R-1	1285	100-Year	24.56	24.56	0.00			0.00	41.98	0.01	62.00
R-1	1250		Culvert								
R-1	1218	100-Year	24.29	24.28	0.00	0.00	0.00		42.00		49.73
R-1	1140	100-Year	24.28	24.28	0.00				42.00		51.00

APPENDIX D

2010 DRAINAGE AGREEMENT
BETWEEN CITY OF TRACY AND WEST SIDE IRRIGATION DIST.

**2010 DRAINAGE AGREEMENT
BETWEEN THE CITY OF TRACY
AND THE WEST SIDE IRRIGATION DISTRICT**

Recitals

1. Definitions
2. Discharge Limits
3. Westside Channel Area, Main Drain: Connection Fee and O&M Fees
4. Westside Channel Area Drainage Improvements
5. Lammers Area Provisions.
6. Routine Maintenance
7. Connection Costs
8. Meters
9. Water Quality
10. Restrictions on Relocation
11. Prior Agreement
12. Disputes
13. Attorneys Fees
14. Right of First Refusal
15. Hold Harmless
16. Modifications
17. Binding on Successors

Exhibit A: Diagram Showing Defined Areas, Channels and Drains

2010 DRAINAGE AGREEMENT
BETWEEN THE CITY OF TRACY
AND THE WEST SIDE IRRIGATION DISTRICT

This 2010 Drainage Agreement ("2010 Agreement") is entered into this 21st day of ~~November~~ ^{December}, by and between the WEST SIDE IRRIGATION DISTRICT ("District"), a political subdivision of the State of California, and the CITY OF TRACY ("City"), a municipal corporation.

RECITALS

This 2010 Agreement is based on the following facts:

- A. District operates a drainage system pursuant to Division 11 of the California Water Code; and
- B. The City of Tracy is a general law City duly organized as a municipal corporation which operates a storm drain system; and
- C. District and City are parties to a sequence of agreements beginning in 1972 which provide for the joint use of certain drainage systems:
 - (1) a Prior Agreement (date not known). See Section 11 below.
 - (2) an Agreement dated October 4, 1972, which provides for joint use of certain drainage systems ("Original 1972 Agreement");
 - (3) Amendment No. 1, entered into on September 8, 1999;
 - (4) Amendment No. 2, entered into on December 6, 1999;
 - (5) Amendment No. 3, entered into on December 12, 2001; and
 - (6) Integrated Amended 1972 Drainage Agreement, entered into in June, 2002, that consolidated terms from the Original 1972 Agreement and Amendments for ease of reference.
- D. The parties have negotiated additional terms that they wish to incorporate into this 2010 Agreement. This 2010 Agreement supersedes the Original 1972 Agreement, Amendments 1, 2 and 3, and the Integrated Amended 1972 Drainage Agreement, all referenced in Recital Section C (2) through (6) above.

NOW THEREFORE, the parties agree as follows:

- 1. Definitions. In this 2010 Agreement unless the context otherwise requires:
 - a. "Connection Fees" means the initial fee that has already been paid by City to District for the Westside Channel Area (\$800,000) and the fee being paid pursuant to this 2010 Agreement for the Lammers Area (\$240,000) as a part of obtaining authorization for discharges to District facilities. See Sections 3, 5.a and 7.
 - b. "Drainage Fee" means the fees paid from City to the District regarding the Lammers Area, under Section 5.b of this 2010 Agreement.
 - c. "Eastside Channel" means the City owned and operated open channel that generally serves as the storm drainage outfall for the east half of the City, discharging into Old River via the Sugar Cut.

- d. "Lammers Area" means the portions of the Lammers Watershed shown on Exhibit A, attached.
- e. "Main Drain" means the District's owned and operated Main Drain Outfall System of which the primary function has been for the collection and conveyance of irrigation tailwater for eventual discharge into Old River at the Wicklund Road outfall, near Wicklund Road and Bethany Road northwest of the City. The Main Drain consists of underground storm drains located within the Westside Channel Area, an underground 72" storm drain extending west along Grant Line Road toward Lammers Road, an open ditch that extends northwesterly from the end of the 72" storm drain to the Wicklund Road outfall, and the Drainage Improvements constructed by the City under Section 4.
- f. "O & M Fees" means the fees paid by the City to the District for operation, maintenance and repair of the District's Main Drain, in the Westside Channel Area, as set forth in Section 3 of this 2010 Agreement.
- g. "Option Fee" means the fee paid by City to the District regarding the Lammers Area, under potential circumstances described Section 5.d.
- h. "Sub-Main Drain" means the District's owned and operated Sub-Main Drain tributary to the Main Drain of which the primary function has been for the collection and conveyance of irrigation tailwater for eventual discharge to the Main Drain. The Sub-Main Drain consists of a 60" irrigation pipeline extending underneath Interstate 205 from Lammers Road just south of Byron Road, discharging to an open ditch that extends northwest and parallel to the south side of Byron Road and then north along the east side of San Jose Road, joining the Main Drain roughly 1,800 feet north of Byron Road.
- i. "Westside Channel Area" means roughly a 2-square mile area occupying the northeastern most quadrant of the Westside Channel Watershed as defined in the City's Storm Drainage Master Plan. The majority of the property to be served is west of Tracy Blvd., south of 1-205, east of ½ mile west of Corral Hollow Road and north of 11th Street. Another portion extends south of 11th Street for roughly ¾ mile on both sides of Tracy Blvd. The Westside Channel Area is depicted on Exhibit "A", attached.

2. Discharge Limits. Each party may discharge up to the following amounts of storm water runoff, in cubic feet per second (cfs), subject to the terms of this 2010 Agreement:

- a. City may discharge:
 - (1) 20 cfs into the District's Main Drain, as an irrevocable license to discharge originally established under the Original 1972 Agreement and continuing under this 2010 Agreement; and
 - (2) an additional 125 cfs into the District's Main Drain; and
 - (3) up to 30 cfs from the Lammers Area to District's Sub-Main Drain, subject to Section 5, as long as the maximum City discharge to the Main Drain from all sources (below the Sub-Main Drain), does not exceed 145 cfs.
- b. District may discharge:
 - (1) 20 cfs into the City's Eastside Channel, as an irrevocable license to discharge originally established under the Original 1972 Agreement, and continuing under this 2010 Agreement; and
 - (2) an additional 15 cfs into City's Eastside Channel.

c. Point of Discharge. Regarding the Westside Channel drainage rights granted pursuant to the Original 1972 Agreement, the City may discharge its water into the facilities of the District at any point(s) west of Tracy Boulevard, and the District shall discharge its water at the eastern end of the extension of the 48" drain line of the Southern Pacific Company, located on Grant Line Road east of the Southern Pacific Spur. In this regard, District has constructed an extension of the 48" line of the Southern Pacific Company, 545 feet to the east of its eastern end, and has dedicated the line to the City. District has entered into a number of agreements permitting residential developers to discharge into portions of its Main Drain system. Except as specifically set forth in this 2010 Agreement, District agrees that it will not enter into additional such agreements with developers within the Westside Channel Area or Lammers Area without the written consent of the City.

3. Westside Channel Area, Main Drain: Connection Fee and O&M Fees.

City has paid to the District a one-time Connection Fee of \$800,000, in 1999. In addition, the City shall make the following payments to the District as its agreed upon contribution towards operation and maintenance (O&M) of the District's drainage facilities serving the Westside Channel Area. City has the right, but not the obligation, to periodically inspect the downstream channel system to assure that maintenance is adequate. So long as the channel downstream of the Main Drain is maintained by District, City shall have no authority to direct how the District uses the O&M Fees.

a. O&M Fees through 2020. City paid District \$200,000 before December 31, 1999, constituting payment in advance of O&M Fees through December 31, 2009. City paid District \$300,000 before December 31, 2009, constituting payment in advance of O&M Fees through December 31, 2020.

b. O&M Fees after 2020. On or before December 31, 2020, and on or before each December 31 thereafter, City shall make annual payments to District in advance for routine O&M Fees for the following calendar year. The initial annual payment shall be in the amount of \$35,000 and this amount shall be increased by a rate of \$1,000 every two years. The parties further agree that these sums are the best estimate of the actual, routine O&M fees that will be expended by the District.

c. Extraordinary O&M after 2020. The parties acknowledge that after 2020 District may be required to undertake extraordinary maintenance expenditures with regard to the District's Drainage Facilities, due to the occurrence of circumstances which are not foreseeable. City agrees that it shall be responsible for paying the costs associated with any required extraordinary maintenance. District shall advise City of its intention to perform any extraordinary O&M. Within 30 days of the end of the calendar year for which such extraordinary maintenance was undertaken, District shall provide City with an invoice and supporting documentation for amounts actually spent by District for extraordinary maintenance of the Main Drain and related facilities during the preceding year, and City shall reimburse District for any such amounts within 30 days of the invoice.

d. Payments for Upgrades. City shall be responsible for paying the costs associated with any required upgrades to the District's Main Drain and Related Facilities necessary to comply with any local, state or federal mandated requirements on municipal and industrial dischargers.

4. Westside Channel Area Drainage Improvements. The City was required to and did (at its own expense) construct the following Drainage Improvements associated with discharge to the District's Main Drain. District has accepted these improvements and City warranted the improvements against defects in materials or workmanship for a period of one year from the date the improvements were completed and accepted by District.

a. Drainage Structure Crossings: City enlarged the following drainage structure crossings of

the Main Drain: (1) at the Grant Line Road/Lammers Road intersection; (2) at a farm access road located roughly 1000 feet west of Lammers Road; (3) at San Jose Road; and (4) at Reeve Road. In each instance, the existing drainage structures were replaced with a new 72" reinforced concrete pipe.

- b. Additional Drainage Structure Crossing: The City constructed an additional Farm Road Culvert Crossing northwest of Reeve Road, as shown on PP-5 of the Design Package for Drainage Improvements along the WSID Main Drain, Project 84010017 dated 10/22/01.
- c. Intake Canal: City replaced an existing 60" pipe with a 72" reinforced concrete pipe at the outlet of the Main Drain into the Intake Canal. The City also installed stabilization material at the inlet and outlet of the existing drainage structure entering the District's Intake Canal, and provided similar stabilization on the opposite west bank of the Intake Canal for a length of less than 100 feet.
- d. Old River Pump Station: The City constructed and installed a pump station at Old River, as shown in PS-1-11 of the Design Package for Drainage Improvements along the WSID Main Drain, Project 84010017 dated 10/22/01.
- e. Byron Road: During the construction of the City's Outfall Project, the City replaced District's existing structure extending between Byron Road and Lowell Avenue (a distance of roughly 1,000 feet along an alignment roughly 800 feet west of Corral Hollow Road) with a 36" RCP.
- f. Lammers Road Interflow Structure: During the construction of the City's Outfall Project, the City installed a pipe connection between the City's Outfall Project's 84" SD pipeline and the District's Main Drain at a location where the two facilities align adjacent to each other along Lammers Road approximately 1,300 feet north of Grant Line Road. This pipe connection will essentially operate as an interflow structure for the City and/or the District to use to divert flow from one system to the other in the event of an emergency or to facilitate maintenance. The interflow structure may be regulated by a manually operated sluice gate, with secured use only allowed by City and District personnel. The sluice gate shall remain shut during all times when interflow releases are not needed for emergency or maintenance purposes. City and District shall inform each other, and a representative of each party shall be present when they are proposing to open the sluice gate; unless the other party representative is not present upon two hours prior telephonic notice. City and District shall be held responsible for any adverse downstream impacts caused by their respective opening and use of the sluice gate governing the interflow structure.
- g. Byron Road/Belconte Drive Interflow Structure: During the construction of the City's Outfall Project, the City installed a 36" pipe connection between the City's Outfall Project junction structure at the Byron Road/Belconte Drive intersection and the City's open channel segment to the north that drains to District facilities at Grant Line Road and ultimately discharges into the District's Main Drain. The purpose of this interflow structure will be to allow the City to divert a portion of flow from the City's Outfall Project to the District's Main Drain in the event of an emergency, provided that all such flow shall be within the parameters allowed by Section 2(a) of this 2010 Agreement. The interflow structure may be regulated by a manually operated sluice gate, with secured use allowed only by City personnel. The sluice gate shall remain shut at all

times when interflow releases are not needed for emergency purposes. City shall immediately inform District, and a representative of each party shall be present when the City is proposing to open the sluice gate; unless the other party representative is not present upon reasonable prior telephonic notice. This interflow structure shall be owned and operated by the City, and City shall be held responsible for any adverse downstream impacts caused by their respective opening and use of the sluice gate governing the interflow structure.

h. District Open Ditch Adjacent to Plasencia Field: The City allowed the filling in of the District's existing, narrow open ditch extending along the west edge of Plasencia Field and placement of an underground pipe within and parallel to the west edge of Plasencia Field to accommodate the District's operational spill and drainage flow.

5. Lammers Area Provisions. The City's discharge of storm water into the District's Sub-Main Drain authorized by this 2010 Agreement is further subject to the terms of this Section 5.

a. Connection. Before the City makes a physical connection to discharge into the District's Sub-Main Drain, City shall:

(1) construct (at its own expense) a new 42" SD culvert crossing of Von Sosten Road at the District's Sub-Main Drain. The District shall have the right to review, and shall approve the construction plans for the new culvert crossing in writing prior to City's initiation of construction. The parties agree to coordinate construction activities to avoid undue interruption to District or City operations. City warrants the construction of the new culvert crossing against defects in materials or workmanship for a period of one year from and after the date of completion and acceptance by District. During this period, the City shall replace any defective component of the construction at its own expense; and

(2) pay the District a one-time Connection Fee of \$240,000.

b. Drainage Fees. Drainage fees are the fees the City pays the District regarding the Lammers Area. The City shall prepay Drainage Fees to authorize discharges for the following blocks of time:

- (1) Years 1 through 10 – \$480,000 due before connection and initial discharge;
- (2) Years 11 through 20 – \$600,000 due before the beginning of Year 11; and
- (3) Years 21 through 30 – \$720,000 due before the beginning of Year 21.

The trigger dates for required Drainage Fee payments shall be the date that the City's physical discharge "connection" is made to the District's Sub-Main Drain and its subsequent applicable anniversary dates.

c. Term; Extension Option. The term of the City's Lammers Area discharge rights granted pursuant to this 2010 Agreement shall be 30 years. The City has the option to negotiate a second 30-year term with the District for Lammers Area discharge rights prior to the end of the initial 30-year period. The parties agree to come to reasonable terms as part of the negotiation process.

d. Interim Drainage Agreements/Option Fee. If the District is approached by a private party requesting a discharge allocation to the District's Sub-Main Drain that will reduce the discharge allocation available to the City prior to City payment of the Lammers Area Connection Fee and the initial Lammers Area Drainage Fee, the District shall extend to the City the Right of First Refusal to preserve its discharge allocation. In order to exercise its Right of First refusal, the City

shall pay, within 30 days of written notice by District of the City Right of First Refusal, an initial Option Fee of \$35,000 and shall pay Option Fees in that same amount annually until such time as the City formally exercises its discharge rights by payment of the Connection Fee and initial Drainage Fee.

If the City elects not to exercise its Right of First Refusal by payment of the Option Fee, the District shall have the right to negotiate interim drainage agreements with private parties before the City acquires its drainage rights (by paying the initial Connection Fee and Drainage Fee described in this Section). However, the interim drainage agreements shall not interfere with the City's installation of improvements and shall terminate on the date that the City's physical "connection" is made to the District's Sub-Main Drain. To facilitate this section, City shall provide District with written notice of its intent to install improvements at least 12 months prior to beginning installation of improvements.

e. Disconnection. If at any time the City does not satisfy the Drainage Fee or Extension Option provisions of this Section, and in the absence of other provisions or circumstances that facilitate a continuing right to discharge, the City shall disconnect its storm drainage facilities and cease further discharge to the District's Sub-Main Drain until subsequent authorization for resumption of discharge is formally given by the District.

6. Routine Maintenance. Except as specifically provided in this 2010 Agreement to the contrary, both the City and District shall maintain their respective facilities at all times so that the other may discharge the quantities of drain water specified in Section 2 into the facilities of the other in perpetuity. Each of the parties agrees to maintain its respective drainage facilities at its own cost and expense.

7. Connection Costs. Except as specifically provided in this 2010 Agreement to the contrary, each of the parties shall bear the expense of connecting to the other's drainage facility (and shall bear the expense of installing and maintaining any metering devices). The other party shall approve the connections (and meters) before they are completed and/or installed.

8. Meters. The parties each contemplate that the other will maintain drainage pursuant to this 2010 Agreement within the cfs limitations specified in Section 2. However, if either party expresses concerns that the other is meeting or exceeding the cfs limitations, it may demand a meeting with the other party, and request assurances in that respect. If the requesting party continues to express concerns that the other is meeting or exceeding the cfs limitations after the meet and confer process, it shall have the right to demand that the other party install meters, at the other party's sole cost, upon three months' prior written notice.

9. Water Quality.

a. Each of the parties will insure at all times that the drainage water to be discharged into the facilities of the other meets the water quality standards and waste discharge requirements prescribed by the State of California, the United States Government, any department or agency of either of them and any other governmental agency or authority which has or shall have jurisdiction or prescribe and enforce water quality standards and waste discharge requirements. The discharge of each party shall meet the requirements imposed upon the receiving party.

b. The parties agree that urban and agricultural discharge constituents are generally different and governed by different regulatory requirements. The parties agree that neither party shall discharge water into the other's system at any time or in any amount, that will create a risk of harm to property of any kind or character from flooding or otherwise. The parties further agree that neither party shall discharge water into the other's system that is deleterious to plant or animal life.

10. Restrictions on Relocation. After the construction and installation of connections to the respective drainage facilities, the entity owning the drainage facility shall not relocate or otherwise change the facility without concurrently changing the location of the connections (and meter, if any) at the cost and expense of the entity relocating the facility, including but not limited to the acquisition and dedication of any and all easements and rights of way necessary for the relocation and improvement.

11. Prior Agreement. City and District acknowledge that prior to entering into the Original 1972 Agreement they entered into a drainage agreement on an acreage basis whereby City is permitted to drain a certain number of acres into the District's drainage facility for a specified price per acre. It is mutually agreed that: the former drainage agreement shall remain in full force and effect; there are approximately 30 acres that can be drained under the former drainage agreement; and the drainage of all of the acres covered by the former drainage agreement does not affect the quantity of drainage provided for in this 2010 Agreement.

12. Disputes. If a dispute occurs between the parties as to any right, alleged right, obligation or alleged obligation under this 2010 Agreement, the parties shall make a good faith effort to resolve the dispute. Notice of such dispute shall be given in writing to the other and shall include a description of the disputed issue or issues, a description of the possible solutions or resolutions to the dispute and the course of action or solution advocated by the party declaring the dispute. The party receiving the notice of dispute described shall respond in writing within 30 days. The response shall contain that party's response to the issues raised and the responding party's proposed resolution to the dispute. The parties shall then meet within 30 days and attempt to resolve the dispute.

13. Attorneys Fees. If legal action is necessary to enforce any of the provisions of this 2010 Agreement, the losing party agrees to pay the prevailing party (as a part of the damages) reasonable attorney fees and court costs established by a court of competent jurisdiction.

14. Right of First Refusal. Upon dissolution of the District or the District's discontinuation of providing drainage through the Main Drain and Sub-Main Drain, the City is hereby granted the Right of First Refusal to acquire the Main Drain and Sub-Main Drain.

15. Hold Harmless. Each party shall indemnify, defend, and hold harmless the other (including its elected officials, officers, agents, and employees) from and against any and all claims, demands, damages, liabilities, costs and expenses (including court costs and attorney's fees) resulting from or arising out of the performance of this 2010 Agreement except for the negligent acts of the other party.

16. Modifications. This 2010 Agreement supersedes all prior negotiations and agreements of the parties, including, but not limited to the Original 1972 Agreement and Amendments, and contains the entire agreement of the parties on the matters covered here. This 2010 Agreement may not be modified orally, or in any other manner, other than by an agreement in writing signed by the parties.

17. Binding on Successors. This 2010 Agreement shall inure to the benefit and bind the successors and assigns of the parties.

IN WITNESS WHEREOF the parties agree to the full performance of the terms set forth in this 2010 Agreement.

***** SIGNATURES CONTAINED ON NEXT PAGE*****

THE WEST SIDE IRRIGATION DISTRICT
A Political Subdivision of the State of California

By: Jack Alvarez
Jack Alvarez, President

ATTEST:
By: Barbara Kleinert
Barbara Kleinert, Secretary

By: [Signature]
Attorney for District
Approved as to Form

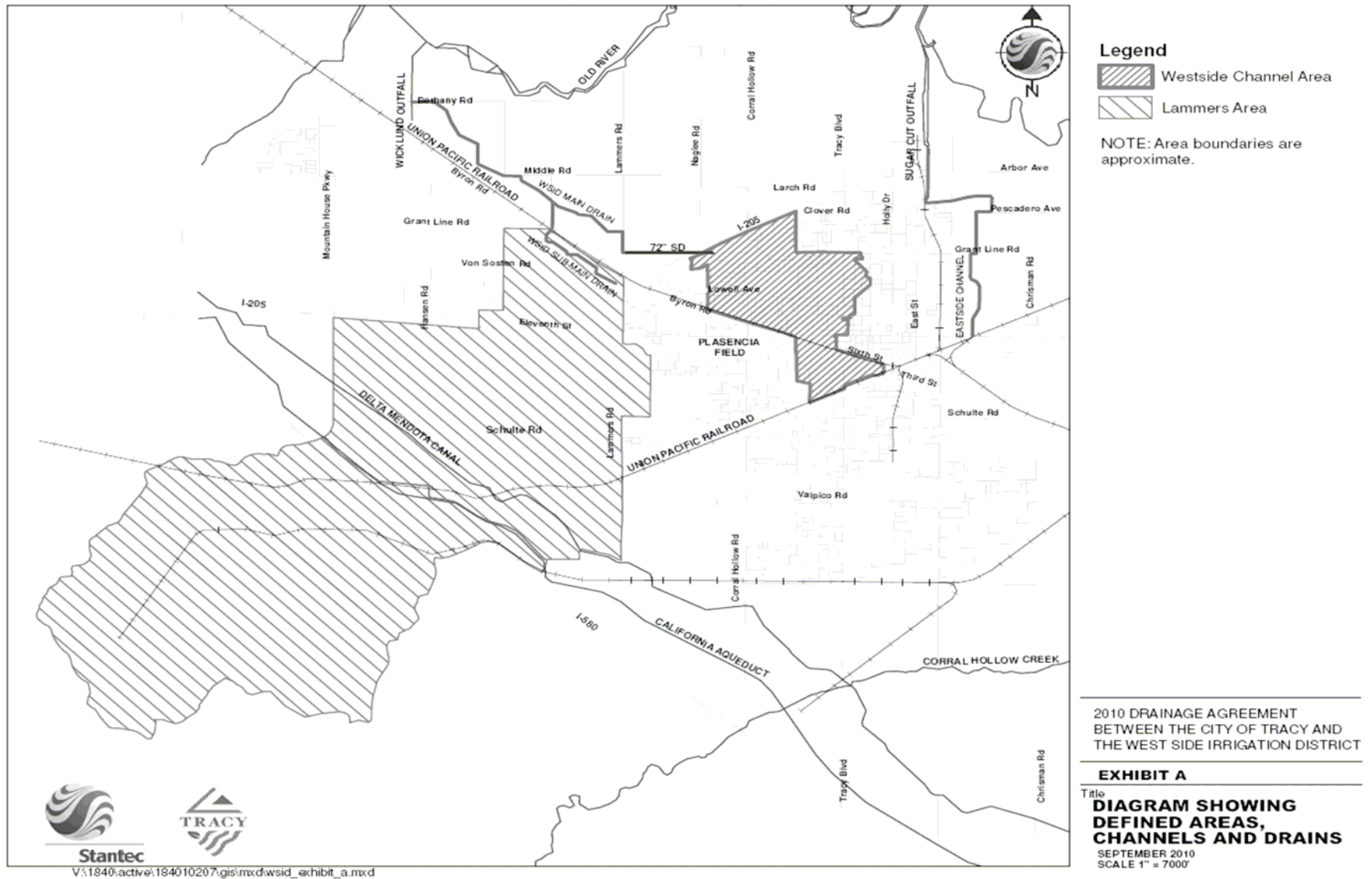
THE CITY OF TRACY
A Municipal Corporation

By: [Signature]
Mayor

ATTEST:
By: [Signature]
City Clerk

By: [Signature]
City Attorney
Approved as to Form

Exhibit A: Diagram Showing Defined Areas, Channels and Drains





Stantec Consulting Services Inc.

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